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# Camperdown Condominiums

## PRELIMINARY STORMWATER MANAGEMENT REPORT

2220740 Ontario Inc. c/o Romspen Investment Corp.

# Document Control

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Prepared by:

**Tatham Engineering Limited**  
115 Sandford Fleming Drive,  
Suite 200  
Collingwood, Ontario L9Y 5A6  
**T 705-444-2565**  
**tathameng.com**

Prepared for:

**2220740 Ontario Inc. c/o Romspen  
Investment Corp.**  
162 Cumberland Street, Suite 302  
Toronto, Ontario M5R 3N5

Authored by:	Reviewed by:
	
Andrew Schoof, B.A.Sc., M.A.Sc. Intern Engineer	Randy Simpson, B.A.Sc., P.Eng. Director, Group Leader

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Issue	Date	Description
1	June 9, 2020	Draft Plan Submission
2	October 8, 2020	Draft Plan Submission

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STM-1:	Stormwater Catchment Area Plan



# 1 Introduction

Tatham Engineering Limited has been retained by 222074 Ontario Inc. to provide engineering services in support of a proposed condo development located on Old Lakeshore Road and Camperdown Road in the Town of The Blue Mountains. Specifically, this report has been prepared to demonstrate the preferred site servicing strategy and provide information relating to stormwater management and drainage.

## 1.1 SITE DESCRIPTION

The site is located on Old Lakeshore Road within the Craighleith Camperdown sub-watershed study area in the Town of The Blue Mountains. Currently, the site is zoned Residential (R3-H), Public Open Space (OS1) and Hazard (H) in accordance with Town By-law 2006-22. It is legally described as Part Lot 26 Concession 6 in the former Collingwood Township. A portion of the proposed development resides within the Nipissing Ridge geological region of the Georgian Bay Peninsula. We have enclosed a Site Location Plan (Fig.1) for your reference.

## 1.2 GEOTECHNICAL INVESTIGATION & REPORTS

Based on the Soil Survey of Grey County Map No. 17 (North), the on-site soils are Tecumseth Sand Loam (TS), Waterloo Sand Loam (Wsl) and Dunedin Clay (Duc). The soil material is characterized as poorly sorted outwash sand and clay. Tecumseth Sand Loam, Waterloo Sand Loam and Dunedin Clay have hydrologic soil group classifications of 'AB', 'A' and 'D' respectively. Soils of this nature are categorized as having 'good to imperfect' drainage which results in moderate infiltration.

Background reports prepared by C.F. Crozier & Associates Inc. obtained from the Grey County and Town of The Blue Mountains website suggests that Peto MacCallum Ltd. completed five test pit investigations on site to review the existing soil conditions in June of 2004. The report suggests that a uniform layer of topsoil (0.10 m depth) generally covers the site with underlying subsurface soil material consisting of native silty-clay overlaying weathered bedrock at a depth of between 0.8 m to 2.0 m.

Further geotechnical investigations were completed on May 10, 2019 by Central Earth Engineering. The investigation included excavating 8 test pits across the site to provide recommendations for foundations, slab on grade, pavement structure, soil excavation, compaction and ground water control.

The test pits encountered a topsoil layer at the ground surface between 250 mm to 400 mm thick. Underlying the topsoil, the test pits encountered undisturbed native soils that extended down to the bedrock surface. The undisturbed soil primarily consisted of silt and clay with trace sand



extending to elevations ranging from 190.1 to 188.8 m). Bedrock was encountered beneath the overburden soils in each test pit at elevations ranging from 188.5 to 190.1 m. The upper 0.3 to 1.0 m of the bedrock is considered rippable, and the excavations were advanced until bucket refusal on bedrock at elevations of 189.5 to 188.2 m.

The geotechnical report has been submitted under separate cover.

### **1.3 EXISTING LAND USE**

The site is located at the base of the Nipissing Ridge formation on a flat plateau containing forested and open space areas with the land sloping from southwest to northeast between 2% and 5%.

### **1.4 PROPOSED LAND USE**

The current site plan prepared by Innovative Planning Solutions (IPS) illustrates the proposed development consisting of 33 residential units, a 18.0 m ROW, open space (non-developable land) and a stormwater management block.



## 2 Stormwater Management

### 2.1 STORMWATER MANAGEMENT OBJECTIVES AND BACKGROUND

The primary objective of the Stormwater Management Plan is to identify the existing and future drainage conditions in the area of the site to develop a plan that will mitigate the impact of the development of the local drainage systems. In addition, this plan will demonstrate that the development can be completed in accordance with applicable Municipal, Regional and Provincial guidelines.

This will be accomplished by evaluating the effect of the development on the local drainage conditions, constructing on-site quality control measures, and providing solutions to mitigate siltation and erosion during and after construction.

The stormwater management strategy for the proposed development site has been prepared recognizing the pertinent Conservation Authority, Municipal and Provincial guidelines on water resources including the following:

- *Policies for the Administration of the Development, Interference with Wetlands and Alterations to Shorelines and Watercourses Regulation (Ontario Regulation 151/06)*, Grey Sauble Conservation Authority, (January 2010);
- *The Blue Mountains Engineering Standards*, Town of The Blue Mountains, (April 2009); and,
- *Stormwater Management Planning and Design Manual*, Ministry of the Environment, (March 2003).

### 2.2 STORMWATER MANAGEMENT CRITERIA

Several environmental factors and site conditions govern the design of the stormwater management plan for the residential development. The SWM criteria to be adhered to during detailed design are as follows:

- Pre-to -post peak runoff flow rate matching for the 2 through 100-year design storm event;
- Safe conveyance of the Regional Storm Event through the development into the township's right-of-way; and
- Level 1 'Enhanced' water quality treatment will be designed to meet or exceed Municipal and Provincial standards. The plan must achieve 80% total suspended solids (TSS) removal prior to off-site discharge.



### 3 Pre-Development Conditions

#### 3.1 EXISTING SITE CONDITIONS

The subject property, which consists of parts of Catchment 101 and Catchment 102 (see DP-1), currently contains forested and open space areas and slopes from the southwest to northeast towards Old Lakeshore Road between 2% and 5%.

Catchment 101 contains external drainage from the rear-yards of the existing residential properties on the north side of Camperdown Court. This external drainage flows northeast through the subject property and crosses Old Lakeshore Road through an existing 900 mm x 900 mm concrete box culvert.

Catchment 102 contains external drainage from the rear-yards of a portion of the existing residential properties on the north side of Barton Boulevard. This external drainage flows northeast through the subject property and crosses Old Lakeshore Road through an existing 1500 mm x 1500 mm concrete box culvert.

Stormwater from Catchments 101 and 102 travels east after crossing Old Lakeshore Road, and eventually crosses Highway 26 through existing culverts where it outlets to an existing watercourse, and ultimately discharges into Nottawasaga Bay.

A preliminary Visual OTTHYMO model has been developed based on the current concept plan to quantify the pre-development peak runoff flow rates from the site. The model has been developed utilizing the Ministry of Transportation IDF Curve Lookup rainfall data and existing information from the Camperdown East 1 Limited Residential Development Preliminary Servicing and Stormwater Management Report (February 2009). Table 1 summarizes the hydrological parameters for the existing site.

**Table 1: Pre-Development Hydrologic Parameters**

Catchment ID	Catchment Area (Ha)	CN Number	Initial Abstraction (mm)	Runoff Coefficient	Time of Concentration (min)	Time to Peak (hr)
101	5.92	52.48	7.7	0.26	15.4	0.17
102	2.91	59.65	7.23	0.36	13.5	0.15





### 3.2 PRE-DEVELOPMENT PEAK RUNOFF FLOW RATE ANALYSIS

Table 2 summarizes the pre-development peak runoff flow rates from each catchment and the total peak runoff flow rate for the property. We have enclosed the Pre-Development Drainage Plan (DP-1) for reference.

**Table 2: Pre-Development Peak Runoff Flow Rate Summary**

Design Storm	4-Hour Chicago (m <sup>3</sup> /s)	24-Hr SCS Type II (m <sup>3</sup> /s)
25 mm	0.044	-
2-Year	0.051	0.199
5-Year	0.112	0.364
10-Year	0.164	0.496
25-Year	0.242	0.681
50-Year	0.305	0.828
100-Year	0.375	0.985
Regional	0.567	-

Detailed pre-development Visual OTTHYMO modeling results have been enclosed in Appendix A.



## 4 Post-Development Water Quantity Control

### 4.1 PROPOSED SITE CONDITIONS

The proposed stormwater management plan for the development will ensure the post-development peak runoff flow rates from the site are attenuated to pre-development levels while providing 'Enhanced' Level 1 stormwater quality control.

The proposed 33-unit development will have a combination of impervious areas consisting of houses, driveways and the proposed road, while the remaining areas will consist of the SWM Pond block, open space (non-developable land) and grassed/lawn areas (see DP-2).

The proposed drainage conditions will include an enhanced low flow cut off swale along the south limit of the development. This swale will intercept drainage from the rear-yards above the Nipissing Ridge and the open space south of the development (Catchments 202 & 203) and convey the runoff around the proposed development directly to the Old Lakeshore Road roadside ditch. Catchment 201 consists of the lots on the south side of the internal road, the majority of the internal road and also accounts for the driveways and front half of the houses on lots 22, 24 and 26 through 33. Runoff from Catchment 201 will be directed to an end of pipe stormwater management facility via an internal storm sewer system (minor system) and overland flow through the roadway (major system). Controlled runoff from the SWM facility will be conveyed eastward via the roadside ditch along Old Lakeshore Road, ultimately discharging to the existing 1500 mm concrete box culvert, thereby combining with the intermittent watercourse.

Uncontrolled drainage from the remaining catchments (204, 205, 206 & 207) will be intercepted by the Old Lakeshore Road ditch and will ultimately outlet to the intermittent watercourse at the east limit of the property via the previously mentioned box culvert.

The Old Lakeshore Road ditch and culverts have been sized to convey the 100-year post development runoff from the site to the existing concrete box culvert. The 750 mm dia. culvert has a conveyance capacity of 0.43 m<sup>3</sup>/s and the 900 mm dia. culvert has a conveyance capacity of 0.69 m<sup>3</sup>/s

### 4.2 POST-DEVELOPMENT PEAK RUNOFF FLOW RATE ANALYSIS

Detailed impervious calculations for each catchment are enclosed in Appendix B. Table 3 summarizes the hydrologic parameters for the proposed development.



**Table 3: Post-Development Catchment Parameters**

Catchment ID	Catchment Area (Ha)	Curve Number (CN)	% Impervious	% Impervious Directly Connect
Catchment 201	2.20	-	43.4	20.9
Catchment 202	1.63	44.1	-	-
Catchment 203	2.89	42.9	-	-
Catchment 204	0.64	48.1	-	-
Catchment 205	0.03	-	56.0	28.0
Catchment 206	0.73	-	31.8	12.7
Catchment 207	0.78	66.3	-	-

Table 4 below summarizes the post-development total peak flow rates from the development site. We have enclosed the Post-Development Drainage Plan (DP-2), storm sewer design sheet and detailed post-development Visual OTTHYMO modelling results in Appendix B for reference.

**Table 4: Post-Development Peak Runoff Flow Rate Summary**

Design Storm	Peak Runoff Flow Rate (m <sup>3</sup> /s)	
	4-Hour Chicago	24-Hr SCS Type II
25 mm	0.039 (0.044)	-
2-Year	0.051 (0.051)	0.157 (0.199)
5-Year	0.106 (0.112)	0.281 (0.364)
10-Year	0.152 (0.164)	0.370 (0.496)
25-Year	0.212 (0.242)	0.491 (0.681)
50-Year	0.259 (0.305)	0.583 (0.828)
100-Year	0.310 (0.375)	0.682 (0.985)
Regional	0.390 (0.567)	-

(0.010) – Pre-development Flow Rates

#### 4.3 STORMWATER MANAGEMENT FACILITY

The development will contain an extended detention wet pond constructed in accordance with the MECP Stormwater Management Planning and Design Manual (March 2003).



The outlet control structure will be located at the northeast corner of the SWM facility. Discharge from the facility will be released by an outlet structure to the roadside ditch on Old Lakeshore Road where it will be conveyed to the existing 1500 mm x 1500 mm box culvert crossing Old Lakeshore Road at the northeast corner of the property.

Extended detention will be achieved utilizing a 300 mm diameter orifice. All flows up to and including the 100-year event will be safely conveyed through the outlet control chamber facility. The Regional storm event will be safely conveyed to Old Lakeshore Road via overland flow.

The Visual OTTHYMO hydrologic model has been used to evaluate the function of the proposed wet pond. A summary of the storage volumes and water levels for the facility are provided in Table 5.

**Table 5: Post-Development SWM Facility Volume Summary**

Design Storm	Storage Volume Used (m <sup>3</sup> )	Water Surface Elevation (m)
25 mm	41	189.14
2-year	219	189.72
5-year	339	189.97
10-year	422	190.15
25-year	614	190.37
50-year	711	190.50
100-year	813	190.60
Regional	1898	191.74

Detailed modelling results have been included in Appendix B for reference.

#### **4.4 WATER QUALITY CONTROL**

The primary outlet receiver for the site is the existing intermittent watercourse on the east side of the site and ultimately drains to Nottawasaga Bay which is a cold-water fishery. Level 1 'Enhanced' water quality treatment is required in the form of 80% total suspended solids (TSS) removal prior to off-site discharge. This will be achieved on-site through lot level controls and an oil/grit separator that will be sized during final design.



## 5 Siltation and Erosion Controls

Siltation and erosion controls will be implemented for all construction activities within the development site, including vegetation clearing, topsoil stripping, material stockpiling, road construction activities and grading operations. The detailed erosion and sediment control measures proposed will be implemented during and after construction and will be provided during final design and may include the following:

- heavy duty silt fence will be erected around the perimeter of the site before any grading operations commence to control sediment movement;
- a construction vehicle entrance will be constructed and maintained consisting of a stone mud mat to reduce off-site tracking of material; and
- rock check flow dams and straw bale check flow dams will be installed prior to construction and will be maintained and inspected throughout the course of construction as required to prevent the transportation of sediment and delirious materials offsite.



## 6 Summary

The proposed Stormwater Management Plan demonstrates that the development will meet the established criteria with respect to stormwater management set forth in governing documents and can proceed without negatively impacting the local drainage systems. Level 1 'Enhanced' water quality control in the form of 80% TSS removal and water quantity control in the form of post to pre-development peak flow matching will be satisfied through the use of internal storm sewers and overland flow culminating in an end-of-pipe stormwater management pond.

In conclusion, the proposed stormwater management plan supports the concept of an environmentally sustainable development and will mitigate anticipated stormwater impacts associated with the construction of the proposed development.





**CAMPERDOWN CONDOMINIUMS**  
SITE LOCATION PLAN


SCALE: N.T.S.

DATE: NOV/19

DWG NO. FIG. 1

## **Appendix A: Pre-Development Hydrological Analysis**



	<b>Project:</b>	Camperdown Condominiums
	<b>File No.:</b>	117304
	<b>Date:</b>	March 2018
	<b>Designed By:</b>	AS
	<b>Checked By:</b>	RS
	<b>Subject:</b>	CN Calculator

**CURVE NUMBER, INITIAL ABSTRACTION & TIME TO PEAK CALCULATIONS**

Catchment 101 Area 5.92 ha

WEIGHTED CN VALUE																									
Soil Series	Soil Series	Hydrologic Soil Group	Soil Texture	Runoff Coefficient Type	Catchment Soil Characteristics		Forest/Woodland			Pasture/Lawns			Meadows			Cultivated			Impervious			Wetland/Lakes/SWMF			Average CN for Soil Type
					Area	Percent	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	
TS	TECUMSETH	AB	Sand Loam	1	4.14	0.7	2.49	0.6	46	0.83	0.2	59	0.8288	0.2	51	0		68	0		100	0		49.6	
WSL	WATERLOO	A	Sand Loam	1	1.78	0.3	0.53	0.30	32	0.71	0.40	49	0	0	38	0		62	0.5328	0.3	100	0		59.2	
	#N/A	#N/A	#N/A	#N/A	0		0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		0	
	#N/A	#N/A	#N/A	#N/A	0		0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		0	
	#N/A	#N/A	#N/A	#N/A	0		0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		0	
Totals					5.92	1	3.0192	0.51		1.5392	0.26		0.8288	0.14		0	0		0.5328	0.09		0	0	52.48	

**Time of Concentration Calculations**

For Runoff Coefficients greater than 0.4

**Bransby-Williams Formula**

Maximum Catchment Elevation 223 m  
 Minimum Catchment Elevation 190.5 m  
 Catchment length 200 m  
 Catchment Slope 16%  
 Catchment Area 5.92 ha

Time of Concentration (Minutes) 5.46  
 Time of Concentration (Hours) 0.09  
 Time to Peak (2/3 x Time of Concentration) 0.06

**Time to Peak** 0.17 hrs

For Runoff Coefficients less than 0.4

**Airport Method**

Maximum Catchment Elevation 223 m  
 Minimum Catchment Elevation 190.5 m  
 Catchment length 200 m  
 Catchment Slope 16%  
 Catchment Area 5.92 ha


Time of Concentration (Minutes) 15.39  
 Time of Concentration (Hours) 0.26  
 Time to Peak (2/3 x Time of Concentration) 0.17

**Initial Abstraction** 7.7 mm

**Runoff Coefficient** 0.26

Wetlands	12
Woods	10
Meadows	8
Cultivated	7
Lawns	5
Impervious	2

Landuse Type	Soil Series				
	TS	WSL	0	0	0
Forest/Woodland	0.18	0.18	#N/A	#N/A	#N/A
Cultivated	0.4	0.4	#N/A	#N/A	#N/A
Pasture/Lawn	0.22	0.22	#N/A	#N/A	#N/A
Impervious	0.95	0.95	#N/A	#N/A	#N/A
Wetland/Lake/SWMF	0.05	0.05	#N/A	#N/A	#N/A
Meadows	0.20	0.20	#N/A	#N/A	#N/A
Soil Series Total	0.192	0.427	#N/A	#N/A	#N/A

	Project:	Camperdown Condominiums
	File No.:	117304
	Date:	March 2018
	Designed By:	AS
	Checked By:	RS
	Subject:	CN Calculator

# CURVE NUMBER, INITIAL ABSTRACTION & TIME TO PEAK CALCULATIONS

Catchment 102 Area 2.91 ha

WEIGHTED CN VALUE																			
Soil Series	Soil Series	Hydrologic Soil Group	Soil Texture	Runoff Coefficient Type	Catchment Soil Characteristics			Forest/Woodland			Pasture/Lawns			Meadows			Cultivated		
					Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN
TS	TECUMSETH	AB	Sand Loam	1	1.16	0.40	0.93	0.8	46	0.2328	0.2	59	0	51	0	68	0	100	0
WSL	WATERLOO	A	Sand Loam	1	0.73	0.25	0.58	0.8	32	0.15	0.2	49	0	38	0	62	0	100	0
DUC	DUNEDIN	D	Clay Loam or Clay	3	1.02	0.35	0.00	0	79	0.66	0.65	84	0	81	0	86	0.3565	0.35	100
	#N/A	#N/A	#N/A	#N/A	0		0	#N/A	0		#N/A	0	#N/A	0		#N/A	0	#N/A	0
	#N/A	#N/A	#N/A	#N/A	0		0	#N/A	0		#N/A	0	#N/A	0		#N/A	0	#N/A	0
Totals					2.91	1.00	1.51	0.52		1.04	0.36		0	0	0	0	0.35648	0.1225	0
																			59.65

## Time of Concentration Calculations

For Runoff Coefficients greater than 0.4

## Bransby-Williams Formula

Maximum Catchment Elevation 223 m  
Minimum Catchment Elevation 190.5 m  
Catchment length 200 m  
Catchment Slope 16%  
Catchment Area 2.91 ha

Time of Concentration (Minutes) 5.87  
Time of Concentration (Hours) 0.10  
Time to Peak (2/3 x Time of Concentration) 0.07

**Time to Peak** 0.15 hrs

For Runoff Coefficients less than 0.4

## Airport Method

Maximum Catchment Elevation 223 m  
Minimum Catchment Elevation 190.5 m  
Catchment length 200 m  
Catchment Slope 16%  
Catchment Area 2.91 ha

Time of Concentration (Minutes) 13.53  
Time of Concentration (Hours) 0.23  
Time to Peak (2/3 x Time of Concentration) 0.15

**Initial Abstraction** 7.2325 mm

Wetlands	12
Woods	10
Meadows	8
Cultivated	7
Lawns	5
Impervious	2

**Runoff Coefficient** 0.36

Landuse Type	Soil Series				
	TS	WSL	DUC	0	0
Forest/Woodland	0.18	0.18	0.52	#N/A	#N/A
Cultivated	0.4	0.4	0.7	#N/A	#N/A
Pasture/Lawn	0.22	0.22	0.55	#N/A	#N/A
Impervious	0.95	0.95	0.95	#N/A	#N/A
Wetland/Lake/SWMF	0.05	0.05	0.05	#N/A	#N/A
Meadows	0.20	0.20	0.54	#N/A	#N/A
Soil Series Total	0.188	0.188	0.69	#N/A	#N/A

## Active coordinate

44° 32' 15" N, 80° 23' 45" W (44.537500,-80.395833)

Retrieved: Fri, 05 Jan 2018 21:16:03 GMT



### Location summary

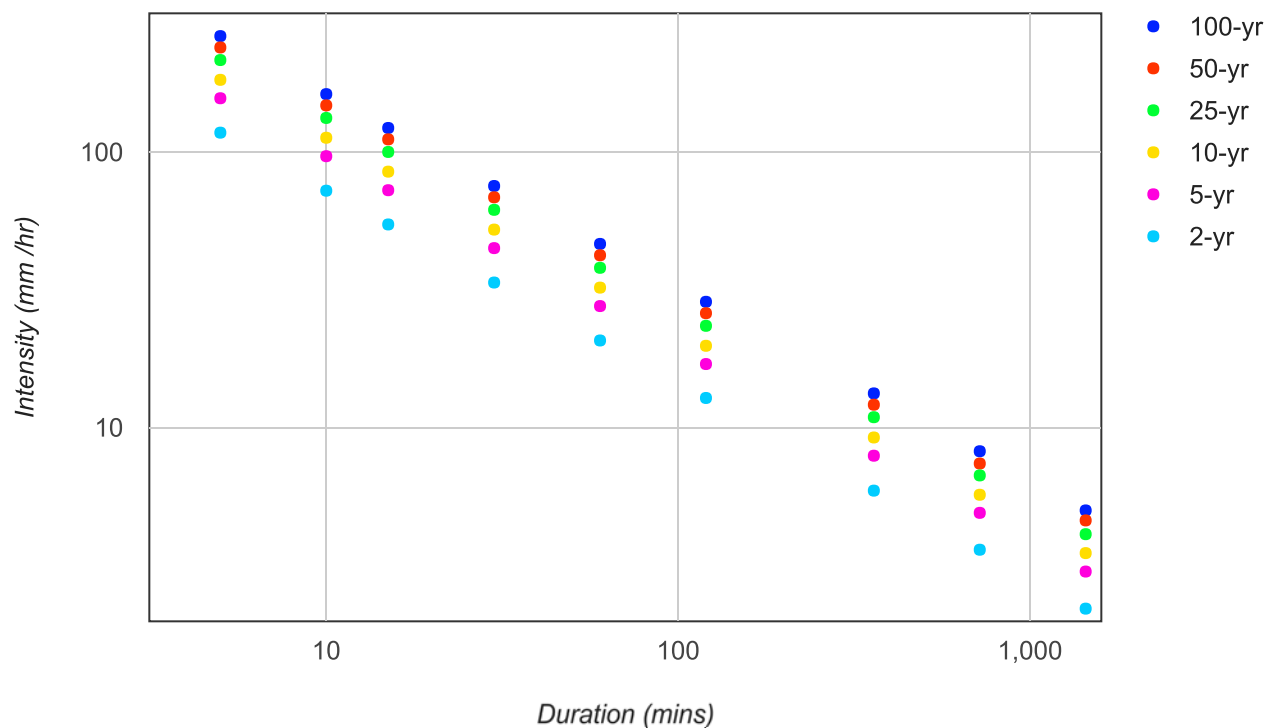
These are the locations in the selection.

**IDF Curve:** 44° 32' 15" N, 80° 23' 45" W (44.537500,-80.395833)

### Results

An IDF curve was found.

Coordinate: 44.537500, -80.395833  
IDF curve year: 2010



## Coefficient summary

**IDF Curve:** 44° 32' 15" N, 80° 23' 45" W (44.537500,-80.395833)

Retrieved: Fri, 05 Jan 2018 21:16:03 GMT

**Data year:** 2010

**IDF curve year:** 2010

Return period	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr
<b>A</b>	20.7	27.6	32.2	38.0	42.2	46.4
<b>B</b>	-0.699	-0.699	-0.699	-0.699	-0.699	-0.699

## Statistics

### Rainfall intensity (mm hr<sup>-1</sup>)

Duration	5-min	10-min	15-min	30-min	1-hr	2-hr	6-hr	12-hr	24-hr
<b>2-yr</b>	117.6	72.4	54.6	33.6	20.7	12.8	5.9	3.6	2.2
<b>5-yr</b>	156.8	96.6	72.7	44.8	27.6	17.0	7.9	4.9	3.0
<b>10-yr</b>	182.9	112.7	84.9	52.3	32.2	19.8	9.2	5.7	3.5
<b>25-yr</b>	215.8	133.0	100.1	61.7	38.0	23.4	10.9	6.7	4.1
<b>50-yr</b>	239.7	147.7	111.2	68.5	42.2	26.0	12.1	7.4	4.6
<b>100-yr</b>	263.6	162.3	122.3	75.3	46.4	28.6	13.3	8.2	5.0

### Rainfall depth (mm)

Duration	5-min	10-min	15-min	30-min	1-hr	2-hr	6-hr	12-hr	24-hr
<b>2-yr</b>	9.8	12.1	13.6	16.8	20.7	25.5	35.5	43.7	53.9
<b>5-yr</b>	13.1	16.1	18.2	22.4	27.6	34.0	47.3	58.3	71.8
<b>10-yr</b>	15.2	18.8	21.2	26.1	32.2	39.7	55.2	68.0	83.8
<b>25-yr</b>	18.0	22.2	25.0	30.8	38.0	46.8	65.2	80.3	98.9
<b>50-yr</b>	20.0	24.6	27.8	34.3	42.2	52.0	72.4	89.2	109.8
<b>100-yr</b>	22.0	27.1	30.6	37.7	46.4	57.2	79.6	98.0	120.8

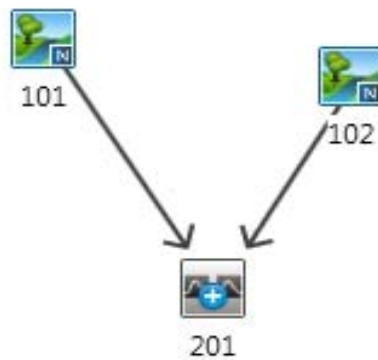
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Last Modified: September 2016

**CAMPERDOWN CONDOMINIUM**  
**PRE-DEVELOPMENT CONDITIONS**



Nashyd



Standhyd



Addhyd



Route Pipe



Route Channel



Route Reservoir



Duhyd



Diverthyd



**Project:** Camperdown Condominium

**File No.:** 117304

**Subject:** Otthymo Flow Schematic

**Date:** Nov 2019 **Figure:** 1

CHI PRE.txt  
=====

V V I SSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* S U M M A R Y O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vi sual OTTHYMO 5.0\V02\voi n.dat

Output filename:  
C:\Users\vaschoof\AppData\Local \Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\072a7  
622-91ba-426a-b460-64955a0e79ef\scen

Summary filename:  
C:\Users\vaschoof\AppData\Local \Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\072a7  
622-91ba-426a-b460-64955a0e79ef\scen

DATE: 11/06/2019 TIME: 10: 42: 18

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 01  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT min	AREA ha	Qpeak cms	Tpeak hrs	R. V. mm	R. C.	Qbase cms	
START @ 0.00 hrs									
-----									
READ STORM		6.0							
[ Ptot= 24.97 mm ]									
fname :									
C:\Users\vaschoof\AppData\Local \Temp\341e2ecf-1ead-49b0-bcdf-2b5feaea94a3\395184f1-35									
bb-4c48-8e68-693									
remark: OWEN SOUND 25 mm (from a 2 year-4hr storm)									
*****									
** CALIB NASHYD	0101	1	2.0	5.92	0.03	2.13	1.21	0.05	0.000
[CN=52.5									
[ N = 3.0: Tp 0.17]									
*****									
** CALIB NASHYD	0102	1	2.0	2.91	0.02	2.10	1.66	0.07	0.000
[CN=59.7									
]									

CHI PRE.txt  
[ N = 3.0: Tp 0.15]

\* ADD [ 0101+ 0102] 0201 3 2.0 8.83 0.04 2.10 1.36 n/a 0.000

\*\*\*\*\*

V V I SSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* S U M M A R Y O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vi sual OTTHYMO 5.0\V02\voi n.dat

Output filename:  
C:\Users\vaschoof\AppData\Local \Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\5f24e  
3d2-8ab3-4143-9b38-ca5b6e82f73e\scen

Summary filename:  
C:\Users\vaschoof\AppData\Local \Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\5f24e  
3d2-8ab3-4143-9b38-ca5b6e82f73e\scen

DATE: 11/06/2019 TIME: 10: 42: 18

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 02  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT min	AREA ha	Qpeak cms	Tpeak hrs	R. V. mm	R. C.	Qbase cms	
START @ 0.00 hrs									
-----									
CHI C STORM		15.0							
[ Ptot= 31.36 mm ]									
*****									
** CALIB NASHYD	0101	1	2.0	5.92	0.03	1.40	2.21	0.07	0.000
[CN=52.5									
[ N = 3.0: Tp 0.17]									
*****									
** CALIB NASHYD	0102	1	2.0	2.91	0.02	1.37	2.97	0.09	0.000
[CN=59.7									
[ N = 3.0: Tp 0.15]									

CHI PRE.txt  
\* ADD [ 0101+ 0102] 0201 3 2.0 8.83 0.05 1.37 2.46 n/a 0.000  
\*  
=====

V V I SSSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A L  
V V I SSSSS UUUU A A LLLL  
000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\V02\voinput.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Civica\NH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\421d  
8f0-96b4-4a3d-8bbf-0ca22a158c6b\scen  
Summary filename:  
C:\Users\vaschoof\AppData\Local\Civica\NH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\421d  
8f0-96b4-4a3d-8bbf-0ca22a158c6b\scen

DATE: 11/06/2019 TIME: 10:42:18

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 03  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT min	AREA ha	Qpeak ' cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs								
-----								
CHIC STORM [ Ptot= 41.81 mm ]	15.0							
* ** CALIB NASHYD [CN=52.5 [ N = 3.0: Tp 0.17]	0101	1 2.0	5.92	0.07	1.37	4.40	0.11	0.000
* ** CALIB NASHYD [CN=59.7 [ N = 3.0: Tp 0.15]	0102	1 2.0	2.91	0.05	1.33	5.79	0.14	0.000
*****								

CHI PRE.txt  
\* ADD [ 0101+ 0102] 0201 3 2.0 8.83 0.11 1.37 4.86 n/a 0.000  
\*  
FINISH  
=====

V V I SSSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A L  
V V I SSSSS UUUU A A LLLL  
000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\V02\voinput.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Civica\NH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\9dc74  
5c6-67cd-4763-9d84-bbb05ca214f6\scen  
Summary filename:  
C:\Users\vaschoof\AppData\Local\Civica\NH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\9dc74  
5c6-67cd-4763-9d84-bbb05ca214f6\scen

DATE: 11/06/2019 TIME: 10:42:18

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 04  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT min	AREA ha	Qpeak ' cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs								
-----								
CHIC STORM [ Ptot= 48.78 mm ]	15.0							
* ** CALIB NASHYD [CN=52.5 [ N = 3.0: Tp 0.17]	0101	1 2.0	5.92	0.10	1.37	6.22	0.13	0.000
*****								

=====

000	TTTT	TTTT	H	H	Y	Y	M	M	000	TM
0 0	T	T	H	H	Y	Y	MM	MM	0 0	
0 0	T	T	H	H	Y		M	M	0 0	
000	T	T	H	H	Y		M	M	000	

\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Output filename:  
C:\Users\aschoof\AppData\Local\Ci vi ca\VH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\ae9eb  
bb1-9df4-4a2c-84f5-56decbb77878a\scen

Summary filename:  
C:\Users\aschoof\AppData\Local\Ci vi ca\VH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\ae9eb  
bb1-9df4-4a2c-84f5-56decbb77878a\scen

USER:

COMMENTS:

```
*****
** SIMULATION : Run 05                               **
*****
```

Page 5

=====

000	TTTT	TTTT	H	H	Y	Y	M	M	000	TM
0 0	T	T	H	H	Y	Y	MM	MM	0 0	
0 0	T	T	H	H	Y		M	M	0 0	
000	T	T	H	H	Y		M	M	000	

\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

```
Output file name:
C:\Users\aschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\2a66f
c61-7e89-47f2-8b9e-736ee42e356e\scen
Summary file name:
C:\Users\aschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\2a66f
c61-7e89-47f2-8b9e-736ee42e356e\scen
```

USER:

COMMENTS: \_\_\_\_\_

```
*****
** SIMULATION : Run 06                               **
*****
```

Page 6



CHI PRE.txt  
[ N = 3.0:Tp 0.15]  
ADD [ 0101+ 0102] 0201 3 2.0 8.83 0.31 1.37 12.04 n/a 0.000  
=====

V V I SSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL  
000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vi sual OTTHYMO 5.0\V02\voi n.dat  
Output filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\bcc5d  
207-9d45-42f4-af66-24f9ce52ad17\scen  
Summary filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\bcc5d  
207-9d45-42f4-af66-24f9ce52ad17\scen

DATE: 11/06/2019 TIME: 10:42:18

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 07  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT mi n	AREA ha	Opeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs								
CHI C STORM [ Ptot= 70.29 mm ]	15.0							
** CALIB NASHYD [CN=52.5 [ N = 3.0:Tp 0.17]	0101	1	2.0	5.92	0.22	1.37	13.39	0.19 0.000
** CALIB NASHYD [CN=59.7 [ N = 3.0:Tp 0.15]	0102	1	2.0	2.91	0.15	1.33	16.93	0.24 0.000

CHI PRE.txt  
ADD [ 0101+ 0102] 0201 3 2.0 8.83 0.38 1.33 14.55 n/a 0.000  
=====

V V I SSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL  
000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vi sual OTTHYMO 5.0\V02\voi n.dat  
Output filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\va4614  
08b-251c-4502-ae3-ceb22c683360\scen  
Summary filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\va4614  
08b-251c-4502-ae3-ceb22c683360\scen

DATE: 11/06/2019 TIME: 10:42:18

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 08  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT mi n	AREA ha	Opeak cms	Tpeak hrs	R.V. mm	R.C.	Qbase cms
START @ 0.00 hrs								
READ STORM [ Ptot=193.00 mm ] fname :	12.0							
C:\Users\vaschoof\AppData\Local\Temp\341e2ecf-1ead-49b0-bcdf-2b5feaea94a3\4c311f83-81 af-42c6-ab26-d9d remark: T1MM1NS REGIONAL 12 HOUR DURATION STORM								
** CALIB NASHYD [CN=52.5 [ N = 3.0:Tp 0.17]	0101	1	2.0	5.92	0.36	7.00	82.67	0.43 0.000

```

*
* * CHI PRE. txt
* * CALIB NASHYD 0102 1 2.0 2.91 0.21 7.00 96.49 0.50 0.000
* * [CN=59.7]
* * [ N = 3.0: Tp 0.15]
* * ADD [ 0101+ 0102] 0201 3 2.0 8.83 0.57 7.00 87.23 n/a 0.000
*

```

## SCS PRE. txt

=====

```
V   V   I   SSSSS U   U   A   L
V   V   I   SS   U   U   A A  L
V   V   I   SS   U   U   A A A L
V   V   I   SS   U   U   A   A L
VV    I   SSSSS UUUUU A   A LLLLL
```

```
000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
0 0  T  T  H  H  Y  Y  MM MM 0 0
0 0  T  T  H  H  Y  Y  M  M 0 0
000  T  T  H  H  Y  Y  M  M 000
```

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## \*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\VO2\voir.n.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\67109  
a76-acb2-47aa-8e2a-6345a87fc810\scen  
Summary filename:  
C:\Users\vaschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\67109  
a76-acb2-47aa-8e2a-6345a87fc810\scen

DATE: 11/06/2019

TIME: 10:44:49

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 01  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT min	AREA ha	' '	Qpeak cms	Tpeak hrs	R. V. mm	R. C.	Qbase cms
START @ 0.00 hrs									
-----									
MASS STORM [ Ptot= 53.74 mm ]	15.0								
* ** CALIB NASHYD [CN=52.5 [ N = 3.0: Tp 0.17]	0101	1 2.0	5.92		0.12	11.83	7.68	0.14	0.000
* ** CALIB NASHYD [CN=59.7 [ N = 3.0: Tp 0.15]	0102	1 2.0	2.91		0.08	11.80	9.91	0.18	0.000
* ADD [ 0101+ 0102]	0201	3 2.0	8.83		0.20	11.83	8.41	n/a	0.000

## SCS PRE. txt

=====

```
V   V   I   SSSSS U   U   A   L
V   V   I   SS   U   U   A A  L
V   V   I   SS   U   U   A A A L
V   V   I   SS   U   U   A   A L
VV    I   SSSSS UUUUU A   A LLLLL
```

```
000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
0 0  T  T  H  H  Y  Y  MM MM 0 0
0 0  T  T  H  H  Y  Y  M  M 0 0
000  T  T  H  H  Y  Y  M  M 000
```

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## \*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\VO2\voir.n.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\67109  
a76-acb2-47aa-8e2a-6345a87fc810\scen  
Summary filename:  
C:\Users\vaschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\67109  
a76-acb2-47aa-8e2a-6345a87fc810\scen

DATE: 11/06/2019

TIME: 10:44:49

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 02  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT min	AREA ha	' '	Qpeak cms	Tpeak hrs	R. V. mm	R. C.	Qbase cms
START @ 0.00 hrs									
-----									
MASS STORM [ Ptot= 71.58 mm ]	15.0								
* ** CALIB NASHYD [CN=52.5 [ N = 3.0: Tp 0.17]	0101	1 2.0	5.92		0.22	11.83	13.89	0.19	0.000
* ** CALIB NASHYD [CN=59.7 [ N = 3.0: Tp 0.15]	0102	1 2.0	2.91		0.15	11.80	17.53	0.24	0.000
* ADD [ 0101+ 0102]	0201	3 2.0	8.83		0.36	11.80	15.09	n/a	0.000
FINISH									

SCS PRE.txt  
=====

V V I SSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL  
000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vi sual OTTHYMO 5.0\V02\vo in.dat  
Output filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\5180e  
208-118e-4943-a8d0-2b76bca49467\scen  
Summary filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\5180e  
208-118e-4943-a8d0-2b76bca49467\scen

DATE: 11/06/2019 TIME: 10: 44: 49

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 03 \*\*  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT mi n	AREA ha	' Qpeak cms	Tpeak hrs	R. V. mm	R. C.	Qbase cms
START @ 0.00 hrs								
-----								
MASS STORM [ Ptot= 83.55 mm ]	15.0							
* ** CALI B NASHYD [CN=52.5 [ N = 3.0: Tp 0.17]	0101	1 2.0	5.92	0.30	11.83	18.81	0.23	0.000
* ** CALI B NASHYD [CN=59.7 [ N = 3.0: Tp 0.15]	0102	1 2.0	2.91	0.20	11.80	23.47	0.28	0.000

SCS PRE.txt  
\* ADD [ 0101+ 0102] 0201 3 2.0 8.83 0.50 11.80 20.34 n/a 0.000  
=====

V V I SSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL  
000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vi sual OTTHYMO 5.0\V02\vo in.dat  
Output filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\6e6bc  
7fb-557c-468c-a763-b8a630ebe11e\scen  
Summary filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\6e6bc  
7fb-557c-468c-a763-b8a630ebe11e\scen

DATE: 11/06/2019 TIME: 10: 44: 49

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 04 \*\*  
\*\*\*\*\*

W/E COMMAND	HYD ID	DT mi n	AREA ha	' Qpeak cms	Tpeak hrs	R. V. mm	R. C.	Qbase cms
START @ 0.00 hrs								
-----								
MASS STORM [ Ptot= 98.60 mm ]	15.0							
* ** CALI B NASHYD [CN=52.5 [ N = 3.0: Tp 0.17]	0101	1 2.0	5.92	0.41	11.83	25.75	0.26	0.000
* ** CALI B NASHYD [CN=59.7 [ N = 3.0: Tp 0.15]	0102	1 2.0	2.91	0.27	11.80	31.72	0.32	0.000
* ADD [ 0101+ 0102] 0201 3 2.0 8.83 0.68 11.80 27.72 n/a 0.000								

SCS PRE.txt

V V I SSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
O O T T H H Y Y M M O O  
O O T T H H Y Y M M O O  
000 T T H H Y M M 000

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\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vi sual OTTHYMO 5.0\VO2\voi n.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\de5e8  
f76-c456-442a-82c1-bb8559fc12cf\scen

Summary filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\de5e8  
f76-c456-442a-82c1-bb8559fc12cf\scen

DATE: 11/06/2019 TIME: 10: 44: 49

USER:

COMMENTS:

\*\*\*\*\*  
\*\* SIMULATION : Run 05  
\*\*\*\*\*

W/E COMMAND	HYD I D	DT mi n	AREA ha	' '	Qpeak cms	Tpeak hrs	R. V. mm	R. C.	Qbase cms
START @ 0.00 hrs									
-----									
MASS STORM [ Ptot=109.47 mm ]	15.0								
* ** CALI B NASHYD [CN=52.5 [ N = 3.0: Tp 0.17]	0101	1 2.0	5.92		0.50	11.83	31.22	0.29	0.000
* ** CALI B NASHYD [CN=59.7 [ N = 3.0: Tp 0.15]	0102	1 2.0	2.91		0.33	11.80	38.14	0.35	0.000
* ADD [ 0101+ 0102]	0201	3 2.0	8.83		0.83	11.80	33.50	n/a	0.000
*									

SCS PRE.txt

V V I SSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
O O T T H H Y Y M M O O  
O O T T H H Y Y M M O O  
000 T T H H Y M M 000

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\*\*\*\*\* SUMMARY OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vi sual OTTHYMO 5.0\VO2\voi n.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\6bd8f  
db0-be21-42f6-b312-abbedae6ba6\scen

Summary filename:  
C:\Users\vaschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\6bd8f  
db0-be21-42f6-b312-abbedae6ba6\scen

DATE: 11/06/2019 TIME: 10: 44: 49

USER:

COMMENTS:

\*\*\*\*\*  
\*\* SIMULATION : Run 06  
\*\*\*\*\*

W/E COMMAND	HYD I D	DT mi n	AREA ha	' '	Qpeak cms	Tpeak hrs	R. V. mm	R. C.	Qbase cms
START @ 0.00 hrs									
-----									
MASS STORM [ Ptot=120.44 mm ]	15.0								
* ** CALI B NASHYD [CN=52.5 [ N = 3.0: Tp 0.17]	0101	1 2.0	5.92		0.60	11.83	37.08	0.31	0.000
* ** CALI B NASHYD [CN=59.7 [ N = 3.0: Tp 0.15]	0102	1 2.0	2.91		0.39	11.80	44.96	0.37	0.000
* ADD [ 0101+ 0102]	0201	3 2.0	8.83		0.98	11.80	39.68	n/a	0.000
*									

## **Appendix B: Post-Development Hydrological Analysis**

Approved:



Design Storm - <b>OWEN SOUND MOE</b>			
<u>5 YEAR</u>	<u>Runoff Coefficients</u>	Project Name:	Camperdown Condominiums
A - 1234.576	Residential 0.55	Project Number:	117304
B - 8.297	Open Space 0.25	Municipality:	Town of The Blue Mountains
C - 0.851		Designed By:	AS
		Date:	March 2018
		Checked By:	
		Date:	March 2018
		Revised By:	AS
		Date:	October 2020
		Checked By:	

[illegible]

Site Area (Catchment 201)	=	22,000	sq.m	
Impervious Area	=	9,545	sq.m	(Asphalt, Driveway, House, Pond)
Pervious Area	=	12,455	sq.m	
Directly Connected Area	=	4,605	sq.m	(Asphalt, Driveway, House)

% Impervious	=	43.4
--------------	---	------

% Directly Connected	=	20.9
----------------------	---	------

Site Area (Catchment 205)	=	300	sq.m	
Impervious Area	=	168	sq.m	(Asphalt, Driveway, House)
Pervious Area	=	132	sq.m	
Directly Connected Area	=	84	sq.m	(Asphalt, Driveway, House)

% Impervious	=	56.0
--------------	---	------


% Directly Connected	=	28.0
----------------------	---	------

Site Area (Catchment 206)	=	7,300	sq.m	
Impervious Area	=	2,319	sq.m	(Asphalt, Driveway, House)
Pervious Area	=	4,981	sq.m	
Directly Connected Area	=	927	sq.m	(Asphalt, Driveway, House)

% Impervious	=	31.8
--------------	---	------

% Directly Connected	=	12.7
----------------------	---	------



	<b>Project:</b>	Camperdown Condominiums
	<b>File No.:</b>	117304
	<b>Date:</b>	June 2018
	<b>Designed By:</b>	AS
	<b>Checked By:</b>	RS
	<b>Subject:</b>	CN Calculator

# **CURVE NUMBER, INITIAL ABSTRACTION & TIME TO PEAK CALCULATIONS**

Catchment 202 Area 1.63 ha

WEIGHTED CN VALUE																									
Soil Series	Soil Series	Hydrologic Soil Group	Soil Texture	Runoff Coefficient Type	Catchment Soil Characteristics		Forest/Woodland			Pasture/Lawns			Meadows			Cultivated			Impervious			Wetland/Lakes/SWMF			Average CN for Soil Type
					Area	Percent	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	
WSL	WATERLOO	A	Sand Loam	1	1.22	0.75	1.22	1	32	0	49	0	38	0	62	0	100	0	50	32					
DUC	DUNEDIN	D	Clay Loam or Clay	3	0.41	0.25	0.29	0.7	79	0.12	0.3	84	0	81	0	86	0	100	0	50	80.5				
	#N/A	#N/A	#N/A	#N/A	0.00		0.00		#N/A	0.00		#N/A	0			#N/A	0		#N/A	0					
	#N/A	#N/A	#N/A	#N/A	0		0		#N/A	0		#N/A	0			#N/A	0		#N/A	0					
	#N/A	#N/A	#N/A	#N/A	0		0		#N/A	0		#N/A	0			#N/A	0		#N/A	0					
Totals					1.63	1.00	1.51	0.93		0.12	0.08	0	0	0	0	0	0	0	0	0	44.13				

## Time of Concentration Calculations

For Runoff Coefficients greater than 0.4

## Bransby-Williams Formula

Maximum Catchment Elevation 223 m  
Minimum Catchment Elevation 190.5 m  
Catchment length 200 m  
Catchment Slope 16%  
Catchment Area 1.63 ha

Time of Concentration (Minutes) 6.22  
Time of Concentration (Hours) 0.10  
Time to Peak (2/3 x Time of Concentration) 0.07

**Time to Peak** 0.17 hrs

For Runoff Coefficients less than 0.4

## Airport Method

Maximum Catchment Elevation 223 m  
Minimum Catchment Elevation 190.5 m  
Catchment length 200 m  
Catchment Slope 16%  
Catchment Area 1.63 ha


Time of Concentration (Minutes) 15.30  
Time of Concentration (Hours) 0.25  
Time to Peak (2/3 x Time of Concentration) 0.17

**Initial Abstraction** 9.625 mm

Wetlands	12
Woods	10
Meadows	8
Cultivated	7
Lawns	5
Impervious	2

**Runoff Coefficient** 0.27

Landuse Type	Soil Series				
	WSL	DUC	0	0	0
Forest/Woodland	0.18	0.52	#N/A	#N/A	#N/A
Cultivated	0.4	0.7	#N/A	#N/A	#N/A
Pasture/Lawn	0.22	0.55	#N/A	#N/A	#N/A
Impervious	0.95	0.95	#N/A	#N/A	#N/A
Wetland/Lake/SWMF	0.05	0.05	#N/A	#N/A	#N/A
Meadows	0.20	0.54	#N/A	#N/A	#N/A
Soil Series Total	0.18	0.529	#N/A	#N/A	#N/A

	<b>Project:</b>	Camperdown Condominiums
	<b>File No.:</b>	117304
	<b>Date:</b>	June 2018
	<b>Designed By:</b>	AS
	<b>Checked By:</b>	RS
	<b>Subject:</b>	CN Calculator

**CURVE NUMBER, INITIAL ABSTRACTION & TIME TO PEAK CALCULATIONS**

Catchment 203 Area 2.89 ha

WEIGHTED CN VALUE																										
Soil Series	Soil Series	Hydrologic Soil Group	Soil Texture	Runoff Coefficient Type	Catchment Soil Characteristics		Forest/Woodland			Pasture/Lawns			Meadows			Cultivated			Impervious			Wetland/Lakes/SWMF			Average CN for Soil Type	
					Area	Percent	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN		
TS	TECUMSETH	AB	Sand Loam	1	0.98	0.34	0.98	1	46	0			59	0		51	0		68	0		100	0		50	46
WSL	WATERLOO	A	Sand Loam	1	1.91	0.66	1.43	0.75	32	0.29	0.15		49	0		38	0		62	0.1907	0.1	100	0		50	41.35
	#N/A	#N/A	#N/A	#N/A	0.00				#N/A				#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0
	#N/A	#N/A	#N/A	#N/A	0		0		#N/A	0			#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0
	#N/A	#N/A	#N/A	#N/A	0		0		#N/A	0			#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0
Totals					2.89	1.00	2.41	0.84		0.29	0.10		0	0		0	0		0	0		0.19074	0.066	0	0	42.93

**Time of Concentration Calculations**

For Runoff Coefficients greater than 0.4

**Bransby-Williams Formula**

Maximum Catchment Elevation 223 m  
Minimum Catchment Elevation 190.5 m  
Catchment length 200 m  
Catchment Slope 16%  
Catchment Area 2.89 ha

Time of Concentration (Minutes) 5.87  
Time of Concentration (Hours) 0.10  
Time to Peak (2/3 x Time of Concentration) 0.07

<b>Time to Peak</b>	<b>0.18 hrs</b>
---------------------	-----------------

For Runoff Coefficients less than 0.4

**Airport Method**

Maximum Catchment Elevation 223 m  
Minimum Catchment Elevation 190.5 m  
Catchment length 200 m  
Catchment Slope 16%  
Catchment Area 2.89 ha


Time of Concentration (Minutes) 15.90  
Time of Concentration (Hours) 0.26  
Time to Peak (2/3 x Time of Concentration) 0.18

<b>Initial Abstraction</b>	<b>8.977 mm</b>
----------------------------	-----------------

Wetlands	12
Woods	10
Meadows	8
Cultivated	7
Lawns	5
Impervious	2

<b>Runoff Coefficient</b>	<b>0.23</b>
---------------------------	-------------

Landuse Type	Soil Series				
	TS	WSL	0	0	0
Forest/Woodland	0.18	0.18	#N/A	#N/A	#N/A
Cultivated	0.4	0.4	#N/A	#N/A	#N/A
Pasture/Lawn	0.22	0.22	#N/A	#N/A	#N/A
Impervious	0.95	0.95	#N/A	#N/A	#N/A
Wetland/Lake/SWMF	0.05	0.05	#N/A	#N/A	#N/A
Meadows	0.20	0.20	#N/A	#N/A	#N/A
Soil Series Total	0.18	0.263	#N/A	#N/A	#N/A

	<b>Project:</b>	Camperdown Condominiums
	<b>File No.:</b>	117304
	<b>Date:</b>	June 2018
	<b>Designed By:</b>	AS
	<b>Checked By:</b>	RS
<b>Subject:</b>		CN Calculator

**CURVE NUMBER, INITIAL ABSTRACTION & TIME TO PEAK CALCULATIONS**

Catchment 204 Area 0.64 ha

WEIGHTED CN VALUE																			
Soil Series	Soil Series	Hydrologic Soil Group	Soil Texture	Runoff Coefficient Type	Catchment Soil Characteristics			Forest/Woodland			Pasture/Lawns			Meadows			Cultivated		
					Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN
TS	TECUMSETH	AB	Sand Loam	1	0.64	1.00	0.54	0.84	46	0.1024	0.16	59	0	51	0	68	0	100	50
	#N/A	#N/A	#N/A	#N/A	0.00		0.00	#N/A	0.00	#N/A	0	#N/A	0	#N/A	0	0.1	#N/A	0	#N/A
	#N/A	#N/A	#N/A	#N/A	0.00			#N/A		#N/A	0	#N/A	0	#N/A	0		#N/A	0	#N/A
	#N/A	#N/A	#N/A	#N/A	0		0	#N/A	0	#N/A	0	#N/A	0	#N/A	0		#N/A	0	#N/A
	#N/A	#N/A	#N/A	#N/A	0		0	#N/A	0	#N/A	0	#N/A	0	#N/A	0		#N/A	0	#N/A
Totals					0.64	1.00	0.54	0.84		0.10	0.16		0	0	0	0	0	0	48.08

**Time of Concentration Calculations**

For Runoff Coefficients greater than 0.4

Bransby-Williams Formula

Maximum Catchment Elevation 192 m  
 Minimum Catchment Elevation 190 m  
 Catchment length 115 m  
 Catchment Slope 2%  
 Catchment Area 0.64 ha

Time of Concentration (Minutes) 6.14  
 Time of Concentration (Hours) 0.10  
 Time to Peak (2/3 x Time of Concentration) 0.07

**Time to Peak** 0.33 hrs

For Runoff Coefficients less than 0.4

Airport Method

Maximum Catchment Elevation 192 m  
 Minimum Catchment Elevation 190 m  
 Catchment length 115 m  
 Catchment Slope 2%  
 Catchment Area 0.64 ha


Time of Concentration (Minutes) 29.61  
 Time of Concentration (Hours) 0.49  
 Time to Peak (2/3 x Time of Concentration) 0.33

**Initial Abstraction** 9.2 mm

**Runoff Coefficient** 0.08

Wetlands	12
Woods	10
Meadows	8
Cultivated	7
Lawns	5
Impervious	2

Landuse Type	Soil Series				
	TS	0	0	0	0
Forest/Woodland	0.08	#N/A	#N/A	#N/A	#N/A
Cultivated	0.22	#N/A	#N/A	#N/A	#N/A
Pasture/Lawn	0.1	#N/A	#N/A	#N/A	#N/A
Impervious	0.95	#N/A	#N/A	#N/A	#N/A
Wetland/Lake/SWMF	0.05	#N/A	#N/A	#N/A	#N/A
Meadows	0.09	#N/A	#N/A	#N/A	#N/A
Soil Series Total	0.0832	#N/A	#N/A	#N/A	#N/A

	<b>Project:</b>	Camperdown Condominiums
	<b>File No.:</b>	117304
	<b>Date:</b>	March 2018
	<b>Designed By:</b>	AS
	<b>Checked By:</b>	RS
	<b>Subject:</b>	CN Calculator

**CURVE NUMBER, INITIAL ABSTRACTION & TIME TO PEAK CALCULATIONS**

Catchment 207 Area 0.78 ha

WEIGHTED CN VALUE																									
Soil Series	Soil Series	Hydrologic Soil Group	Soil Texture	Runoff Coefficient Type	Catchment Soil Characteristics		Forest/Woodland			Pasture/Lawns			Meadows			Cultivated			Impervious			Wetland/Lakes/SWMF			Average CN for Soil Type
					Area	Percent	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	Area	Percent	CN	
TS	TECUMSETH	AB	Sand Loam	1	0.78	1.00	0.08	0.1	46	0.538	0.69	59	0		51	0		68	0.164	0.21	100	0		50	66.31
	#N/A	#N/A	#N/A	#N/A	0.00		0.00		#N/A	0.00		#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0
	#N/A	#N/A	#N/A	#N/A	0.00				#N/A			#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0
	#N/A	#N/A	#N/A	#N/A	0		0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0
	#N/A	#N/A	#N/A	#N/A	0		0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0		#N/A	0
Totals					0.78	1.00	0.08	0.10		0.54	0.69		0	0		0	0		0.1638	0.21		0	0		66.31

**Time of Concentration Calculations**

For Runoff Coefficients greater than 0.4

**Bransby-Williams Formula**

Maximum Catchment Elevation 191 m  
 Minimum Catchment Elevation 187 m  
 Catchment length 100 m  
 Catchment Slope 4%  
 Catchment Area 0.78 ha

Time of Concentration (Minutes) 4.43  
 Time of Concentration (Hours) 0.07  
 Time to Peak (2/3 x Time of Concentration) 0.05

**Time to Peak** 0.19 hrs

For Runoff Coefficients less than 0.4

**Airport Method**

Maximum Catchment Elevation 191 m  
 Minimum Catchment Elevation 187 m  
 Catchment length 100 m  
 Catchment Slope 4%  
 Catchment Area 0.78 ha

Time of Concentration (Minutes) 16.99  
 Time of Concentration (Hours) 0.28  
 Time to Peak (2/3 x Time of Concentration) 0.19

**Initial Abstraction** 4.87 mm

Wetlands	12
Woods	10
Meadows	8
Cultivated	7
Lawns	5
Impervious	2

**Runoff Coefficient** 0.28

Landuse Type	Soil Series				
	TS	0	0	0	0
Forest/Woodland	0.08	#N/A	#N/A	#N/A	#N/A
Cultivated	0.22	#N/A	#N/A	#N/A	#N/A
Pasture/Lawn	0.1	#N/A	#N/A	#N/A	#N/A
Impervious	0.95	#N/A	#N/A	#N/A	#N/A
Wetland/Lake/SWMF	0.05	#N/A	#N/A	#N/A	#N/A
Meadows	0.09	#N/A	#N/A	#N/A	#N/A
Soil Series Total	0.277	#N/A	#N/A	#N/A	#N/A

Camperdown Condominiums  
SWM Pond Volume Table

Designed: AS  
Checked: RS  
Date: Nov 2019

Wet Pond Characteristics:

Side Slope: 5 :1  
Top Elevation: 190.80 m  
Bottom Elev: 188.50 m  
Permanent Pool: 189.50 m  
Stage 0.1 m

Stormwater Management Pond								
Pond Geometry				Pond Volume (m <sup>3</sup> )				
Elevation (m)	Depth (m)	Area (m <sup>2</sup> )	Avg. Area (m)	Dead	Accum. Dead	Live	Accum. Live	Accum. Total
188.50	0.00	5.00	5.00	0.00	0.00		0.00	0.00
188.60	0.10	17.00	11.00	1.10	1.10		0.00	1.10
188.70	0.20	31.00	24.00	2.40	3.50		0.00	3.50
188.80	0.30	49.00	40.00	4.00	7.50		0.00	7.50
188.90	0.40	78.00	63.50	6.35	13.85		0.00	13.85
189.00	0.50	110.00	94.00	9.40	23.25		0.00	23.25
189.10	0.60	154.00	132.00	13.20	36.45		0.00	36.45
189.20	0.70	191.00	172.50	17.25	53.70		0.00	53.70
189.30	0.80	238.00	214.50	21.45	75.15		0.00	75.15
189.40	0.90	283.00	260.50	26.05	101.20		0.00	101.20
189.50	1.00	338.00	310.50	31.05	132.25	0.00	0.00	132.25
189.60	1.10	374.00	356.00		132.25	35.60	35.60	167.85
189.70	1.20	429.00	401.50		132.25	40.15	75.75	208.00
189.80	1.30	467.00	448.00		132.25	44.80	120.55	252.80
189.90	1.40	523.00	495.00		132.25	49.50	170.05	302.30
190.00	1.50	578.00	550.50		132.25	55.05	225.10	357.35
190.10	1.60	635.00	606.50		132.25	60.65	285.75	418.00
190.20	1.70	697.00	666.00		132.25	66.60	352.35	484.60
190.30	1.80	761.00	729.00		132.25	72.90	425.25	557.50
190.40	1.90	827.00	794.00		132.25	79.40	504.65	636.90
190.50	2.00	894.00	860.50		132.25	86.05	590.70	722.95
190.60	2.10	963.00	928.50		132.25	92.85	683.55	815.80
190.70	2.20	1034.00	998.50		132.25	99.85	783.40	915.65
190.80	2.30	1106.00	1070.00		132.25	107.00	890.40	1022.65

Camperdown Condominiums  
SWM Pond Volume Table

Designed: AS  
Checked: RS  
Date: Nov 2019

Pond Discharge Table:

<b>Orifice #1:</b>		<b>Orifice #2:</b>		<b>Overflow Weir:</b>	
Diameter:	300	Diameter:	0 mm	Bottom Length:	3.5 m
Area:	0.0707	Area:	0.0000 m <sup>2</sup>	Sill Elevation:	190.4 m
C:	0.63	C:	0.63	D/S Weir Length:	10 m
Invert:	189.50	Invert:	m	Side Slopes (H:V)	5 :1

Elevation (m)	Orifice #1		Orifice #2		Overflow Weir		Hydraulic Control	Discharge (m <sup>3</sup> /s)
	Head (m)	Discharge (m)	Head (m)	Discharge (m)	Head (m)	Discharge (m)		
188.50	0.000	0.000	188.500	0.000	0	0	Orifice	0.000
188.60	0.000	0.000	188.600	0.000	0	0	Orifice	0.000
188.70	0.000	0.000	188.700	0.000	0	0	Orifice	0.000
188.80	0.000	0.000	188.800	0.000	0	0	Orifice	0.000
188.90	0.000	0.000	188.900	0.000	0	0	Orifice	0.000
189.00	0.000	0.000	189.000	0.000	0	0	Orifice	0.000
189.10	0.000	0.000	189.100	0.000	0	0	Orifice	0.000
189.20	0.000	0.000	189.200	0.000	0	0	Orifice	0.000
189.30	0.000	0.000	189.300	0.000	0	0	Orifice	0.000
189.40	0.000	0.000	189.400	0.000	0	0	Orifice	0.000
189.50	0.000	0.000	189.500	0.000	0	0	Orifice	0.000
189.60	0.000	0.000	189.600	0.000	0	0	Orifice	0.000
189.70	0.050	0.044	189.700	0.000	0	0	Orifice	0.044
189.80	0.150	0.076	189.800	0.000	0	0	Orifice	0.076
189.90	0.250	0.099	189.900	0.000	0	0	Orifice	0.099
190.00	0.350	0.117	190.000	0.000	0	0	Orifice	0.117
190.10	0.450	0.132	190.100	0.000	0	0	Orifice	0.132
190.20	0.550	0.146	190.200	0.000	0	0	Orifice	0.146
190.30	0.650	0.159	190.300	0.000	0	0	Orifice	0.159
190.40	0.750	0.171	190.400	0.000	0	0	Orifice	0.171
190.50	0.850	0.182	190.500	0.000	0.1	0.17	Orifice	0.352
190.60	0.950	0.192	190.600	0.000	0.20	0.53	Orifice/Weir	0.722
190.70	1.050	0.202	190.700	0.000	0.30	1.09	Orifice/Weir	1.292
190.80	1.150	0.212	190.800	0.000	0.40	1.85	Orifice/Weir	2.062

Comments:

- 1 0.15 - Calculation based on preferred NVCA weir flow spreadsheet
- 2 N/A - Not Applicable
- 3 Orifice Equation is:

$$Q = C \times A \times (2gH)^{0.5}$$

Where:

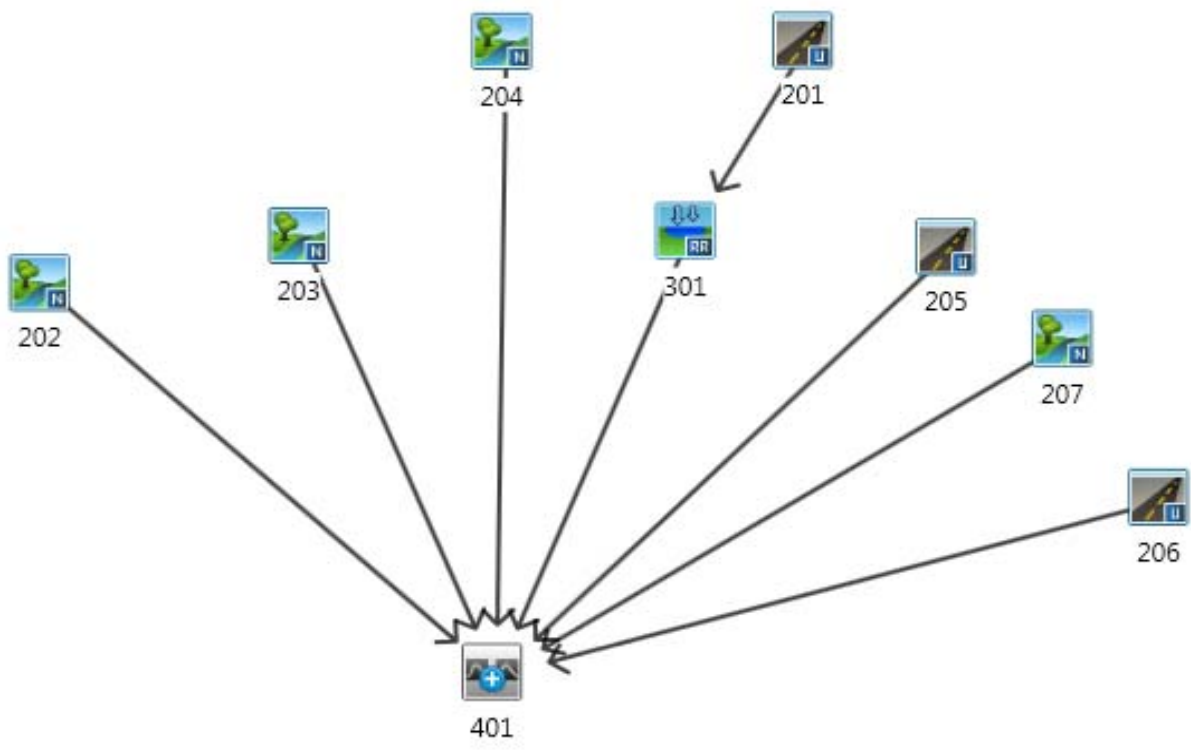
- Q = flow rate (cms)
- C = constant
- A = area of opening(sq. m)
- H = net head on the orifice
- g = Acceleration due to gravity

Camperdown Condominiums  
Stage-Storage-Discharge

Designed: AS  
Checked: RS  
Date: Nov 2019

Stormwater Management Pond								
Pond Geometry				Pond Volume (m <sup>3</sup> )				Discharge (m <sup>3</sup> /s)
Elevation (m)	Depth (m)	Area (m <sup>2</sup> )	Avg. Area (m)	Dead	Accum. Dead	Live	Accum. Live	
188.50	0.00	5.00	5.00	0.00	0.00	0.00	0.00	0.000
188.60	0.10	17.00	11.00	1.10	1.10	0.00	0.00	0.000
188.70	0.20	31.00	24.00	2.40	3.50	0.00	0.00	0.000
188.80	0.30	49.00	40.00	4.00	7.50	0.00	0.00	0.000
188.90	0.40	78.00	63.50	6.35	13.85	0.00	0.00	0.000
189.00	0.50	110.00	94.00	9.40	23.25	0.00	0.00	0.000
189.10	0.60	154.00	132.00	13.20	36.45	0.00	0.00	0.000
189.20	0.70	191.00	172.50	17.25	53.70	0.00	0.00	0.000
189.30	0.80	238.00	214.50	21.45	75.15	0.00	0.00	0.000
189.40	0.90	283.00	260.50	26.05	101.20	0.00	0.00	0.000
189.50	1.00	338.00	310.50	31.05	132.25	0.00	0.00	0.000
189.60	1.10	374.00	356.00	0.00	132.25	35.60	35.60	0.000
189.70	1.20	429.00	401.50	0.00	132.25	40.15	75.75	0.044
189.80	1.30	467.00	448.00	0.00	132.25	44.80	120.55	0.076
189.90	1.40	523.00	495.00	0.00	132.25	49.50	170.05	0.099
190.00	1.50	578.00	550.50	0.00	132.25	55.05	225.10	0.117
190.10	1.60	635.00	606.50	0.00	132.25	60.65	285.75	0.132
190.20	1.70	697.00	666.00	0.00	132.25	66.60	352.35	0.146
190.30	1.80	761.00	729.00	0.00	132.25	72.90	425.25	0.159
190.40	1.90	827.00	794.00	0.00	132.25	79.40	504.65	0.171
190.50	2.00	894.00	860.50	0.00	132.25	86.05	590.70	0.352
190.60	2.10	963.00	928.50	0.00	132.25	92.85	683.55	0.722
190.70	2.20	1034.00	998.50	0.00	132.25	99.85	783.40	1.292
190.80	2.30	1106.00	1070.00	0.00	132.25	107.00	890.40	2.062

**CAMPERDOWN CONDOMINIUM**  
**PROPOSED CONDITIONS**



Nashyd



Standhyd



Addhyd



Route Pipe



Route Channel



Route Reservoir



Duhyd



Diverthyd



**Project:** Camperdown Condominium

**File No.:** 117304

**Subject:** Otthymo Flow Schematic

**Date:** Nov 2019

**Figure:** 1



# CHI CAGO.txt

=====

```
V  V  I  SSSSS  U  U  A  L
V  V  I  SS  U  U  A  A  L
V  V  I  SS  U  U  AAAAA  L
V  V  I  SS  U  U  A  A  L
VV  I  SSSSS  UUUUU  A  A  LLLLL
```

```
000  TTTT  TTTT  H  H  Y  Y  M  M  000  TM
0  0  T  T  H  H  Y  Y  MM  MM  0  0
0  0  T  T  H  H  Y  Y  M  M  0  0
000  T  T  H  H  Y  Y  M  M  000
```

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## \*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYM0 5.0\VO2\vo in.dat

Output filename:  
C:\Users\aschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\faa30  
3c6-b5d2-4c09-926e-f7cc23e787ce\scen  
Summary filename:  
C:\Users\aschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\faa30  
3c6-b5d2-4c09-926e-f7cc23e787ce\scen

DATE: 10/09/2020

TIME: 05:30:25

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 01  
\*\*\*\*\*

-----  
READ STORM  
Ptotal = 24.97 mm  
-----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.10	1.29	1.10	2.81	2.10	13.05	3.10	2.04
0.20	1.36	1.20	3.22	2.20	8.44	3.20	1.89
0.30	1.44	1.30	3.77	2.30	6.21	3.30	1.76
0.40	1.53	1.40	4.55	2.40	4.91	3.40	1.65
0.50	1.63	1.50	5.77	2.50	4.06	3.50	1.55
0.60	1.75	1.60	7.86	2.60	3.47	3.60	1.46
0.70	1.89	1.70	12.27	2.70	3.03	3.70	1.39
0.80	2.06	1.80	26.17	2.80	2.70	3.80	1.32

Page 1

# CHI CAGO.txt

0.90	2.26	1.90	72.58	2.90	2.43	3.90	1.26
1.00	2.50	2.00	26.96	3.00	2.22	4.00	1.20

-----  
CALIB  
NASHYD ( 0203)  
ID= 1 DT= 2.0 min  
-----

Area (ha)= 2.89  
Curve Number (CN)= 42.9  
# of Linear Res. (N)= 3.00  
Ia (mm)= 8.98  
U.H. Tp(hrs)= 0.18

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.033	1.29	1.033	2.81	2.033	13.05	3.03	2.04
0.067	1.29	1.067	2.81	2.067	13.05	3.07	2.04
0.100	1.29	1.100	2.81	2.100	13.05	3.10	2.04
0.133	1.36	1.133	3.22	2.133	8.44	3.13	1.89
0.167	1.36	1.167	3.22	2.167	8.44	3.17	1.89
0.200	1.36	1.200	3.22	2.200	8.44	3.20	1.89
0.233	1.44	1.233	3.77	2.233	6.21	3.23	1.76
0.267	1.44	1.267	3.77	2.267	6.21	3.27	1.76
0.300	1.44	1.300	3.77	2.300	6.21	3.30	1.76
0.333	1.53	1.333	4.55	2.333	4.91	3.33	1.65
0.367	1.53	1.367	4.55	2.367	4.91	3.37	1.65
0.400	1.53	1.400	4.55	2.400	4.91	3.40	1.65
0.433	1.63	1.433	5.77	2.433	4.06	3.43	1.55
0.467	1.63	1.467	5.77	2.467	4.06	3.47	1.55
0.500	1.63	1.500	5.77	2.500	4.06	3.50	1.55
0.533	1.75	1.533	7.86	2.533	3.47	3.53	1.46
0.567	1.75	1.567	7.86	2.567	3.47	3.57	1.46
0.600	1.75	1.600	7.86	2.600	3.47	3.60	1.46
0.633	1.89	1.633	12.27	2.633	3.03	3.63	1.39
0.667	1.89	1.667	12.27	2.667	3.03	3.67	1.39
0.700	1.89	1.700	12.27	2.700	3.03	3.70	1.39
0.733	2.06	1.733	26.17	2.733	2.70	3.73	1.32
0.767	2.06	1.767	26.17	2.767	2.70	3.77	1.32
0.800	2.06	1.800	26.17	2.800	2.70	3.80	1.32
0.833	2.26	1.833	72.58	2.833	2.43	3.83	1.26
0.867	2.26	1.867	72.58	2.867	2.43	3.87	1.26
0.900	2.26	1.900	72.58	2.900	2.43	3.90	1.26
0.933	2.50	1.933	26.96	2.933	2.22	3.93	1.20
0.967	2.50	1.967	26.96	2.967	2.22	3.97	1.20
1.000	2.50	2.000	26.96	3.000	2.22	4.00	1.20

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.007 (i)  
TIME TO PEAK (hrs)= 2.167  
RUNOFF VOLUME (mm)= 0.723  
TOTAL RAINFALL (mm)= 24.971  
RUNOFF COEFFICIENT = 0.029

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
CALIB  
NASHYD ( 0202)  
ID= 1 DT= 2.0 min  
-----

Area (ha)= 1.63  
Curve Number (CN)= 44.1  
# of Linear Res. (N)= 3.00  
Ia (mm)= 9.62  
Page 2

----- CHICAGO.txt  
U. H. Tp(hrs)= 0.17

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.004 (i)  
TIME TO PEAK (hrs)= 2.167  
RUNOFF VOLUME (mm)= 0.699  
TOTAL RAINFALL (mm)= 24.971  
RUNOFF COEFFICIENT = 0.028

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
ADD HYD ( 0053)  
1 + 2 = 3  
-----  
ID1= 1 ( 0202): AREA (ha)= 1.63 QPEAK (cms)= 0.004 TPEAK (hrs)= 2.17 R. V. (mm)= 0.70  
+ ID2= 2 ( 0203): AREA (ha)= 2.89 QPEAK (cms)= 0.007 TPEAK (hrs)= 2.17 R. V. (mm)= 0.72  
===== ID = 3 ( 0053): AREA (ha)= 4.52 QPEAK (cms)= 0.010 TPEAK (hrs)= 2.17 R. V. (mm)= 0.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
CALIB  
NASHYD ( 0204) Area (ha)= 0.64 Curve Number (CN)= 48.1  
ID= 1 DT= 2.0 min Ia (mm)= 9.20 # of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.33

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.001 (i)  
TIME TO PEAK (hrs)= 2.367  
RUNOFF VOLUME (mm)= 0.856  
TOTAL RAINFALL (mm)= 24.971  
RUNOFF COEFFICIENT = 0.034

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
ADD HYD ( 0054)  
1 + 2 = 3  
-----  
ID1= 1 ( 0204): AREA (ha)= 0.64 QPEAK (cms)= 0.001 TPEAK (hrs)= 2.37 R. V. (mm)= 0.86  
+ ID2= 2 ( 0053): AREA (ha)= 4.52 QPEAK (cms)= 0.010 TPEAK (hrs)= 2.17 R. V. (mm)= 0.71  
===== ID = 3 ( 0054): AREA (ha)= 5.16 QPEAK (cms)= 0.011 TPEAK (hrs)= 2.17 R. V. (mm)= 0.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
CALIB  
STANDHYD ( 0201) Area (ha)= 2.20  
ID= 1 DT= 5.0 min Total Imp(%)= 43.00 Dir. Conn. (%)= 21.00

Surface Area (ha)= IMPERVIOUS 0.95 PERVIOUS (i) 1.25

CHICAGO.txt  
Dep. Storage (mm)= 1.00 1.50  
Average Slope (%)= 1.00 1.00  
Length (m)= 121.11 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.083 1.29 1.083 2.81 2.083 13.05 3.08 2.04  
0.167 1.35 1.167 3.14 2.167 9.36 3.17 1.92  
0.250 1.41 1.250 3.55 2.250 7.10 3.25 1.81  
0.333 1.48 1.333 4.08 2.333 5.69 3.33 1.72  
0.417 1.55 1.417 4.79 2.417 4.74 3.42 1.63  
0.500 1.63 1.500 5.77 2.500 4.06 3.50 1.55  
0.583 1.75 1.583 7.86 2.583 3.47 3.58 1.46  
0.667 1.86 1.667 11.39 2.667 3.12 3.67 1.40  
0.750 1.99 1.750 20.61 2.750 2.83 3.75 1.35  
0.833 2.14 1.833 44.73 2.833 2.59 3.83 1.30  
0.917 2.31 1.917 63.46 2.917 2.39 3.92 1.25  
1.000 2.50 2.000 26.96 3.000 2.22 4.00 1.20

Max. Eff. Inten. (mm/hr)= 63.46 28.18  
over (min)= 5.00 20.00  
Storage Coeff. (min)= 3.44 (ii) 17.86 (ii)  
Unit Hyd. Tpeak (min)= 5.00 20.00  
Unit Hyd. peak (cms)= 0.26 0.06

PEAK FLOW (cms)= 0.07 0.05 \*TOTALS\*  
TIME TO PEAK (hrs)= 1.92 2.17 0.087 (iii)  
RUNOFF VOLUME (mm)= 23.97 5.08 9.05  
TOTAL RAINFALL (mm)= 24.97 24.97 24.97  
RUNOFF COEFFICIENT = 0.96 0.20 0.36

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:

Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
RESERVOIR( 0301)  
IN= 2---> OUT= 1  
DT= 5.0 min  
-----  
OUTFLOW STORAGE OUTFLOW STORAGE  
(cms) (ha. m.) (cms) (ha. m.)  
0.0000 0.1680 0.1590 0.5570  
0.0440 0.2080 0.3520 0.7230  
0.0990 0.3020 1.2920 0.9160  
0.1320 0.4180 2.0620 1.0230

AREA OPEAK TPEAK R. V.  
(ha) (cms) (hrs) (mm)  
INFLOW : ID= 2 ( 0201) 2.200 0.087 1.92 9.05  
OUTFLOW: ID= 1 ( 0301) 2.200 0.000 0.00 0.00

PEAK FLOW REDUCTION [Qout/Qin](%)= 0.00  
TIME SHIFT OF PEAK FLOW (min)=\*\*\*\*\*

CHI CAGO. txt  
MAXIMUM STORAGE USED (ha. m.) = 0.0041

ADD HYD ( 0055 )				
1	2	3		
ID1= 1 ( 0301):	2.20	0.000	0.00	0.00
+ ID2= 2 ( 0054):	5.16	0.011	2.17	0.73
=====				
ID = 3 ( 0055):	7.36	0.011	2.17	0.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0205 )			
ID= 1 DT= 5.0 min	Area Total	(ha)= 0.03 Imp(%)= 56.00	Dir. Conn. (%)= 28.00

IMPERVIOUS		
PERVIOUS (i)		
Surface Area (ha)=	0.02	0.01
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	1.00
Length (m)=	14.14	20.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	63.46	52.08
over (min)=	5.00	10.00
Storage Coeff. (min)=	0.95 (ii)	8.39 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.34	0.12

*TOTALS*		
PEAK FLOW (cms)=	0.00	0.00
TIME TO PEAK (hrs)=	1.92	2.00
RUNOFF VOLUME (mm)=	23.97	6.40
TOTAL RAINFALL (mm)=	24.97	24.97
RUNOFF COEFFICIENT =	0.96	0.26
		0.42

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0056 )				
1	2	3		
ID1= 1 ( 0205):	0.03	0.002	1.92	10.55
+ ID2= 2 ( 0055):	7.36	0.011	2.17	0.51
=====				
ID = 3 ( 0056):	7.39	0.012	2.13	0.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO. txt

CALIB NASHYD ( 0207 )			
ID= 1 DT= 5.0 min	Area Total	(ha)= 0.78 (mm)= 4.87	Curve Number (CN)= 66.3 # of Linear Res. (N)= 3.00
	U. H. Tp	(hrs)= 0.19	

Unit Hyd Qpeak (cms)= 0.157

PEAK FLOW (cms)=	0.008 (i)
TIME TO PEAK (hrs)=	2.083
RUNOFF VOLUME (mm)=	2.701
TOTAL RAINFALL (mm)=	24.971
RUNOFF COEFFICIENT =	0.108

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057 )				
1	2	3		
ID1= 1 ( 0207):	0.78	0.008	2.08	2.70
+ ID2= 2 ( 0056):	7.39	0.012	2.13	0.55
=====				
ID = 3 ( 0057):	8.17	0.020	2.13	0.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0206 )			
ID= 1 DT= 5.0 min	Area Total	(ha)= 0.73 Imp(%)= 32.00	Dir. Conn. (%)= 13.00

IMPERVIOUS		
PERVIOUS (i)		
Surface Area (ha)=	0.23	0.50
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	69.76	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	63.46	22.44
over (min)=	5.00	20.00
Storage Coeff. (min)=	2.47 (ii)	15.30 (ii)
Unit Hyd. Tpeak (min)=	5.00	20.00
Unit Hyd. peak (cms)=	0.29	0.07

PEAK FLOW (cms)=	0.02	0.02	*TOTALS*
TIME TO PEAK (hrs)=	1.92	2.17	0.020 (iii)
RUNOFF VOLUME (mm)=	23.97	4.39	1.92
TOTAL RAINFALL (mm)=	24.97	24.97	6.93
RUNOFF COEFFICIENT =	0.96	0.18	24.97
			0.28

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.

CHI CAGO. txt

ADD	HYD	(	0401)
1	+	2	= 3

FINISH

```

V      V      I      SSSSS  U      U      A      L
V      V      I      SS     U      U      A A     L
  V      V      I      SS     U      U      AAAAA  L
  V      V      I      SS     U      U      A A A   L
    WW      I      SSSSS  UUUUU  A      A  LLLLL

```

000	TTTT	TTTT	H	H	Y	Y	M	M	000	TM
0 0	T	T	H	H	Y	Y	MM	MM	0 0	
0 0	T	T	H	H	Y	Y	M	M	0 0	
000	T	T	H	H	Y	Y	M	M	000	

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\*\*\*\*\* D E T A I L E D   O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\V02\vojn.dat

```
Output filename:
C:\Users\aschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\9d8a2
525-1597-4cef-bbf6-aa461870bb58\scen
Summary filename:
C:\Users\aschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\9d8a2
525-1597-4cef-bbf6-aa461870bb58\scen
```

DATE: 10/09/2020 TIME: 05:30:25

USER:

COMMENTS: \_\_\_\_\_

Page 7

CHI CAGO. txt

```
*****
** SIMULATION : Run 02                      **
*****
```

CHI CAGO STORM  
Ptotal = 31.36 mm

IDF curve parameters: A= 362.158  
B= 0.000  
C= 0.699  
used in: INTENSITY = A / (t + B)^C

```
Duration of storm = 4.00 hrs
Storm time step   = 15.00 min
Time to peak ratio = 0.33
```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.99	1.25	54.55	2.25	4.58	3.25	2.85
0.50	3.70	1.50	11.55	2.50	3.93	3.50	2.63
0.75	5.06	1.75	7.28	2.75	3.47	3.75	2.45
1.00	9.41	2.00	5.55	3.00	3.13	4.00	2.29

CALIB  
NASHYD ( 0203)  
ID= 1 DT= 2.0 min

Area (ha)= 2.89      Curve Number (CN)= 42.9  
 Ia (mm)= 8.98      # of Linear Res. (N)= 3.00  
 U. H. Tp(hrs)= 0.18

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

--- TRANSFORMED				HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.033	2.99	1.033	54.55	2.033	4.58	3.03	2.85
0.067	2.99	1.067	54.55	2.067	4.58	3.07	2.85
0.100	2.99	1.100	54.55	2.100	4.58	3.10	2.85
0.133	2.99	1.133	54.55	2.133	4.58	3.13	2.85
0.167	2.99	1.167	54.55	2.167	4.58	3.17	2.85
0.200	2.99	1.200	54.55	2.200	4.58	3.20	2.85
0.233	2.99	1.233	54.55	2.233	4.58	3.23	2.85
0.267	3.34	1.267	33.05	2.267	4.25	3.27	2.74
0.300	3.70	1.300	11.55	2.300	3.93	3.30	2.63
0.333	3.70	1.333	11.55	2.333	3.93	3.33	2.63
0.367	3.70	1.367	11.55	2.367	3.93	3.37	2.63
0.400	3.70	1.400	11.55	2.400	3.93	3.40	2.63
0.433	3.70	1.433	11.55	2.433	3.93	3.43	2.63
0.467	3.70	1.467	11.55	2.467	3.93	3.47	2.63
0.500	3.70	1.500	11.55	2.500	3.93	3.50	2.63
0.533	5.06	1.533	7.28	2.533	3.47	3.53	2.45
0.567	5.06	1.567	7.28	2.567	3.47	3.57	2.45
0.600	5.06	1.600	7.28	2.600	3.47	3.60	2.45
0.633	5.06	1.633	7.28	2.633	3.47	3.63	2.45
0.667	5.06	1.667	7.28	2.667	3.47	3.67	2.45
0.700	5.06	1.700	7.28	2.700	3.47	3.70	2.45
0.733	5.06	1.733	7.28	2.733	3.47	3.73	2.45
0.767	7.24	1.767	6.42	2.767	3.30	3.77	2.37
0.800	9.41	1.800	5.55	2.800	3.13	3.80	2.29
0.833	9.41	1.833	5.55	2.833	3.13	3.83	2.29
0.867	9.41	1.867	5.55	2.867	3.13	3.87	2.29
0.900	9.41	1.900	5.55	2.900	3.13	3.90	2.29
0.933	9.41	1.933	5.55	2.933	3.13	3.93	2.29

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CHI CAGO. txt  
 0.967 9.41 1.967 5.55 2.967 3.13 3.97 2.29  
 1.000 9.41 2.000 5.55 3.000 3.13 4.00 2.29

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.008 (i)  
 TIME TO PEAK (hrs)= 1.433  
 RUNOFF VOLUME (mm)= 1.391  
 TOTAL RAINFALL (mm)= 31.358  
 RUNOFF COEFFICIENT = 0.044

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB  
 NASHYD ( 0202)  
 ID= 1 DT= 2.0 min  
 Area (ha)= 1.63 Curve Number (CN)= 44.1  
 la (mm)= 9.62 # of Linear Res. (N)= 3.00  
 U. H. Tp(hrs)= 0.17

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.004 (i)  
 TIME TO PEAK (hrs)= 1.433  
 RUNOFF VOLUME (mm)= 1.375  
 TOTAL RAINFALL (mm)= 31.358  
 RUNOFF COEFFICIENT = 0.044

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0053)  
 1 + 2 = 3  
 AREA QPEAK TPEAK R. V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0202): 1.63 0.004 1.43 1.38  
 + ID2= 2 ( 0203): 2.89 0.008 1.43 1.39  
 ID = 3 ( 0053): 4.52 0.012 1.43 1.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 NASHYD ( 0204)  
 ID= 1 DT= 2.0 min  
 Area (ha)= 0.64 Curve Number (CN)= 48.1  
 la (mm)= 9.20 # of Linear Res. (N)= 3.00  
 U. H. Tp(hrs)= 0.33

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.002 (i)  
 TIME TO PEAK (hrs)= 1.700  
 RUNOFF VOLUME (mm)= 1.655  
 TOTAL RAINFALL (mm)= 31.358  
 RUNOFF COEFFICIENT = 0.053

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO. txt

ADD HYD ( 0054)  
 1 + 2 = 3  
 AREA QPEAK TPEAK R. V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0204): 0.64 0.002 1.70 1.65  
 + ID2= 2 ( 0053): 4.52 0.012 1.43 1.39  
 ID = 3 ( 0054): 5.16 0.013 1.47 1.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 STANDHYD ( 0201)  
 ID= 1 DT= 5.0 min  
 Area (ha)= 2.20  
 Total Imp(%)= 43.00 Dir. Conn. (%)= 21.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha)= 0.95 1.25  
 Dep. Storage (mm)= 1.00 1.50  
 Average Slope (%)= 1.00 1.00  
 Length (m)= 121.11 40.00  
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	2.99	1.083	54.55
0.167	2.99	1.167	54.55
0.250	2.99	1.250	54.55
0.333	3.70	1.333	11.55
0.417	3.70	1.417	11.55
0.500	3.70	1.500	11.55
0.583	5.06	1.583	7.28
0.667	5.06	1.667	7.28
0.750	5.06	1.750	7.28
0.833	9.41	1.833	5.55
0.917	9.41	1.917	5.55
1.000	9.41	2.000	5.55

Max. Eff. Inten. (mm/hr)= 54.55 38.22  
 over (min)= 5.00 20.00  
 Storage Coeff. (min)= 3.65 (ii) 16.42 (ii)  
 Unit Hyd. Tpeak (min)= 5.00 20.00  
 Unit Hyd. peak (cms)= 0.25 0.06

PEAK FLOW (cms)= 0.07 0.06  
 TIME TO PEAK (hrs)= 1.25 1.42  
 RUNOFF VOLUME (mm)= 30.36 6.89  
 TOTAL RAINFALL (mm)= 31.36 31.36  
 RUNOFF COEFFICIENT = 0.97 0.22

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO. txt

RESERVOIR ( 0301 )  
IN= 2---> OUT= 1  
DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 ( 0201 )	2.200	0.102	1.25	11.82
OUTFLOW: ID= 1 ( 0301 )	2.200	0.000	0.00	0.00

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 0.00  
TIME SHIFT OF PEAK FLOW (min) = -75.00  
MAXIMUM STORAGE USED (ha. m.) = 0.0061

ADD HYD ( 0055 )  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0301 ):	2.20	0.000	0.00	0.00
+ ID2= 2 ( 0054 ):	5.16	0.013	1.47	1.42
ID = 3 ( 0055 ):	7.36	0.013	1.47	0.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0205 )  
ID= 1 DT= 5.0 min

Area (ha) = 0.03  
Total Imp (%) = 56.00 Dir. Conn. (%) = 28.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.02	0.01
Dep. Storage (mm)	1.00	1.50
Average Slope (%)	1.00	1.00
Length (m)	14.14	20.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	54.55	61.43
over (min)	5.00	10.00
Storage Coeff. (min)	1.01 (ii)	7.97 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.34	0.13

			*TOTALS*
PEAK FLOW (cms)	0.00	0.00	0.003 (iii)
TIME TO PEAK (hrs)	1.25	1.25	
RUNOFF VOLUME (mm)	30.36	8.18	12.94
TOTAL RAINFALL (mm)	31.36	31.36	31.36
RUNOFF COEFFICIENT	0.97	0.26	0.41

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr) = 50.00 K (1/hr) = 2.00  
Fc (mm/hr) = 7.50 Cum. Inf. (mm) = 0.00  
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CHI CAGO. txt

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0056 )  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0205 ):	0.03	0.003	1.25	12.94
+ ID2= 2 ( 0055 ):	7.36	0.013	1.47	0.99
ID = 3 ( 0056 ):	7.39	0.014	1.43	1.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
NASHYD ( 0207 )  
ID= 1 DT= 5.0 min

Area (ha) = 0.78 Curve Number (CN) = 66.3  
la (mm) = 4.87 # of Linear Res. (N) = 3.00  
U. H. Tp (hrs) = 0.19

Unit Hyd Qpeak (cms) = 0.157

PEAK FLOW (cms) = 0.009 (i)  
TIME TO PEAK (hrs) = 1.333  
RUNOFF VOLUME (mm) = 4.498  
TOTAL RAINFALL (mm) = 31.358  
RUNOFF COEFFICIENT = 0.143

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057 )  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0207 ):	0.78	0.009	1.33	4.50
+ ID2= 2 ( 0056 ):	7.39	0.014	1.43	1.04
ID = 3 ( 0057 ):	8.17	0.023	1.40	1.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0206 )  
ID= 1 DT= 5.0 min

Area (ha) = 0.73  
Total Imp (%) = 32.00 Dir. Conn. (%) = 13.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.23	0.50
Dep. Storage (mm)	1.00	1.50
Average Slope (%)	1.00	2.00
Length (m)	69.76	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	54.55	31.69
over (min)	5.00	15.00
Storage Coeff. (min)	2.62 (ii)	13.80 (ii)

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CHI CAGO. txt  
 Unit Hyd. Tpeak (min)= 5.00 15.00  
 Unit Hyd. peak (cms)= 0.29 0.08  
 \*TOTALS\*  
 PEAK FLOW (cms)= 0.01 0.02 0.032 (iii)  
 TIME TO PEAK (hrs)= 1.25 1.33 1.25  
 RUNOFF VOLUME (mm)= 30.36 6.19 9.33  
 TOTAL RAINFALL (mm)= 31.36 31.36 31.36  
 RUNOFF COEFFICIENT = 0.97 0.20 0.30

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 \*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0401)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 ( 0206):	0.73	0.032	1.25	9.33
+ ID2= 2 ( 0057):	8.17	0.023	1.40	1.37
ID = 3 ( 0401):	8.90	0.051	1.37	2.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L  
 V V I SS U U A A L  
 V V I SS U U A A A A A L  
 V V I SS U U A A L  
 V V I SSSSS UUUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM  
 0 0 T T H H Y Y MM MM 0 0  
 0 0 T T H H Y Y M M 0 0  
 000 T T H H Y Y M M 000

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\VO2\voim.n.dat

Output filename:  
 C:\Users\vaschoof\AppData\Local\Civica\NH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\0528  
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 Summary filename:  
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CHI CAGO. txt

DATE: 10/09/2020

TIME: 05:30:25

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
 \*\* SIMULATION : Run 03  
 \*\*\*\*\*

CHI CAGO STORM  
 Ptotal = 41.81 mm

IDF curve parameters: A= 482.877  
 B= 0.000  
 C= 0.699  
 used in: INTENSITY = A / (t + B)^C

Duration of storm = 4.00 hrs  
 Storm time step = 15.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	3.98	1.25	72.74	2.25	6.10	3.25	3.80
0.50	4.93	1.50	15.40	2.50	5.24	3.50	3.51
0.75	6.75	1.75	9.71	2.75	4.63	3.75	3.26
1.00	12.55	2.00	7.41	3.00	4.17	4.00	3.06

CALIB  
 NASHYD ( 0203)  
 ID= 1 DT= 2.0 min

Area (ha)= 2.89 Curve Number (CN)= 42.9  
 Ia (mm)= 8.98 # of Linear Res. (N)= 3.00  
 U. H. Tp(hrs)= 0.18

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.033	3.98	1.033	72.74	2.033	6.10	3.03	3.80
0.067	3.98	1.067	72.74	2.067	6.10	3.07	3.80
0.100	3.98	1.100	72.74	2.100	6.10	3.10	3.80
0.133	3.98	1.133	72.74	2.133	6.10	3.13	3.80
0.167	3.98	1.167	72.74	2.167	6.10	3.17	3.80
0.200	3.98	1.200	72.74	2.200	6.10	3.20	3.80
0.233	3.98	1.233	72.74	2.233	6.10	3.23	3.80
0.267	4.46	1.267	44.07	2.267	5.67	3.27	3.66
0.300	4.93	1.300	15.40	2.300	5.24	3.30	3.51
0.333	4.93	1.333	15.40	2.333	5.24	3.33	3.51
0.367	4.93	1.367	15.40	2.367	5.24	3.37	3.51
0.400	4.93	1.400	15.40	2.400	5.24	3.40	3.51
0.433	4.93	1.433	15.40	2.433	5.24	3.43	3.51
0.467	4.93	1.467	15.40	2.467	5.24	3.47	3.51
0.500	4.93	1.500	15.40	2.500	5.24	3.50	3.51

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0.533	6.75	1.533	9.71	2.533	4.63	3.53	3.26
0.567	6.75	1.567	9.71	2.567	4.63	3.57	3.26
0.600	6.75	1.600	9.71	2.600	4.63	3.60	3.26
0.633	6.75	1.633	9.71	2.633	4.63	3.63	3.26
0.667	6.75	1.667	9.71	2.667	4.63	3.67	3.26
0.700	6.75	1.700	9.71	2.700	4.63	3.70	3.26
0.733	6.75	1.733	9.71	2.733	4.63	3.73	3.26
0.767	9.65	1.767	8.56	2.767	4.40	3.77	3.16
0.800	12.55	1.800	7.41	2.800	4.17	3.80	3.06
0.833	12.55	1.833	7.41	2.833	4.17	3.83	3.06
0.867	12.55	1.867	7.41	2.867	4.17	3.87	3.06
0.900	12.55	1.900	7.41	2.900	4.17	3.90	3.06
0.933	12.55	1.933	7.41	2.933	4.17	3.93	3.06
0.967	12.55	1.967	7.41	2.967	4.17	3.97	3.06
1.000	12.55	2.000	7.41	3.000	4.17	4.00	3.06

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.019 (i)  
 TIME TO PEAK (hrs)= 1.400  
 RUNOFF VOLUME (mm)= 2.909  
 TOTAL RAINFALL (mm)= 41.810  
 RUNOFF COEFFICIENT = 0.070

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB  
 NASHYD ( 0202)  
 ID= 1 DT= 2.0 min

Area (ha)= 1.63 Curve Number (CN)= 44.1  
 Ia (mm)= 9.62 # of Linear Res. (N)= 3.00  
 U. H. Tp(hrs)= 0.17

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.011 (i)  
 TIME TO PEAK (hrs)= 1.400  
 RUNOFF VOLUME (mm)= 2.928  
 TOTAL RAINFALL (mm)= 41.810  
 RUNOFF COEFFICIENT = 0.070

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0053)  
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1.63	0.011	1.40	2.93
2.89	0.019	1.40	2.91
4.52	0.030	1.40	2.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 NASHYD ( 0204)  
 ID= 1 DT= 2.0 min

Area (ha)= 0.64 Curve Number (CN)= 48.1  
 Ia (mm)= 9.20 # of Linear Res. (N)= 3.00  
 U. H. Tp(hrs)= 0.33

CHI CAGO. txt

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.004 (i)  
 TIME TO PEAK (hrs)= 1.667  
 RUNOFF VOLUME (mm)= 3.464  
 TOTAL RAINFALL (mm)= 41.810  
 RUNOFF COEFFICIENT = 0.083

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0054)  
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.64	0.004	1.67	3.46
4.52	0.030	1.40	2.92
5.16	0.033	1.40	2.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 STANDHYD ( 0201)  
 ID= 1 DT= 5.0 min

Area Total Imp(%)= 43.00 Dir. Conn. (%)= 21.00

IMPERVIOUS	PERVIOUS (i)
0.95	1.25
1.00	1.50
1.00	1.00
121.11	40.00
0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	3.98	1.083	72.74	2.083	6.10	3.08	3.80
0.167	3.98	1.167	72.74	2.167	6.10	3.17	3.80
0.250	3.98	1.250	72.74	2.250	6.10	3.25	3.80
0.333	4.93	1.333	15.40	2.333	5.24	3.33	3.51
0.417	4.93	1.417	15.40	2.417	5.24	3.42	3.51
0.500	4.93	1.500	15.40	2.500	5.24	3.50	3.51
0.583	6.75	1.583	9.71	2.583	4.63	3.58	3.26
0.667	6.75	1.667	9.71	2.667	4.63	3.67	3.26
0.750	6.75	1.750	9.71	2.750	4.63	3.75	3.26
0.833	12.55	1.833	7.41	2.833	4.17	3.83	3.06
0.917	12.55	1.917	7.41	2.917	4.17	3.92	3.06
1.000	12.55	2.000	7.41	3.000	4.17	4.00	3.06

Max. Eff. Inten. (mm/hr)= 72.74  
 over (min)= 5.00  
 Storage Coeff. (min)= 3.25 (ii)  
 Unit Hyd. Tpeak (min)= 5.00  
 Unit Hyd. peak (cms)= 0.27

PEAK FLOW (cms)= 0.09  
 TIME TO PEAK (hrs)= 1.25

\*TOTALS\*  
 0.197 (iii)  
 1.25



CHI CAGO. txt  
 RUNOFF VOLUME (mm)= 40.81 12.20 18.21  
 TOTAL RAINFALL (mm)= 41.81 41.81 41.81  
 RUNOFF COEFFICIENT = 0.98 0.29 0.44

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0301 )  
 IN= 2----> OUT= 1  
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
2.200	0.197	1.25	18.21
2.200	0.000	0.00	0.00

INFLOW : ID= 2 ( 0201)  
 OUTFLOW: ID= 1 ( 0301)

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.00  
 TIME SHIFT OF PEAK FLOW (min) = -75.00  
 MAXIMUM STORAGE USED (ha. m.) = 0.0096

ADD HYD ( 0055 )  
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
2.20	0.000	0.00	0.00
5.16	0.033	1.40	2.98
7.36	0.033	1.40	2.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 STANDHYD ( 0205 )  
 ID= 1 DT= 5.0 min

Area (ha)= 0.03  
 Total Imp(%)= 56.00 Dir. Conn. (%) = 28.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 0.02	0.01
Dep. Storage (mm)= 1.00	1.50
Average Slope (%)= 1.00	1.00
Length (m)= 14.14	20.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr)= 72.74 94.41  
 over (min)= 5.00 10.00  
 Storage Coeff. (min)= 0.90 (ii) 6.76 (ii)  
 Unit Hyd. Tpeak (min)= 5.00 10.00

CHI CAGO. txt  
 Unit Hyd. peak (cms)= 0.34 0.14  
 PEAK FLOW (cms)= 0.00 0.00  
 TIME TO PEAK (hrs)= 1.25 1.25  
 RUNOFF VOLUME (mm)= 40.81 14.53  
 TOTAL RAINFALL (mm)= 41.81 41.81  
 RUNOFF COEFFICIENT = 0.98 0.35

\*TOTALS\*  
 0.004 (iii)  
 1.25  
 20.64  
 41.81  
 0.49

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0056 )  
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.03	0.004	1.25	20.64
7.36	0.033	1.40	2.09
7.39	0.034	1.40	2.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 NASHYD ( 0207 )  
 ID= 1 DT= 5.0 min

Area (ha)= 0.78 Curve Number (CN)= 66.3  
 la (mm)= 4.87 # of Linear Res. (N)= 3.00  
 U. H. Tp(hrs)= 0.19

Unit Hyd Qpeak (cms)= 0.157

PEAK FLOW (cms)= 0.017 (i)  
 TIME TO PEAK (hrs)= 1.333  
 RUNOFF VOLUME (mm)= 8.198  
 TOTAL RAINFALL (mm)= 41.810  
 RUNOFF COEFFICIENT = 0.196

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057 )  
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.78	0.017	1.33	8.20
7.39	0.034	1.40	2.17
8.17	0.051	1.40	2.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

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STANDHYD ( 0206)	Area (ha)= 0.73
ID= 1 DT= 5.0 min	Total Imp(%)= 32.00 Dir. Conn.(%)= 13.00

---

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.23	0.50
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	69.76	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	72.74	65.64
over (min)=	5.00	15.00
Storage Coeff. (min)=	2.34 (ii)	10.69 (ii)
Unit Hyd. Tpeak (min)=	5.00	15.00
Unit Hyd. peak (cms)=	0.30	0.09
PEAK FLOW (cms)=	0.02	0.05
TIME TO PEAK (hrs)=	1.25	1.33
RUNOFF VOLUME (mm)=	40.81	11.29
TOTAL RAINFALL (mm)=	41.81	41.81
RUNOFF COEFFICIENT =	0.98	0.27

\*TOTALS\*  
0.059 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0401)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0206):	0.73	0.059	1.25	15.12
+ ID2= 2 ( 0057):	8.17	0.051	1.40	2.74
ID = 3 ( 0401):	8.90	0.106	1.33	3.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V	V	I	SSSS	U	U	A	L
V	V	I	SS	U	U	A A	L
V	V	I	SS	U	U	AAAA	L
V	V	I	SS	U	U	A A	L
VV	I	SSSS	UUUU	A	A	LLLL	
000	TTTT	TTTT	H	H	Y	Y	M M 000 TM
0	0	T	T	H	H	Y Y	MM MM 0 0
0	0	T	T	H	H	Y	M M 0 0
000	T	T	H	H	Y	M	M 000

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CHICAGO.txt

\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\V02\voimn.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Civica\NH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\b69a1  
566-71c7-40a3-81c1-16e605128619\scen

Summary filename:  
C:\Users\vaschoof\AppData\Local\Civica\NH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\b69a1  
566-71c7-40a3-81c1-16e605128619\scen

DATE: 10/09/2020

TIME: 05:30:25

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 04  
\*\*\*\*\*

CHICAGO STORM	IDF curve parameters: A= 563.357
Ptotal= 48.78 mm	B= 0.000
	C= 0.699

used in: INTENSITY = A / (t + B)^C

Duration of storm = 4.00 hrs  
Storm time step = 15.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	4.65	1.25	84.86	2.25	7.12	3.25	4.44
0.50	5.75	1.50	17.97	2.50	6.12	3.50	4.09
0.75	7.87	1.75	11.33	2.75	5.40	3.75	3.81
1.00	14.64	2.00	8.64	3.00	4.86	4.00	3.56

CALIB	Area (ha)= 2.89	Curve Number (CN)= 42.9
NASHYD ( 0203)	Ia (mm)= 8.98	# of Linear Res. (N)= 3.00
ID= 1 DT= 2.0 min	U.H. Tp(hrs)= 0.18	

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.033	4.65	1.033	84.86	2.033	7.12	3.03	4.44
0.067	4.65	1.067	84.86	2.067	7.12	3.07	4.44

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0.100	4.65	1.100	84.86	2.100	7.12	3.10	4.44
0.133	4.65	1.133	84.86	2.133	7.12	3.13	4.44
0.167	4.65	1.167	84.86	2.167	7.12	3.17	4.44
0.200	4.65	1.200	84.86	2.200	7.12	3.20	4.44
0.233	4.65	1.233	84.86	2.233	7.12	3.23	4.44
0.267	5.20	1.267	51.41	2.267	6.62	3.27	4.26
0.300	5.75	1.300	17.97	2.300	6.12	3.30	4.09
0.333	5.75	1.333	17.97	2.333	6.12	3.33	4.09
0.367	5.75	1.367	17.97	2.367	6.12	3.37	4.09
0.400	5.75	1.400	17.97	2.400	6.12	3.40	4.09
0.433	5.75	1.433	17.97	2.433	6.12	3.43	4.09
0.467	5.75	1.467	17.97	2.467	6.12	3.47	4.09
0.500	5.75	1.500	17.97	2.500	6.12	3.50	4.09
0.533	7.87	1.533	11.33	2.533	5.40	3.53	3.81
0.567	7.87	1.567	11.33	2.567	5.40	3.57	3.81
0.600	7.87	1.600	11.33	2.600	5.40	3.60	3.81
0.633	7.87	1.633	11.33	2.633	5.40	3.63	3.81
0.667	7.87	1.667	11.33	2.667	5.40	3.67	3.81
0.700	7.87	1.700	11.33	2.700	5.40	3.70	3.81
0.733	7.87	1.733	11.33	2.733	5.40	3.73	3.81
0.767	11.26	1.767	9.99	2.767	5.13	3.77	3.69
0.800	14.64	1.800	8.64	2.800	4.86	3.80	3.56
0.833	14.64	1.833	8.64	2.833	4.86	3.83	3.56
0.867	14.64	1.867	8.64	2.867	4.86	3.87	3.56
0.900	14.64	1.900	8.64	2.900	4.86	3.90	3.56
0.933	14.64	1.933	8.64	2.933	4.86	3.93	3.56
0.967	14.64	1.967	8.64	2.967	4.86	3.97	3.56
1.000	14.64	2.000	8.64	3.000	4.86	4.00	3.56

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.029 (i)  
 TIME TO PEAK (hrs)= 1.400  
 RUNOFF VOLUME (mm)= 4.196  
 TOTAL RAINFALL (mm)= 48.779  
 RUNOFF COEFFICIENT = 0.086

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0202) ID= 1 DT= 2.0 min	Area (ha)= 1.63 Ia (mm)= 9.62 U. H. Tp(hrs)= 0.17	Curve Number (CN)= 44.1 # of Linear Res. (N)= 3.00
--	---	---

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.017 (i)  
 TIME TO PEAK (hrs)= 1.367  
 RUNOFF VOLUME (mm)= 4.249  
 TOTAL RAINFALL (mm)= 48.779  
 RUNOFF COEFFICIENT = 0.087

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0053) 1 + 2 = 3	AREA (ha)= 1.63 OPEAK (cms)= 0.017 TPEAK (hrs)= 1.37 R. V. (mm)= 4.25
------------------------------	--

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CHI CAGO. txt

+ ID2= 2 ( 0203): 2.89 0.029 1.40 4.20  
 ID = 3 ( 0053): 4.52 0.046 1.40 4.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ( 0204) ID= 1 DT= 2.0 min	Area (ha)= 0.64 Ia (mm)= 9.20 U. H. Tp(hrs)= 0.33	Curve Number (CN)= 48.1 # of Linear Res. (N)= 3.00
--	---	---

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.005 (i)  
 TIME TO PEAK (hrs)= 1.633  
 RUNOFF VOLUME (mm)= 4.990  
 TOTAL RAINFALL (mm)= 48.779  
 RUNOFF COEFFICIENT = 0.102

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0054) 1 + 2 = 3	AREA (ha)= 0.64 OPEAK (cms)= 0.005 TPEAK (hrs)= 1.63 R. V. (mm)= 4.99	AREA (ha)= 4.52 OPEAK (cms)= 0.046 TPEAK (hrs)= 1.40 R. V. (mm)= 4.22	AREA (ha)= 5.16 OPEAK (cms)= 0.050 TPEAK (hrs)= 1.40 R. V. (mm)= 4.31
------------------------------	--	--	--

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0201) ID= 1 DT= 5.0 min	Area Total (ha)= 2.20 Imp(%)= 43.00	Dir. Conn. (%)= 21.00
--	--	-----------------------

Surface Area (ha)= 0.95	IMPERVIOUS (i)	PERVIOUS (i)
Dep. Storage (mm)= 1.00		
Average Slope (%)= 1.00		
Length (m)= 121.11		
Mannings n = 0.013		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----							
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	4.65	1.083	84.86	2.083	7.12	3.08	4.44
0.167	4.65	1.167	84.86	2.167	7.12	3.17	4.44
0.250	4.65	1.250	84.86	2.250	7.12	3.25	4.44
0.333	5.75	1.333	17.97	2.333	6.12	3.33	4.09
0.417	5.75	1.417	17.97	2.417	6.12	3.42	4.09
0.500	5.75	1.500	17.97	2.500	6.12	3.50	4.09
0.583	7.87	1.583	11.33	2.583	5.40	3.58	3.81
0.667	7.87	1.667	11.33	2.667	5.40	3.67	3.81

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CHICAGO.txt							
0.750	7.87	1.750	11.33	2.750	5.40	3.75	3.81
0.833	14.64	1.833	8.64	2.833	4.86	3.83	3.56
0.917	14.64	1.917	8.64	2.917	4.86	3.92	3.56
1.000	14.64	2.000	8.64	3.000	4.86	4.00	3.56

Max. Eff. Inten. (mm/hr)= 84.86 92.85  
over (min) 5.00 15.00  
Storage Coeff. (min)= 3.06 (ii) 12.01 (ii)  
Unit Hyd. Tpeak (min)= 5.00 15.00  
Unit Hyd. peak (cms)= 0.27 0.09

PEAK FLOW (cms)= 0.11 0.18  
TIME TO PEAK (hrs)= 1.25 1.33  
RUNOFF VOLUME (mm)= 47.78 16.73  
TOTAL RAINFALL (mm)= 48.78 48.78  
RUNOFF COEFFICIENT = 0.98 0.34

\*TOTALS\*  
0.252 (iii)  
1.25  
23.25  
48.78  
0.48

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0301)  
IN= 2--> OUT= 1  
DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

INFLOW : ID= 2 ( 0201)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 ( 0301)	2.200	0.252	1.25	23.25
	2.200	0.000	0.00	0.00

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.00  
TIME SHIFT OF PEAK FLOW (min) = -75.00  
MAXIMUM STORAGE USED (ha. m.) = 0.0118

ADD HYD ( 0055)  
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0301):	2.20	0.000	0.00
+ ID2= 2 ( 0054):	5.16	0.050	1.40
ID = 3 ( 0055):	7.36	0.050	1.40
			3.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0205)

Area (ha)= 0.03  
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CHICAGO.txt  
ID= 1 DT= 5.0 min | Total Imp(%)= 56.00 Dir. Conn. (%)= 28.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	0.02		0.01
Dep. Storage (mm)=	1.00		1.50
Average Slope (%)=	1.00		1.00
Length (m)=	14.14		20.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)= 84.86 116.32  
over (min) 5.00 10.00  
Storage Coeff. (min)= 0.84 (ii) 6.24 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.34 0.15

PEAK FLOW (cms)= 0.00 0.00  
TIME TO PEAK (hrs)= 1.25 1.25  
RUNOFF VOLUME (mm)= 47.78 20.23  
TOTAL RAINFALL (mm)= 48.78 48.78  
RUNOFF COEFFICIENT = 0.98 0.41

\*TOTALS\*  
0.005 (iii)  
1.25  
27.14  
48.78  
0.56

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0056)  
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0205):	0.03	0.005	1.25
+ ID2= 2 ( 0055):	7.36	0.050	1.40
ID = 3 ( 0056):	7.39	0.052	1.40
			3.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
NASHYD ( 0207)  
ID= 1 DT= 5.0 min

Area (ha)= 0.78 Curve Number (CN)= 66.3  
la (mm)= 4.87 # of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.19

Unit Hyd Qpeak (cms)= 0.157

PEAK FLOW (cms)= 0.023 (i)  
TIME TO PEAK (hrs)= 1.333  
RUNOFF VOLUME (mm)= 11.117  
TOTAL RAINFALL (mm)= 48.779  
RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

      CHI CAGO. txt
ADD HYD ( 0057)
  1 + 2 = 3
-----
ID1= 1 ( 0207):   AREA   QPEAK   TPEAK   R. V.
+ ID2= 2 ( 0056):   (ha)   (cms)   (hrs)   (mm)
                    0.78   0.023   1.33   11.12
                    7.39   0.052   1.40   3.12
=====
ID = 3 ( 0057):   8.17   0.075   1.37   3.88

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
STANDHYD ( 0206)
ID= 1 DT= 5.0 min
-----
Area (ha)= 0.73
Total Imp(%)= 32.00 Dir. Conn. (%)= 13.00

```

```

IMPERVIOUS   PERVIOUS (i)
Surface Area (ha)= 0.23 0.50
Dep. Storage (mm)= 1.00 1.50
Average Slope (%)= 1.00 2.00
Length (m)= 69.76 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 84.86 82.84
over (min)= 5.00 10.00
Storage Coeff. (min)= 2.20 (ii) 9.81 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.30 0.11

PEAK FLOW (cms)= 0.02 0.07
TIME TO PEAK (hrs)= 1.25 1.25
RUNOFF VOLUME (mm)= 47.78 15.35
TOTAL RAINFALL (mm)= 48.78 48.78
RUNOFF COEFFICIENT = 0.98 0.31

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 \*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD ( 0401)
  1 + 2 = 3
-----
ID1= 1 ( 0206):   AREA   QPEAK   TPEAK   R. V.
+ ID2= 2 ( 0057):   (ha)   (cms)   (hrs)   (mm)
                    0.73   0.096   1.25   19.56
                    8.17   0.075   1.37   3.88
=====
ID = 3 ( 0401):   8.90   0.152   1.30   5.17

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

      CHI CAGO. txt
V V I SSSSS U U A L
V V I SS U U A A L
V V I SS U U A A L
V V I SS U U A A L
VV I SSSSS UUUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y MM MM 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

```

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#### \*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\VO2\voinput.dat

Output filename:  
 C:\Users\aschoof\AppData\Local\Civica\NH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\de882  
 b0a-435f-40f6-bc51-5789219b519e\scen  
 Summary filename:  
 C:\Users\aschoof\AppData\Local\Civica\NH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\de882  
 b0a-435f-40f6-bc51-5789219b519e\scen

DATE: 10/09/2020

TIME: 05:30:25

USER:

COMMENTS: \_\_\_\_\_

```

*****
** SIMULATION : Run 05
*****

```

```

CHI CAGO STORM
Ptotal = 57.56 mm

```

IDF curve parameters: A= 664.831  
 B= 0.000  
 C= 0.699  
 used in: INTENSITY = A / (t + B)^C

Duration of storm = 4.00 hrs  
 Storm time step = 15.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	5.48	1.25	100.14	2.25	8.40	3.25	5.24
0.50	6.79	1.50	21.21	2.50	7.22	3.50	4.83
0.75	9.29	1.75	13.37	2.75	6.38	3.75	4.49
1.00	17.28	2.00	10.20	3.00	5.74	4.00	4.21

## CHI CAGO. txt

CALIB  
NASHYD ( 0203)  
ID= 1 DT= 2.0 min

Area (ha)= 2.89 Curve Number (CN)= 42.9  
la (mm)= 8.98 # of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.18

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.033	5.48	1.033	100.14	2.033	8.40	3.03	5.24
0.067	5.48	1.067	100.14	2.067	8.40	3.07	5.24
0.100	5.48	1.100	100.14	2.100	8.40	3.10	5.24
0.133	5.48	1.133	100.14	2.133	8.40	3.13	5.24
0.167	5.48	1.167	100.14	2.167	8.40	3.17	5.24
0.200	5.48	1.200	100.14	2.200	8.40	3.20	5.24
0.233	5.48	1.233	100.14	2.233	8.40	3.23	5.24
0.267	6.14	1.267	60.67	2.267	7.81	3.27	5.03
0.300	6.79	1.300	21.21	2.300	7.22	3.30	4.83
0.333	6.79	1.333	21.21	2.333	7.22	3.33	4.83
0.367	6.79	1.367	21.21	2.367	7.22	3.37	4.83
0.400	6.79	1.400	21.21	2.400	7.22	3.40	4.83
0.433	6.79	1.433	21.21	2.433	7.22	3.43	4.83
0.467	6.79	1.467	21.21	2.467	7.22	3.47	4.83
0.500	6.79	1.500	21.21	2.500	7.22	3.50	4.83
0.533	9.29	1.533	13.37	2.533	6.38	3.53	4.49
0.567	9.29	1.567	13.37	2.567	6.38	3.57	4.49
0.600	9.29	1.600	13.37	2.600	6.38	3.60	4.49
0.633	9.29	1.633	13.37	2.633	6.38	3.63	4.49
0.667	9.29	1.667	13.37	2.667	6.38	3.67	4.49
0.700	9.29	1.700	13.37	2.700	6.38	3.70	4.49
0.733	9.29	1.733	13.37	2.733	6.38	3.73	4.49
0.767	13.28	1.767	11.78	2.767	6.06	3.77	4.35
0.800	17.28	1.800	10.20	2.800	5.74	3.80	4.21
0.833	17.28	1.833	10.20	2.833	5.74	3.83	4.21
0.867	17.28	1.867	10.20	2.867	5.74	3.87	4.21
0.900	17.28	1.900	10.20	2.900	5.74	3.90	4.21
0.933	17.28	1.933	10.20	2.933	5.74	3.93	4.21
0.967	17.28	1.967	10.20	2.967	5.74	3.97	4.21
1.000	17.28	2.000	10.20	3.000	5.74	4.00	4.21

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.044 (i)  
TIME TO PEAK (hrs)= 1.400  
RUNOFF VOLUME (mm)= 6.111  
TOTAL RAINFALL (mm)= 57.565  
RUNOFF COEFFICIENT = 0.106

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB  
NASHYD ( 0202)  
ID= 1 DT= 2.0 min

Area (ha)= 1.63 Curve Number (CN)= 44.1  
la (mm)= 9.62 # of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.17

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.026 (i)  
TIME TO PEAK (hrs)= 1.367

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## CHI CAGO. txt

RUNOFF VOLUME (mm)= 6.219  
TOTAL RAINFALL (mm)= 57.565  
RUNOFF COEFFICIENT = 0.108

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0053)  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0202):	1.63	0.026	1.37	6.22
+ ID2= 2 ( 0203):	2.89	0.044	1.40	6.11
=====				
ID = 3 ( 0053):	4.52	0.070	1.37	6.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
NASHYD ( 0204)  
ID= 1 DT= 2.0 min

Area (ha)= 0.64 Curve Number (CN)= 48.1  
la (mm)= 9.20 # of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.33

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.008 (i)  
TIME TO PEAK (hrs)= 1.633  
RUNOFF VOLUME (mm)= 7.249  
TOTAL RAINFALL (mm)= 57.565  
RUNOFF COEFFICIENT = 0.126

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0054)  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0204):	0.64	0.008	1.63	7.25
+ ID2= 2 ( 0053):	4.52	0.070	1.37	6.15
=====				
ID = 3 ( 0054):	5.16	0.076	1.40	6.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0201)  
ID= 1 DT= 5.0 min

Area (ha)= 2.20  
Total Imp(%)= 43.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.95	1.25
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	1.00
Length (m)=	121.11	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

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## CHI CAGO. txt

----- TRANSFORMED HYETOGRAPH -----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	5.48	1.083	100.14	2.083	8.40	3.08	5.24
0.167	5.48	1.167	100.14	2.167	8.40	3.17	5.24
0.250	5.48	1.250	100.14	2.250	8.40	3.25	5.24
0.333	6.79	1.333	21.21	2.333	7.22	3.33	4.83
0.417	6.79	1.417	21.21	2.417	7.22	3.42	4.83
0.500	6.79	1.500	21.21	2.500	7.22	3.50	4.83
0.583	9.29	1.583	13.37	2.583	6.38	3.58	4.49
0.667	9.29	1.667	13.37	2.667	6.38	3.67	4.49
0.750	9.29	1.750	13.37	2.750	6.38	3.75	4.49
0.833	17.28	1.833	10.20	2.833	5.74	3.83	4.21
0.917	17.28	1.917	10.20	2.917	5.74	3.92	4.21
1.000	17.28	2.000	10.20	3.000	5.74	4.00	4.21

Max. Eff. Inten. (mm/hr) = 100.14 116.25  
 over (min) = 5.00 15.00  
 Storage Coeff. (min) = 2.86 (ii) 11.05 (ii)  
 Unit Hyd. Tpeak (min) = 5.00 15.00  
 Unit Hyd. peak (cms) = 0.28 0.09

PEAK FLOW (cms) = 0.13 0.24 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.25 1.33 0.323 (iii)  
 RUNOFF VOLUME (mm) = 56.56 23.86 30.73  
 TOTAL RAINFALL (mm) = 57.56 57.56 57.56  
 RUNOFF COEFFICIENT = 0.98 0.41 0.53

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 $F_o$  (mm/hr) = 50.00  $K$  (1/hr) = 2.00  
 $F_c$  (mm/hr) = 7.50 Cum. Inf. (mm) = 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0301 )  
 IN= 2---> OUT= 1  
 DT= 5.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

INFLOW :	ID= 2 ( 0201 )	AREA	OPEAK	TPEAK	R. V.
OUTFLOW:	ID= 1 ( 0301 )	(ha)	(cms)	(hrs)	(mm)
		2.200	0.323	1.25	30.73
		2.200	0.000	0.00	0.00

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.00  
 TIME SHIFT OF PEAK FLOW (min) = -75.00  
 MAXIMUM STORAGE USED (ha.m.) = 0.0146

| ADD HYD ( 0055 ) |

CHI CAGO. txt				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 ( 0301 ):	2.20	0.000	0.00	0.00
+ ID2= 2 ( 0054 ):	5.16	0.076	1.40	6.29
ID = 3 ( 0055 ):	7.36	0.076	1.40	4.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)=	0.03	Dir. Conn. (%) = 28.00
STANDHYD ( 0205 )	Total	Imp (%) =	56.00	
ID= 1 DT= 5.0 min				

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.02 0.01  
 Dep. Storage (mm) = 1.00 1.50  
 Average Slope (%) = 1.00 1.00  
 Length (m) = 14.14 20.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 100.14 143.70  
 over (min) = 5.00 10.00  
 Storage Coeff. (min) = 0.79 (ii) 5.75 (ii)  
 Unit Hyd. Tpeak (min) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.34 0.15

PEAK FLOW (cms) = 0.00 0.00 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.25 1.25 0.007 (iii)  
 RUNOFF VOLUME (mm) = 56.56 28.04 35.73  
 TOTAL RAINFALL (mm) = 57.56 57.56 57.56  
 RUNOFF COEFFICIENT = 0.98 0.49 0.62

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 $F_o$  (mm/hr) = 50.00  $K$  (1/hr) = 2.00  
 $F_c$  (mm/hr) = 7.50 Cum. Inf. (mm) = 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0056 )				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 ( 0205 ):	0.03	0.007	1.25	35.73
+ ID2= 2 ( 0055 ):	7.36	0.076	1.40	4.41
ID = 3 ( 0056 ):	7.39	0.079	1.37	4.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)=	0.78	Curve Number (CN)= 66.3
NASHYD ( 0207 )	la	(mm)=	4.87	# of Linear Res. (N)= 3.00
ID= 1 DT= 5.0 min	U. H. Tp(hrs)=	0.19		

CHI CAGO. txt  
Unit Hyd Qpeak (cms)= 0.157  
PEAK FLOW (cms)= 0.033 (i)  
TIME TO PEAK (hrs)= 1.333  
RUNOFF VOLUME (mm)= 15.238  
TOTAL RAINFALL (mm)= 57.565  
RUNOFF COEFFICIENT = 0.265

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
| ADD HYD ( 0057) |  
1 + 2 = 3
ID1= 1 ( 0207): AREA (ha)= 0.78 QPEAK (cms)= 0.033 TPEAK (hrs)= 1.33 R. V. (mm)= 15.24  
+ ID2= 2 ( 0056): 7.39 0.079 1.37 4.53  
=====

ID = 3 ( 0057): 8.17 0.111 1.37 5.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
| CALIB |  
| STANDHYD ( 0206) |  
ID= 1 DT= 5.0 min
Area (ha)= 0.73  
Total Imp(%)= 32.00 Dir. Conn.(%)= 13.00

IMPERVIOUS PVIOUS (i)  
Surface Area (ha)= 0.23 0.50  
Dep. Storage (mm)= 1.00 1.50  
Average Slope (%)= 1.00 2.00  
Length (m)= 69.76 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 100.14 104.47  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 2.06 (ii) 8.99 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.31 0.12

\*TOTALS\*  
PEAK FLOW (cms)= 0.03 0.10 0.125 (iii)  
TIME TO PEAK (hrs)= 1.25 1.25 1.25  
RUNOFF VOLUME (mm)= 56.56 21.84 26.35  
TOTAL RAINFALL (mm)= 57.56 57.56 57.56  
RUNOFF COEFFICIENT = 0.98 0.38 0.46

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) HORTONS EQUATION SELECTED FOR PVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----  
ADD HYD ( 0401)

CHI CAGO. txt  
| 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R. V. (mm)  
-----  
ID1= 1 ( 0206): 0.73 0.125 1.25 26.35  
+ ID2= 2 ( 0057): 8.17 0.111 1.37 5.56  
=====

ID = 3 ( 0401): 8.90 0.212 1.30 7.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----  
=====

V V I SSSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A L  
V V I SS U U A A L  
VV I SSSSS UUUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y MM MM 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\V02\vo in. dat

Output filename:

C:\Users\Naschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\9aeee  
e6d-ae35-4491-96bb-d4fad8984bd1\scen

Summary filename:

C:\Users\Naschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\9aeee  
e6d-ae35-4491-96bb-d4fad8984bd1\scen

DATE: 10/09/2020

TIME: 05:30:25

USER:

COMMENTS: \_\_\_\_\_

-----  
\*\*\*\*\*  
\*\* SIMULATION : Run 06 \*\*  
\*\*\*\*\*

-----  
| CHI CAGO STORM |  
Ptotal = 63.93 mm

IDF curve parameters: A= 738.312  
B= 0.000  
C= 0.699

used in: INTENSITY = A / (t + B)^C

Duration of storm = 4.00 hrs



CHICAGO.txt  
Storm time step = 15.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	6.09	1.25	111.21	2.25	9.33	3.25	5.82
0.50	7.54	1.50	23.55	2.50	8.02	3.50	5.36
0.75	10.32	1.75	14.85	2.75	7.08	3.75	4.99
1.00	19.19	2.00	11.32	3.00	6.37	4.00	4.67

CALIB NASHYD ( 0203) ID= 1 DT= 2.0 min	Area (ha)= la (mm)= U. H. Tp(hrs)=	2.89 8.98 0.18	Curve Number (CN)= 42.9 # of Linear Res. (N)= 3.00
--	--	----------------------	---

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.033	6.09	1.033	111.21	2.033	9.33	3.03	5.82
0.067	6.09	1.067	111.21	2.067	9.33	3.07	5.82
0.100	6.09	1.100	111.21	2.100	9.33	3.10	5.82
0.133	6.09	1.133	111.21	2.133	9.33	3.13	5.82
0.167	6.09	1.167	111.21	2.167	9.33	3.17	5.82
0.200	6.09	1.200	111.21	2.200	9.33	3.20	5.82
0.233	6.09	1.233	111.21	2.233	9.33	3.23	5.82
0.267	6.82	1.267	67.38	2.267	8.67	3.27	5.59
0.300	7.54	1.300	23.55	2.300	8.02	3.30	5.36
0.333	7.54	1.333	23.55	2.333	8.02	3.33	5.36
0.367	7.54	1.367	23.55	2.367	8.02	3.37	5.36
0.400	7.54	1.400	23.55	2.400	8.02	3.40	5.36
0.433	7.54	1.433	23.55	2.433	8.02	3.43	5.36
0.467	7.54	1.467	23.55	2.467	8.02	3.47	5.36
0.500	7.54	1.500	23.55	2.500	8.02	3.50	5.36
0.533	10.32	1.533	14.85	2.533	7.08	3.53	4.99
0.567	10.32	1.567	14.85	2.567	7.08	3.57	4.99
0.600	10.32	1.600	14.85	2.600	7.08	3.60	4.99
0.633	10.32	1.633	14.85	2.633	7.08	3.63	4.99
0.667	10.32	1.667	14.85	2.667	7.08	3.67	4.99
0.700	10.32	1.700	14.85	2.700	7.08	3.70	4.99
0.733	10.32	1.733	14.85	2.733	7.08	3.73	4.99
0.767	14.75	1.767	13.09	2.767	6.73	3.77	4.83
0.800	19.19	1.800	11.32	2.800	6.37	3.80	4.67
0.833	19.19	1.833	11.32	2.833	6.37	3.83	4.67
0.867	19.19	1.867	11.32	2.867	6.37	3.87	4.67
0.900	19.19	1.900	11.32	2.900	6.37	3.90	4.67
0.933	19.19	1.933	11.32	2.933	6.37	3.93	4.67
0.967	19.19	1.967	11.32	2.967	6.37	3.97	4.67
1.000	19.19	2.000	11.32	3.000	6.37	4.00	4.67

Unit Hyd Qpeak (cms)= 0.613  
PEAK FLOW (cms)= 0.057 (i)  
TIME TO PEAK (hrs)= 1.367  
RUNOFF VOLUME (mm)= 7.689  
TOTAL RAINFALL (mm)= 63.927  
RUNOFF COEFFICIENT = 0.120

CHICAGO.txt  
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0202) ID= 1 DT= 2.0 min	Area (ha)= la (mm)= U. H. Tp(hrs)=	1.63 9.62 0.17	Curve Number (CN)= 44.1 # of Linear Res. (N)= 3.00
--	--	----------------------	---

Unit Hyd Qpeak (cms)= 0.366  
PEAK FLOW (cms)= 0.034 (i)  
TIME TO PEAK (hrs)= 1.367  
RUNOFF VOLUME (mm)= 7.844  
TOTAL RAINFALL (mm)= 63.927  
RUNOFF COEFFICIENT = 0.123

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0053) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0202):	1.63	0.034	1.37	7.84
+ ID2= 2 ( 0203):	2.89	0.057	1.37	7.69
ID = 3 ( 0053):	4.52	0.090	1.37	7.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ( 0204) ID= 1 DT= 2.0 min	Area (ha)= la (mm)= U. H. Tp(hrs)=	0.64 9.20 0.33	Curve Number (CN)= 48.1 # of Linear Res. (N)= 3.00
--	--	----------------------	---

Unit Hyd Qpeak (cms)= 0.074  
PEAK FLOW (cms)= 0.010 (i)  
TIME TO PEAK (hrs)= 1.600  
RUNOFF VOLUME (mm)= 9.102  
TOTAL RAINFALL (mm)= 63.927  
RUNOFF COEFFICIENT = 0.142

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0054) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0204):	0.64	0.010	1.60	9.10
+ ID2= 2 ( 0053):	4.52	0.090	1.37	7.75
ID = 3 ( 0054):	5.16	0.098	1.40	7.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

## CHI CAGO. txt

CALIB  
STANDHYD ( 0201)  
ID= 1 DT= 5.0 min

Area (ha)= 2.20  
Total Imp(%)= 43.00 Dir. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.95 1.25  
Dep. Storage (mm)= 1.00 1.50  
Average Slope (%)= 1.00 1.00  
Length (m)= 121.11 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	6.09	1.083	111.21
0.167	6.09	1.167	111.21
0.250	6.09	1.250	111.21
0.333	7.54	1.333	23.55
0.417	7.54	1.417	23.55
0.500	7.54	1.500	23.55
0.583	10.32	1.583	14.85
0.667	10.32	1.667	14.85
0.750	10.32	1.750	14.85
0.833	19.19	1.833	11.32
0.917	19.19	1.917	11.32
1.000	19.19	2.000	11.32

Max. Eff. Inten. (mm/hr)= 111.21 133.14  
over (min)= 5.00 15.00  
Storage Coeff. (min)= 2.75 (ii) 10.50 (ii)  
Unit Hyd. Tpeak (min)= 5.00 15.00  
Unit Hyd. peak (cms)= 0.28 0.09

PEAK FLOW (cms)= 0.14 0.28  
TIME TO PEAK (hrs)= 1.25 1.33  
RUNOFF VOLUME (mm)= 62.93 29.44  
TOTAL RAINFALL (mm)= 63.93 63.93  
RUNOFF COEFFICIENT = 0.98 0.46

\*TOTALS\*

0.376 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0301)  
IN= 2--> OUT= 1  
DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

## CHI CAGO. txt

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)  
INFLOW : ID= 2 ( 0201) 2.200 0.376 1.25 36.47  
OUTFLOW: ID= 1 ( 0301) 2.200 0.000 0.00 0.00

PEAK FLOW REDUCTION [Qout/Qin](%)= 0.00  
TIME SHIFT OF PEAK FLOW (min)= -75.00  
MAXIMUM STORAGE USED (ha. m.)= 0.0167

ADD HYD ( 0055)  
1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)  
ID1= 1 ( 0301): 2.20 0.000 0.00 0.00  
+ ID2= 2 ( 0054): 5.16 0.098 1.40 7.91  
ID = 3 ( 0055): 7.36 0.098 1.40 5.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0205)  
ID= 1 DT= 5.0 min

Area (ha)= 0.03  
Total Imp(%)= 56.00 Dir. Conn.(%)= 28.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.02 0.01  
Dep. Storage (mm)= 1.00 1.50  
Average Slope (%)= 1.00 1.00  
Length (m)= 14.14 20.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 111.21 162.90  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 0.76 (ii) 5.47 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.34 0.16

PEAK FLOW (cms)= 0.00 0.01  
TIME TO PEAK (hrs)= 1.25 1.25  
RUNOFF VOLUME (mm)= 62.93 34.06  
TOTAL RAINFALL (mm)= 63.93 63.93  
RUNOFF COEFFICIENT = 0.98 0.53

\*TOTALS\*

0.008 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0056)  
1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)  
ID1= 1 ( 0205): 0.03 0.008 1.25 42.14

CHICAGO.txt  
+ ID2= 2 ( 0055): 7.36 0.098 1.40 5.55  
=====

ID = 3 ( 0056): 7.39 0.102 1.37 5.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
NASHYD ( 0207)  
ID= 1 DT= 5.0 min  
Area (ha)= 0.78 Curve Number (CN)= 66.3  
Ia (mm)= 4.87 # of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.19

Unit Hyd Qpeak (cms)= 0.157

PEAK FLOW (cms)= 0.041 (i)  
TIME TO PEAK (hrs)= 1.333  
RUNOFF VOLUME (mm)= 18.492  
TOTAL RAINFALL (mm)= 63.927  
RUNOFF COEFFICIENT = 0.289

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057)  
1 + 2 = 3  
AREA (ha) QPEAK (cms) TPEAK (hrs) R. V. (mm)  
ID1= 1 ( 0207): 0.78 0.041 1.33 18.49  
+ ID2= 2 ( 0056): 7.39 0.102 1.37 5.70  
=====

ID = 3 ( 0057): 8.17 0.142 1.37 6.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0206)  
ID= 1 DT= 5.0 min  
Area Total Imp(%)= 0.73 32.00 Dir. Conn. (%)= 13.00

IMPERVIOUS Pervious (i)  
Surface Area (ha)= 0.23 0.50  
Dep. Storage (mm)= 1.00 1.50  
Average Slope (%)= 1.00 2.00  
Length (m)= 69.76 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 111.21 120.10  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.97 (ii) 8.53 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.31 0.12

\*TOTALS\*  
PEAK FLOW (cms)= 0.03 0.12 0.146 (iii)  
TIME TO PEAK (hrs)= 1.25 1.25 1.25  
RUNOFF VOLUME (mm)= 62.93 27.21 31.85  
TOTAL RAINFALL (mm)= 63.93 63.93 63.93  
RUNOFF COEFFICIENT = 0.98 0.43 0.50

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

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CHICAGO.txt  
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) HORTONS EQUATION SELECTED FOR PVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0401)  
1 + 2 = 3  
AREA (ha) QPEAK (cms) TPEAK (hrs) R. V. (mm)  
ID1= 1 ( 0206): 0.73 0.146 1.25 31.85  
+ ID2= 2 ( 0057): 8.17 0.142 1.37 6.92  
=====

ID = 3 ( 0401): 8.90 0.259 1.30 8.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L  
VV I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y MM MM 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\VO2\vo1n.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\7b8f4  
fbf-99ea-42f2-b2a9-8c05dc2d42f9\scen

Summary filename:  
C:\Users\vaschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\7b8f4  
fbf-99ea-42f2-b2a9-8c05dc2d42f9\scen

DATE: 10/09/2020

TIME: 05:30:25

USER:

COMMENTS: \_\_\_\_\_

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CHI CAGO.txt

\*\*\*\*\*  
 \*\* SIMULATION : Run 07 \*\*  
 \*\*\*\*\*

CHI CAGO STORM  
 Ptotal= 70.29 mm

IDF curve parameters: A= 811.794  
 B= 0.000  
 C= 0.699  
 used in: INTENSITY =  $A / (t + B)^C$   
 Duration of storm = 4.00 hrs  
 Storm time step = 15.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	6.69	1.25	122.28	2.25	10.26	3.25	6.39
0.50	8.29	1.50	25.89	2.50	8.82	3.50	5.90
0.75	11.34	1.75	16.33	2.75	7.79	3.75	5.48
1.00	21.10	2.00	12.45	3.00	7.01	4.00	5.14

CALIB  
 NASHYD ( 0203)  
 ID= 1 DT= 2.0 min

Area (ha)= 2.89 Curve Number (CN)= 42.9  
 Ia (mm)= 8.98 # of Linear Res. (N)= 3.00  
 U.H. Tp(hrs)= 0.18

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----							
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.033	6.69	1.033	122.28	2.033	10.26	3.03	6.39
0.067	6.69	1.067	122.28	2.067	10.26	3.07	6.39
0.100	6.69	1.100	122.28	2.100	10.26	3.10	6.39
0.133	6.69	1.133	122.28	2.133	10.26	3.13	6.39
0.167	6.69	1.167	122.28	2.167	10.26	3.17	6.39
0.200	6.69	1.200	122.28	2.200	10.26	3.20	6.39
0.233	6.69	1.233	122.28	2.233	10.26	3.23	6.39
0.267	7.49	1.267	74.09	2.267	9.54	3.27	6.15
0.300	8.29	1.300	25.89	2.300	8.82	3.30	5.90
0.333	8.29	1.333	25.89	2.333	8.82	3.33	5.90
0.367	8.29	1.367	25.89	2.367	8.82	3.37	5.90
0.400	8.29	1.400	25.89	2.400	8.82	3.40	5.90
0.433	8.29	1.433	25.89	2.433	8.82	3.43	5.90
0.467	8.29	1.467	25.89	2.467	8.82	3.47	5.90
0.500	8.29	1.500	25.89	2.500	8.82	3.50	5.90
0.533	11.34	1.533	16.33	2.533	7.79	3.53	5.48
0.567	11.34	1.567	16.33	2.567	7.79	3.57	5.48
0.600	11.34	1.600	16.33	2.600	7.79	3.60	5.48
0.633	11.34	1.633	16.33	2.633	7.79	3.63	5.48
0.667	11.34	1.667	16.33	2.667	7.79	3.67	5.48
0.700	11.34	1.700	16.33	2.700	7.79	3.70	5.48
0.733	11.34	1.733	16.33	2.733	7.79	3.73	5.48
0.767	16.22	1.767	14.39	2.767	7.40	3.77	5.31
0.800	21.10	1.800	12.45	2.800	7.01	3.80	5.14
0.833	21.10	1.833	12.45	2.833	7.01	3.83	5.14
0.867	21.10	1.867	12.45	2.867	7.01	3.87	5.14

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CHI CAGO.txt

0.900	21.10	1.900	12.45	2.900	7.01	3.90	5.14
0.933	21.10	1.933	12.45	2.933	7.01	3.93	5.14
0.967	21.10	1.967	12.45	2.967	7.01	3.97	5.14
1.000	21.10	2.000	12.45	3.000	7.01	4.00	5.14

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.071 (i)  
 TIME TO PEAK (hrs)= 1.367  
 RUNOFF VOLUME (mm)= 9.421  
 TOTAL RAINFALL (mm)= 70.290  
 RUNOFF COEFFICIENT = 0.134

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB  
 NASHYD ( 0202)  
 ID= 1 DT= 2.0 min

Area (ha)= 1.63 Curve Number (CN)= 44.1  
 Ia (mm)= 9.62 # of Linear Res. (N)= 3.00  
 U.H. Tp(hrs)= 0.17

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.042 (i)  
 TIME TO PEAK (hrs)= 1.367  
 RUNOFF VOLUME (mm)= 9.627  
 TOTAL RAINFALL (mm)= 70.290  
 RUNOFF COEFFICIENT = 0.137

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0053)  
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0202):	1.63	0.042	1.37	9.63
+ ID2= 2 ( 0203):	2.89	0.071	1.37	9.42
=====				
ID = 3 ( 0053):	4.52	0.113	1.37	9.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 NASHYD ( 0204)  
 ID= 1 DT= 2.0 min

Area (ha)= 0.64 Curve Number (CN)= 48.1  
 Ia (mm)= 9.20 # of Linear Res. (N)= 3.00  
 U.H. Tp(hrs)= 0.33

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.013 (i)  
 TIME TO PEAK (hrs)= 1.600  
 RUNOFF VOLUME (mm)= 11.126  
 TOTAL RAINFALL (mm)= 70.290  
 RUNOFF COEFFICIENT = 0.158

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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CHI CAGO. txt  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0056)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 ( 0205):	0.03	0.009	1.25	48.36
+ ID2= 2 ( 0055):	7.36	0.122	1.40	6.80
=====				
ID = 3 ( 0056):	7.39	0.127	1.37	6.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ( 0207)	Area (ha)	Curve Number (CN)= 66.3
ID= 1 DT= 5.0 min	0.78	# of Linear Res. (N)= 3.00
U. H. Tp(hrs)=	0.19	

Unit Hyd Opeak	(cms)=	0.157
PEAK FLOW	(cms)=	0.049 (i)
TIME TO PEAK	(hrs)=	1.333
RUNOFF VOLUME	(mm)=	21.949
TOTAL RAINFALL	(mm)=	70.290
RUNOFF COEFFICIENT	=	0.312

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 ( 0207):	0.78	0.049	1.33	21.95
+ ID2= 2 ( 0056):	7.39	0.127	1.37	6.97
=====				
ID = 3 ( 0057):	8.17	0.175	1.37	8.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0206)	Area (ha)	Dir. Conn. (%)= 13.00
ID= 1 DT= 5.0 min	0.73	
Total Imp(%)=	32.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)= 0.23	0.50
Dep. Storage	(mm)= 1.00	1.50
Average Slope	(%)= 1.00	2.00
Length	(m)= 69.76	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten. (mm/hr)=	122.28	135.68

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	CHI CAGO. txt	
Storage over (min)	5.00	10.00
Storage Coeff. (min)=	1.90 (ii)	8.15 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.32	0.13
PEAK FLOW (cms)=	0.03	0.14
TIME TO PEAK (hrs)=	1.25	1.25
RUNOFF VOLUME (mm)=	69.29	32.82
TOTAL RAINFALL (mm)=	70.29	70.29
RUNOFF COEFFICIENT =	0.99	0.47
		*TOTALS* (iii)
		0.168 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) HORTONS EQUATION SELECTED FOR PVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0401)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 ( 0206):	0.73	0.168	1.25	37.56
+ ID2= 2 ( 0057):	8.17	0.175	1.37	8.40
=====				
ID = 3 ( 0401):	8.90	0.310	1.33	10.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLLL
```

```
000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y MM MM 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y M M 000
```

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\VO2\voin.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Civica\58194ef53-adad-4f15-90f7-c4eafb4675c3\ae8e6  
add-3119-4302-958a-8c44339408e0\scen  
Summary filename:

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CHI CAGO. txt  
C: \Users\aschoof\AppData\Local \Ci vi ca\ \VH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\ae8e6  
add-3119-4302-958a-8c44339408e0\scen

DATE: 10/09/2020 TIME: 05: 30: 25

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 08 \*\*  
\*\*\*\*\*

-----  
READ STORM  
Ptotal=193.00 mm  
Filename: C: \Users\aschoof\AppData  
ata\Local \Temp\  
ef6ca355-8b23-4216-859d-d5ae6efa7ec3\4c311f83  
Comments: T I M M I N S R E G I O N A L 12 H O U R D U R A T I O N S T O R M

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.20	15.00	3.20	3.00	6.20	43.00	9.20	13.00
0.40	15.00	3.40	3.00	6.40	43.00	9.40	13.00
0.60	15.00	3.60	3.00	6.60	43.00	9.60	13.00
0.80	15.00	3.80	3.00	6.80	43.00	9.80	13.00
1.00	15.00	4.00	3.00	7.00	43.00	10.00	13.00
1.20	20.00	4.20	5.00	7.20	20.00	10.20	13.00
1.40	20.00	4.40	5.00	7.40	20.00	10.40	13.00
1.60	20.00	4.60	5.00	7.60	20.00	10.60	13.00
1.80	20.00	4.80	5.00	7.80	20.00	10.80	13.00
2.00	20.00	5.00	5.00	8.00	20.00	11.00	13.00
2.20	10.00	5.20	20.00	8.20	23.00	11.20	8.00
2.40	10.00	5.40	20.00	8.40	23.00	11.40	8.00
2.60	10.00	5.60	20.00	8.60	23.00	11.60	8.00
2.80	10.00	5.80	20.00	8.80	23.00	11.80	8.00
3.00	10.00	6.00	20.00	9.00	23.00	12.00	8.00

-----  
CALIB  
NASHYD ( 0203)  
ID= 1 DT= 2.0 min  
Area (ha)= 2.89 Curve Number (CN)= 42.9  
Ia (mm)= 8.98 # of Linear Res. (N)= 3.00  
U.H. Tp(hrs)= 0.18

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.033	15.00	3.033	3.00	6.033	43.00	9.03	13.00
0.067	15.00	3.067	3.00	6.067	43.00	9.07	13.00
0.100	15.00	3.100	3.00	6.100	43.00	9.10	13.00
0.133	15.00	3.133	3.00	6.133	43.00	9.13	13.00
0.167	15.00	3.167	3.00	6.167	43.00	9.17	13.00

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0.200	15.00	3.200	3.00	6.200	43.00	9.20	13.00
0.233	15.00	3.233	3.00	6.233	43.00	9.23	13.00
0.267	15.00	3.267	3.00	6.267	43.00	9.27	13.00
0.300	15.00	3.300	3.00	6.300	43.00	9.30	13.00
0.333	15.00	3.333	3.00	6.333	43.00	9.33	13.00
0.367	15.00	3.367	3.00	6.367	43.00	9.37	13.00
0.400	15.00	3.400	3.00	6.400	43.00	9.40	13.00
0.433	15.00	3.433	3.00	6.433	43.00	9.43	13.00
0.467	15.00	3.467	3.00	6.467	43.00	9.47	13.00
0.500	15.00	3.500	3.00	6.500	43.00	9.50	13.00
0.533	15.00	3.533	3.00	6.533	43.00	9.53	13.00
0.567	15.00	3.567	3.00	6.567	43.00	9.57	13.00
0.600	15.00	3.600	3.00	6.600	43.00	9.60	13.00
0.633	15.00	3.633	3.00	6.633	43.00	9.63	13.00
0.667	15.00	3.667	3.00	6.667	43.00	9.67	13.00
0.700	15.00	3.700	3.00	6.700	43.00	9.70	13.00
0.733	15.00	3.733	3.00	6.733	43.00	9.73	13.00
0.767	15.00	3.767	3.00	6.767	43.00	9.77	13.00
0.800	15.00	3.800	3.00	6.800	43.00	9.80	13.00
0.833	15.00	3.833	3.00	6.833	43.00	9.83	13.00
0.867	15.00	3.867	3.00	6.867	43.00	9.87	13.00
0.900	15.00	3.900	3.00	6.900	43.00	9.90	13.00
0.933	15.00	3.933	3.00	6.933	43.00	9.93	13.00
0.967	15.00	3.967	3.00	6.967	43.00	9.97	13.00
1.000	15.00	4.000	3.00	7.000	43.00	10.00	13.00
1.033	20.00	4.033	5.00	7.033	20.00	10.03	13.00
1.067	20.00	4.067	5.00	7.067	20.00	10.07	13.00
1.100	20.00	4.100	5.00	7.100	20.00	10.10	13.00
1.133	20.00	4.133	5.00	7.133	20.00	10.13	13.00
1.167	20.00	4.167	5.00	7.167	20.00	10.17	13.00
1.200	20.00	4.200	5.00	7.200	20.00	10.20	13.00
1.233	20.00	4.233	5.00	7.233	20.00	10.23	13.00
1.267	20.00	4.267	5.00	7.267	20.00	10.27	13.00
1.300	20.00	4.300	5.00	7.300	20.00	10.30	13.00
1.333	20.00	4.333	5.00	7.333	20.00	10.33	13.00
1.367	20.00	4.367	5.00	7.367	20.00	10.37	13.00
1.400	20.00	4.400	5.00	7.400	20.00	10.40	13.00
1.433	20.00	4.433	5.00	7.433	20.00	10.43	13.00
1.467	20.00	4.467	5.00	7.467	20.00	10.47	13.00
1.500	20.00	4.500	5.00	7.500	20.00	10.50	13.00
1.533	20.00	4.533	5.00	7.533	20.00	10.53	13.00
1.567	20.00	4.567	5.00	7.567	20.00	10.57	13.00
1.600	20.00	4.600	5.00	7.600	20.00	10.60	13.00
1.633	20.00	4.633	5.00	7.633	20.00	10.63	13.00
1.667	20.00	4.667	5.00	7.667	20.00	10.67	13.00
1.700	20.00	4.700	5.00	7.700	20.00	10.70	13.00
1.733	20.00	4.733	5.00	7.733	20.00	10.73	13.00
1.767	20.00	4.767	5.00	7.767	20.00	10.77	13.00
1.800	20.00	4.800	5.00	7.800	20.00	10.80	13.00
1.833	20.00	4.833	5.00	7.833	20.00	10.83	13.00
1.867	20.00	4.867	5.00	7.867	20.00	10.87	13.00
1.900	20.00	4.900	5.00	7.900	20.00	10.90	13.00
1.933	20.00	4.933	5.00	7.933	20.00	10.93	13.00
1.967	20.00	4.967	5.00	7.967	20.00	10.97	13.00
2.000	20.00	5.000	5.00	8.000	20.00	11.00	13.00
2.033	10.00	5.033	20.00	8.033	23.00	11.03	8.00
2.067	10.00	5.067	20.00	8.067	23.00	11.07	8.00
2.100	10.00	5.100	20.00	8.100	23.00	11.10	8.00
2.133	10.00	5.133	20.00	8.133	23.00	11.13	8.00
2.167	10.00	5.167	20.00	8.167	23.00	11.17	8.00
2.200	10.00	5.200	20.00	8.200	23.00	11.20	8.00
2.233	10.00	5.233	20.00	8.233	23.00	11.23	8.00
2.267	10.00	5.267	20.00	8.267	23.00	11.27	8.00

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2.300	10.00	5.300	20.00	8.300	23.00	11.30	8.00
2.333	10.00	5.333	20.00	8.333	23.00	11.33	8.00
2.367	10.00	5.367	20.00	8.367	23.00	11.37	8.00
2.400	10.00	5.400	20.00	8.400	23.00	11.40	8.00
2.433	10.00	5.433	20.00	8.433	23.00	11.43	8.00
2.467	10.00	5.467	20.00	8.467	23.00	11.47	8.00
2.500	10.00	5.500	20.00	8.500	23.00	11.50	8.00
2.533	10.00	5.533	20.00	8.533	23.00	11.53	8.00
2.567	10.00	5.567	20.00	8.567	23.00	11.57	8.00
2.600	10.00	5.600	20.00	8.600	23.00	11.60	8.00
2.633	10.00	5.633	20.00	8.633	23.00	11.63	8.00
2.667	10.00	5.667	20.00	8.667	23.00	11.67	8.00
2.700	10.00	5.700	20.00	8.700	23.00	11.70	8.00
2.733	10.00	5.733	20.00	8.733	23.00	11.73	8.00
2.767	10.00	5.767	20.00	8.767	23.00	11.77	8.00
2.800	10.00	5.800	20.00	8.800	23.00	11.80	8.00
2.833	10.00	5.833	20.00	8.833	23.00	11.83	8.00
2.867	10.00	5.867	20.00	8.867	23.00	11.87	8.00
2.900	10.00	5.900	20.00	8.900	23.00	11.90	8.00
2.933	10.00	5.933	20.00	8.933	23.00	11.93	8.00
2.967	10.00	5.967	20.00	8.967	23.00	11.97	8.00
3.000	10.00	6.000	20.00	9.000	23.00	12.00	7.99

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.136 (i)  
 TIME TO PEAK (hrs)= 7.033  
 RUNOFF VOLUME (mm)= 64.906  
 TOTAL RAINFALL (mm)= 192.999  
 RUNOFF COEFFICIENT = 0.336

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB  
 NASHYD ( 0202)  
 ID= 1 DT= 2.0 min

Area (ha)= 1.63 Curve Number (CN)= 44.1  
 Ia (mm)= 9.62 # of Linear Res. (N)= 3.00  
 U. H. Tp(hrs)= 0.17

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.079 (i)  
 TIME TO PEAK (hrs)= 7.000  
 RUNOFF VOLUME (mm)= 66.587  
 TOTAL RAINFALL (mm)= 192.999  
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0053)  
 1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)  
 ID1= 1 ( 0202): 1.63 0.079 7.00 66.59  
 + ID2= 2 ( 0203): 2.89 0.136 7.03 64.91  
 ID = 3 ( 0053): 4.52 0.215 7.00 65.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO. txt

CALIB  
 NASHYD ( 0204)  
 ID= 1 DT= 2.0 min

Area (ha)= 0.64 Curve Number (CN)= 48.1  
 Ia (mm)= 9.20 # of Linear Res. (N)= 3.00  
 U. H. Tp(hrs)= 0.33

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.032 (i)  
 TIME TO PEAK (hrs)= 7.100  
 RUNOFF VOLUME (mm)= 73.745  
 TOTAL RAINFALL (mm)= 192.999  
 RUNOFF COEFFICIENT = 0.382

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0054)  
 1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)  
 ID1= 1 ( 0204): 0.64 0.032 7.10 73.74  
 + ID2= 2 ( 0053): 4.52 0.215 7.00 65.51  
 ID = 3 ( 0054): 5.16 0.247 7.03 66.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 STANDHYD ( 0201)  
 ID= 1 DT= 5.0 min

Area (ha)= 2.20  
 Total Imp(%)= 43.00 Dir. Conn. (%)= 21.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha)= 0.95 1.25  
 Dep. Storage (mm)= 1.00 1.50  
 Average Slope (%)= 1.00 1.00  
 Length (m)= 121.11 40.00  
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	15.00	3.083	3.00	6.083	43.00	9.08	13.00
0.167	15.00	3.167	3.00	6.167	43.00	9.17	13.00
0.250	15.00	3.250	3.00	6.250	43.00	9.25	13.00
0.333	15.00	3.333	3.00	6.333	43.00	9.33	13.00
0.417	15.00	3.417	3.00	6.417	43.00	9.42	13.00
0.500	15.00	3.500	3.00	6.500	43.00	9.50	13.00
0.583	15.00	3.583	3.00	6.583	43.00	9.58	13.00
0.667	15.00	3.667	3.00	6.667	43.00	9.67	13.00
0.750	15.00	3.750	3.00	6.750	43.00	9.75	13.00
0.833	15.00	3.833	3.00	6.833	43.00	9.83	13.00
0.917	15.00	3.917	3.00	6.917	43.00	9.92	13.00
1.000	15.00	4.000	3.00	7.000	43.00	10.00	13.00
1.083	20.00	4.083	5.00	7.083	20.00	10.08	13.00
1.167	20.00	4.167	5.00	7.167	20.00	10.17	13.00



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1.250	20.00	4.250	5.00	7.250	20.00	10.25	13.00
1.333	20.00	4.333	5.00	7.333	20.00	10.33	13.00
1.417	20.00	4.417	5.00	7.417	20.00	10.42	13.00
1.500	20.00	4.500	5.00	7.500	20.00	10.50	13.00
1.583	20.00	4.583	5.00	7.583	20.00	10.58	13.00
1.667	20.00	4.667	5.00	7.667	20.00	10.67	13.00
1.750	20.00	4.750	5.00	7.750	20.00	10.75	13.00
1.833	20.00	4.833	5.00	7.833	20.00	10.83	13.00
1.917	20.00	4.917	5.00	7.917	20.00	10.92	13.00
2.000	20.00	5.000	5.00	8.000	20.00	11.00	13.00
2.083	10.00	5.083	20.00	8.083	23.00	11.08	8.00
2.167	10.00	5.167	20.00	8.167	23.00	11.17	8.00
2.250	10.00	5.250	20.00	8.250	23.00	11.25	8.00
2.333	10.00	5.333	20.00	8.333	23.00	11.33	8.00
2.417	10.00	5.417	20.00	8.417	23.00	11.42	8.00
2.500	10.00	5.500	20.00	8.500	23.00	11.50	8.00
2.583	10.00	5.583	20.00	8.583	23.00	11.58	8.00
2.667	10.00	5.667	20.00	8.667	23.00	11.67	8.00
2.750	10.00	5.750	20.00	8.750	23.00	11.75	8.00
2.833	10.00	5.833	20.00	8.833	23.00	11.83	8.00
2.917	10.00	5.917	20.00	8.917	23.00	11.92	8.00
3.000	10.00	6.000	20.00	9.000	23.00	12.00	8.00

Max. Eff. Inten. (mm/hr)= 43.00 52.10  
over (min) 5.00 20.00  
Storage Coeff. (min)= 4.02 (ii) 15.30 (ii)  
Unit Hyd. Tpeak (min)= 5.00 20.00  
Unit Hyd. peak (cms)= 0.24 0.07

PEAK FLOW (cms)= 0.06 0.18 \*TOTALS\*  
TIME TO PEAK (hrs)= 6.92 7.00 0.232 (iii)  
RUNOFF VOLUME (mm)= 192.00 115.25 131.36  
TOTAL RAINFALL (mm)= 193.00 193.00 193.00  
RUNOFF COEFFICIENT = 0.99 0.60 0.68

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0301)  
IN= 2---> OUT= 1  
DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 ( 0201)	2.200	0.232	7.00	131.36
OUTFLOW: ID= 1 ( 0301)	2.200	0.052	11.00	54.90

PEAK FLOW REDUCTION [Qout/Qin] (%) = 22.58  
TIME SHIFT OF PEAK FLOW (min) = 240.00  
MAXIMUM STORAGE USED (ha.m.) = 0.1898

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## CHI CAGO. txt

ADD HYD ( 0055)  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0301):	2.20	0.052	11.00	54.90
+ ID2= 2 ( 0054):	5.16	0.247	7.03	66.53
ID = 3 ( 0055):	7.36	0.247	7.03	63.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0205)  
ID= 1 DT= 5.0 min

Area (ha)= 0.03  
Total Imp(%)= 56.00 Dir. Conn. (%)= 28.00

## IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.02 0.01  
Dep. Storage (mm)= 1.00 1.50  
Average Slope (%)= 1.00 1.00  
Length (m)= 14.14 20.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 43.00 62.86  
over (min) 5.00 10.00  
Storage Coeff. (min)= 1.11 (ii) 8.01 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.34 0.13

PEAK FLOW (cms)= 0.00 0.00 \*TOTALS\*  
TIME TO PEAK (hrs)= 6.25 7.00 0.003 (iii)  
RUNOFF VOLUME (mm)= 192.00 125.73 7.00  
TOTAL RAINFALL (mm)= 193.00 193.00 144.09  
RUNOFF COEFFICIENT = 0.99 0.65 193.00  
0.75

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0056)  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0205):	0.03	0.003	7.00	144.09
+ ID2= 2 ( 0055):	7.36	0.247	7.03	63.06
ID = 3 ( 0056):	7.39	0.250	7.00	63.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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## CHI CAGO. txt

CALIB				
NASHYD ( 0207)	Area (ha)=	0.78	Curve Number (CN)=	66.3
ID= 1 DT= 5.0 min	la (mm)=	4.87	# of Linear Res. (N)=	3.00
	U. H. Tp(hrs)=	0.19		

Unit Hyd Qpeak (cms)= 0.157

PEAK FLOW (cms)= 0.063 (i)  
 TIME TO PEAK (hrs)= 7.000  
 RUNOFF VOLUME (mm)= 111.306  
 TOTAL RAINFALL (mm)= 193.000  
 RUNOFF COEFFICIENT = 0.577

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0207):	0.78	0.063	7.00	111.31
+ ID2= 2 ( 0056):	7.39	0.250	7.00	63.38
=====				
ID = 3 ( 0057):	8.17	0.313	7.00	67.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
STANDHYD ( 0206)	Area (ha)=	0.73		
ID= 1 DT= 5.0 min	Total Imp(%)=	32.00	Dir. Conn. (%)=	13.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	0.23	0.50	
Dep. Storage (mm)=	1.00	1.50	
Average Slope (%)=	1.00	2.00	
Length (m)=	69.76	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	43.00	47.51	
over (min)=	5.00	15.00	
Storage Coeff. (min)=	2.88 (ii)	12.39 (ii)	
Unit Hyd. Tpeak (min)=	5.00	15.00	
Unit Hyd. peak (cms)=	0.28	0.08	
			*TOTALS*
PEAK FLOW (cms)=	0.01	0.06	0.076 (iii)
TIME TO PEAK (hrs)=	6.83	7.00	7.00
RUNOFF VOLUME (mm)=	192.00	109.90	120.57
TOTAL RAINFALL (mm)=	193.00	193.00	193.00
RUNOFF COEFFICIENT =	0.99	0.57	0.62

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:

Fo (mm/hr)= 50.00 K (1/hr)= 2.00

Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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## CHI CAGO. txt

ADD HYD ( 0401)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0206):	0.73	0.076	7.00	120.57
+ ID2= 2 ( 0057):	8.17	0.313	7.00	67.96
=====				
ID = 3 ( 0401):	8.90	0.390	7.00	72.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

## SCS.txt

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V   V   I   SSSSS U   U   A   L
V   V   I   SS   U   U   A   L
V   V   I   SS   U   U   AAAAA L
V   V   I   SS   U   U   A   A   L
W   W   I   SSSSS UUUUU A   A   LLLLL
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000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000
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## \*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

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Output filename:  
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DATE: 10/09/2020 TIME: 05:31:52

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 01 \*\*  
\*\*\*\*\*

MASS STORM

Ptotal = 53.90 mm

Filename: C:\Users\aschoof\AppData  
Local\Temp\  
d21eaacb-4cf1-4504-bf53-d3c44604e13d\bce6bd08  
Comments: SCS Type II 24 HR MASS CURVE

Duration of storm = 23.75 hrs  
Mass curve time step = 15.00 min

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	0.65	6.25	1.08	12.25	7.76	18.25	0.86
0.50	0.43	6.50	0.86	12.50	4.10	18.50	1.08
0.75	0.65	6.75	1.08	12.75	3.88	18.75	0.86
1.00	0.65	7.00	1.08	13.00	3.02	19.00	1.08
1.25	0.65	7.25	1.29	13.25	2.80	19.25	0.86
1.50	0.43	7.50	1.08	13.50	2.37	19.50	1.08

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## SCS.txt

1.75	0.65	7.75	1.29	13.75	2.16	19.75	0.86
2.00	0.65	8.00	1.29	14.00	1.72	20.00	0.65
2.25	0.86	8.25	1.51	14.25	1.51	20.25	0.65
2.50	0.65	8.50	1.51	14.50	1.72	20.50	0.65
2.75	0.65	8.75	1.51	14.75	1.51	20.75	0.65
3.00	0.65	9.00	1.72	15.00	1.72	21.00	0.65
3.25	0.86	9.25	1.72	15.25	1.51	21.25	0.65
3.50	0.65	9.50	1.94	15.50	1.72	21.50	0.65
3.75	0.65	9.75	1.94	15.75	1.51	21.75	0.65
4.00	0.86	10.00	2.37	16.00	1.08	22.00	0.65
4.25	0.86	10.25	2.59	16.25	0.86	22.25	0.65
4.50	0.86	10.50	3.23	16.50	1.08	22.50	0.65
4.75	0.86	10.75	3.45	16.75	0.86	22.75	0.65
5.00	0.86	11.00	5.17	17.00	1.08	23.00	0.65
5.25	0.86	11.25	5.17	17.25	0.86	23.25	0.65
5.50	0.86	11.50	15.95	17.50	1.08	23.50	0.65
5.75	0.86	11.75	65.97	17.75	0.86	23.75	0.65
6.00	0.86	12.00	7.76	18.00	1.08		

CALIB  
NASHYD ( 0202)  
ID= 1 DT= 2.0 min

Area (ha)= 1.63  
la (mm)= 9.62  
U. H. Tp(hrs)= 0.17

Curve Number (CN)= 44.1  
# of Linear Res. (N)= 3.00

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.033	0.65	6.000	0.86	11.967	7.76	17.93	1.08
0.067	0.65	6.033	1.08	12.000	7.76	17.97	1.08
0.100	0.65	6.067	1.08	12.033	7.76	18.00	1.08
0.133	0.65	6.100	1.08	12.067	7.76	18.03	0.86
0.167	0.65	6.133	1.08	12.100	7.76	18.07	0.86
0.200	0.65	6.167	1.08	12.133	7.76	18.10	0.86
0.233	0.65	6.200	1.08	12.167	7.76	18.13	0.86
0.267	0.54	6.233	1.08	12.200	7.76	18.17	0.86
0.300	0.43	6.267	0.97	12.233	7.76	18.20	0.86
0.333	0.43	6.300	0.86	12.267	5.92	18.23	0.86
0.367	0.43	6.333	0.86	12.300	4.10	18.27	0.97
0.400	0.43	6.367	0.86	12.333	4.10	18.30	1.08
0.433	0.43	6.400	0.86	12.367	4.10	18.33	1.08
0.467	0.43	6.433	0.86	12.400	4.10	18.37	1.08
0.500	0.43	6.467	0.86	12.433	4.10	18.40	1.08
0.533	0.65	6.500	0.86	12.467	4.10	18.43	1.08
0.567	0.65	6.533	1.08	12.500	4.10	18.47	1.08
0.600	0.65	6.567	1.08	12.533	3.88	18.50	1.08
0.633	0.65	6.600	1.08	12.567	3.88	18.53	0.86
0.667	0.65	6.633	1.08	12.600	3.88	18.57	0.86
0.700	0.65	6.667	1.08	12.633	3.88	18.60	0.86
0.733	0.65	6.700	1.08	12.667	3.88	18.63	0.86
0.767	0.65	6.733	1.08	12.700	3.88	18.67	0.86
0.800	0.65	6.767	1.08	12.733	3.88	18.70	0.86
0.833	0.65	6.800	1.08	12.767	3.45	18.73	0.86
0.867	0.65	6.833	1.08	12.800	3.02	18.77	0.97
0.900	0.65	6.867	1.08	12.833	3.02	18.80	1.08
0.933	0.65	6.900	1.08	12.867	3.02	18.83	1.08
0.967	0.65	6.933	1.08	12.900	3.02	18.87	1.08
1.000	0.65	6.967	1.08	12.933	3.02	18.90	1.08
1.033	0.65	7.000	1.08	12.967	3.02	18.93	1.08

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SCS.txt							
1.067	0.65	7.033	1.29	13.000	3.02	18.97	1.08
1.100	0.65	7.067	1.29	13.033	2.80	19.00	1.08
1.133	0.65	7.100	1.29	13.067	2.80	19.03	0.86
1.167	0.65	7.133	1.29	13.100	2.80	19.07	0.86
1.200	0.65	7.167	1.29	13.133	2.80	19.10	0.86
1.233	0.65	7.200	1.29	13.167	2.80	19.13	0.86
1.267	0.54	7.233	1.29	13.200	2.80	19.17	0.86
1.300	0.43	7.267	1.19	13.233	2.80	19.20	0.86
1.333	0.43	7.300	1.08	13.267	2.59	19.23	0.86
1.367	0.43	7.333	1.08	13.300	2.37	19.27	0.97
1.400	0.43	7.367	1.08	13.333	2.37	19.30	1.08
1.433	0.43	7.400	1.08	13.367	2.37	19.33	1.08
1.467	0.43	7.433	1.08	13.400	2.37	19.37	1.08
1.500	0.43	7.467	1.08	13.433	2.37	19.40	1.08
1.533	0.65	7.500	1.08	13.467	2.37	19.43	1.08
1.567	0.65	7.533	1.29	13.500	2.37	19.47	1.08
1.600	0.65	7.567	1.29	13.533	2.16	19.50	1.08
1.633	0.65	7.600	1.29	13.567	2.16	19.53	0.86
1.667	0.65	7.633	1.29	13.600	2.16	19.57	0.86
1.700	0.65	7.667	1.29	13.633	2.16	19.60	0.86
1.733	0.65	7.700	1.29	13.667	2.16	19.63	0.86
1.767	0.65	7.733	1.29	13.700	2.16	19.67	0.86
1.800	0.65	7.767	1.29	13.733	2.16	19.70	0.86
1.833	0.65	7.800	1.29	13.767	1.94	19.73	0.86
1.867	0.65	7.833	1.29	13.800	1.72	19.77	0.75
1.900	0.65	7.867	1.29	13.833	1.72	19.80	0.65
1.933	0.65	7.900	1.29	13.867	1.72	19.83	0.65
1.967	0.65	7.933	1.29	13.900	1.72	19.87	0.65
2.000	0.65	7.967	1.29	13.933	1.72	19.90	0.65
2.033	0.86	8.000	1.29	13.967	1.72	19.93	0.65
2.067	0.86	8.033	1.51	14.000	1.72	19.97	0.65
2.100	0.86	8.067	1.51	14.033	1.51	20.00	0.65
2.133	0.86	8.100	1.51	14.067	1.51	20.03	0.65
2.167	0.86	8.133	1.51	14.100	1.51	20.07	0.65
2.200	0.86	8.167	1.51	14.133	1.51	20.10	0.65
2.233	0.86	8.200	1.51	14.167	1.51	20.13	0.65
2.267	0.75	8.233	1.51	14.200	1.51	20.17	0.65
2.300	0.65	8.267	1.51	14.233	1.51	20.20	0.65
2.333	0.65	8.300	1.51	14.267	1.62	20.23	0.65
2.367	0.65	8.333	1.51	14.300	1.72	20.27	0.65
2.400	0.65	8.367	1.51	14.333	1.72	20.30	0.65
2.433	0.65	8.400	1.51	14.367	1.72	20.33	0.65
2.467	0.65	8.433	1.51	14.400	1.72	20.37	0.65
2.500	0.65	8.467	1.51	14.433	1.72	20.40	0.65
2.533	0.65	8.500	1.51	14.467	1.72	20.43	0.65
2.567	0.65	8.533	1.51	14.500	1.72	20.47	0.65
2.600	0.65	8.567	1.51	14.533	1.51	20.50	0.65
2.633	0.65	8.600	1.51	14.567	1.51	20.53	0.65
2.667	0.65	8.633	1.51	14.600	1.51	20.57	0.65
2.700	0.65	8.667	1.51	14.633	1.51	20.60	0.65
2.733	0.65	8.700	1.51	14.667	1.51	20.63	0.65
2.767	0.65	8.733	1.51	14.700	1.51	20.67	0.65
2.800	0.65	8.767	1.62	14.733	1.51	20.70	0.65
2.833	0.65	8.800	1.72	14.767	1.62	20.73	0.65
2.867	0.65	8.833	1.72	14.800	1.72	20.77	0.65
2.900	0.65	8.867	1.72	14.833	1.72	20.80	0.65
2.933	0.65	8.900	1.72	14.867	1.72	20.83	0.65
2.967	0.65	8.933	1.72	14.900	1.72	20.87	0.65
3.000	0.65	8.967	1.72	14.933	1.72	20.90	0.65
3.033	0.86	9.000	1.72	14.967	1.72	20.93	0.65
3.067	0.86	9.033	1.72	15.000	1.72	20.97	0.65
3.100	0.86	9.067	1.72	15.033	1.51	21.00	0.65
3.133	0.86	9.100	1.72	15.067	1.51	21.03	0.65

SCS.txt							
3.167	0.86	9.133	1.72	15.100	1.51	21.07	0.65
3.200	0.86	9.167	1.72	15.133	1.51	21.10	0.65
3.233	0.86	9.200	1.72	15.167	1.51	21.13	0.65
3.267	0.75	9.233	1.72	15.200	1.51	21.17	0.65
3.300	0.65	9.267	1.83	15.233	1.51	21.20	0.65
3.333	0.65	9.300	1.94	15.267	1.62	21.23	0.65
3.367	0.65	9.333	1.94	15.300	1.72	21.27	0.65
3.400	0.65	9.367	1.94	15.333	1.72	21.30	0.65
3.433	0.65	9.400	1.94	15.367	1.72	21.33	0.65
3.467	0.65	9.433	1.94	15.400	1.72	21.37	0.65
3.500	0.65	9.467	1.94	15.433	1.72	21.40	0.65
3.533	0.65	9.500	1.94	15.467	1.72	21.43	0.65
3.567	0.65	9.533	1.94	15.500	1.72	21.47	0.65
3.600	0.65	9.567	1.94	15.533	1.51	21.50	0.65
3.633	0.65	9.600	1.94	15.567	1.51	21.53	0.65
3.667	0.65	9.633	1.94	15.600	1.51	21.57	0.65
3.700	0.65	9.667	1.94	15.633	1.51	21.60	0.65
3.733	0.65	9.700	1.94	15.667	1.51	21.63	0.65
3.767	0.75	9.733	1.94	15.700	1.51	21.67	0.65
3.800	0.86	9.767	2.16	15.733	1.51	21.70	0.65
3.833	0.86	9.800	2.37	15.767	1.29	21.73	0.65
3.867	0.86	9.833	2.37	15.800	1.08	21.77	0.65
3.900	0.86	9.867	2.37	15.833	1.08	21.80	0.65
3.933	0.86	9.900	2.37	15.867	1.08	21.83	0.65
3.967	0.86	9.933	2.37	15.900	1.08	21.87	0.65
4.000	0.86	9.967	2.37	15.933	1.08	21.90	0.65
4.033	0.86	10.000	2.37	15.967	1.08	21.93	0.65
4.067	0.86	10.033	2.59	16.000	1.08	21.97	0.65
4.100	0.86	10.067	2.59	16.033	0.86	22.00	0.65
4.133	0.86	10.100	2.59	16.067	0.86	22.03	0.65
4.167	0.86	10.133	2.59	16.100	0.86	22.07	0.65
4.200	0.86	10.167	2.59	16.133	0.86	22.10	0.65
4.233	0.86	10.200	2.59	16.167	0.86	22.13	0.65
4.267	0.86	10.233	2.59	16.200	0.86	22.17	0.65
4.300	0.86	10.267	2.91	16.233	0.86	22.20	0.65
4.333	0.86	10.300	3.23	16.267	0.97	22.23	0.65
4.367	0.86	10.333	3.23	16.300	1.08	22.27	0.65
4.400	0.86	10.367	3.23	16.333	1.08	22.30	0.65
4.433	0.86	10.400	3.23	16.367	1.08	22.33	0.65
4.467	0.86	10.433	3.23	16.400	1.08	22.37	0.65
4.500	0.86	10.467	3.23	16.433	1.08	22.40	0.65
4.533	0.86	10.500	3.23	16.467	1.08	22.43	0.65
4.567	0.86	10.533	3.45	16.500	1.08	22.47	0.65
4.600	0.86	10.567	3.45	16.533	0.86	22.50	0.65
4.633	0.86	10.600	3.45	16.567	0.86	22.53	0.65
4.667	0.86	10.633	3.45	16.600	0.86	22.57	0.65
4.700	0.86	10.667	3.45	16.633	0.86	22.60	0.65
4.733	0.86	10.700	3.45	16.667	0.86	22.63	0.65
4.767	0.86	10.733	3.45	16.700	0.86	22.67	0.65
4.800	0.86	10.767	4.31	16.733	0.86	22.70	0.65
4.833	0.86	10.800	5.17	16.767	0.97	22.73	0.65
4.867	0.86	10.833	5.17	16.800	1.08	22.77	0.65
4.900	0.86	10.867	5.17	16.833	1.08	22.80	0.65
4.933	0.86	10.900	5.17	16.867	1.08	22.83	0.65
4.967	0.86	10.933	5.17	16.900	1.08	22.87	0.65
5.000	0.86	10.967	5.17	16.933	1.08	22.90	0.65
5.033	0.86	11.000	5.17	16.967	1.08	22.93	0.65
5.067	0.86	11.033	5.17	17.000	1.08	22.97	0.65
5.100	0.86	11.067	5.17	17.033	0.86	23.00	0.65
5.133	0.86	11.100	5.17	17.067	0.86	23.03	0.65
5.167	0.86	11.133	5.17	17.100	0.86	23.07	0.65
5.200	0.86	11.167	5.17	17.133	0.86	23.10	0.65
5.233	0.86	11.200	5.17	17.167	0.86	23.13	0.65

## SCS.txt

5.267	0.86	11.233	5.17	17.200	0.86	23.17	0.65
5.300	0.86	11.267	10.58	17.233	0.86	23.20	0.65
5.333	0.86	11.300	15.95	17.267	0.97	23.23	0.65
5.367	0.86	11.333	15.95	17.300	1.08	23.27	0.65
5.400	0.86	11.367	15.95	17.333	1.08	23.30	0.65
5.433	0.86	11.400	15.95	17.367	1.08	23.33	0.65
5.467	0.86	11.433	15.95	17.400	1.08	23.37	0.65
5.500	0.86	11.467	15.95	17.433	1.08	23.40	0.65
5.533	0.86	11.500	16.01	17.467	1.08	23.43	0.65
5.567	0.86	11.533	65.97	17.500	1.08	23.47	0.65
5.600	0.86	11.567	65.97	17.533	0.86	23.50	0.65
5.633	0.86	11.600	65.97	17.567	0.86	23.53	0.65
5.667	0.86	11.633	65.97	17.600	0.86	23.57	0.65
5.700	0.86	11.667	65.97	17.633	0.86	23.60	0.65
5.733	0.86	11.700	65.97	17.667	0.86	23.63	0.65
5.767	0.86	11.733	65.97	17.700	0.86	23.67	0.65
5.800	0.86	11.767	36.79	17.733	0.86	23.70	0.65
5.833	0.86	11.800	7.76	17.767	0.97	23.73	0.65
5.867	0.86	11.833	7.76	17.800	1.08	23.77	0.65
5.900	0.86	11.867	7.76	17.833	1.08		
5.933	0.86	11.900	7.76	17.867	1.08		
5.967	0.86	11.933	7.76	17.900	1.08		

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.022 (i)  
 TIME TO PEAK (hrs)= 11.833  
 RUNOFF VOLUME (mm)= 5.321  
 TOTAL RAINFALL (mm)= 53.738  
 RUNOFF COEFFICIENT = 0.099

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB							
NASHYD ( 0203)							
ID= 1 DT= 2.0 min	Area (ha)= 2.89	Curve Number (CN)= 42.9					
	Ia (mm)= 8.98	# of Linear Res. (N)= 3.00					
	U. H. Tp(hrs)= 0.18						

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.037 (i)  
 TIME TO PEAK (hrs)= 11.833  
 RUNOFF VOLUME (mm)= 5.238  
 TOTAL RAINFALL (mm)= 53.738  
 RUNOFF COEFFICIENT = 0.097

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057)							
1 + 2 = 3							
ID1= 1 ( 0202):	AREA (ha)= 1.63	OPEAK (cms)= 0.022	TPEAK (hrs)= 11.83	R. V. (mm)= 5.32			
+ ID2= 2 ( 0203):	2.89	0.037	11.83	5.24			
ID = 3 ( 0057):	4.52	0.059	11.83	5.27			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

## SCS.txt

CALIB							
NASHYD ( 0204)							
ID= 1 DT= 2.0 min	Area (ha)= 0.64	Curve Number (CN)= 48.1					
	Ia (mm)= 9.20	# of Linear Res. (N)= 3.00					
	U. H. Tp(hrs)= 0.33						

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.006 (i)  
 TIME TO PEAK (hrs)= 12.000  
 RUNOFF VOLUME (mm)= 6.220  
 TOTAL RAINFALL (mm)= 53.738  
 RUNOFF COEFFICIENT = 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0058)							
1 + 2 = 3							
ID1= 1 ( 0204):	AREA (ha)= 0.64	OPEAK (cms)= 0.006	TPEAK (hrs)= 12.00	R. V. (mm)= 6.22			
+ ID2= 2 ( 0057):	4.52	0.059	11.83	5.27			
ID = 3 ( 0058):	5.16	0.064	11.83	5.39			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB							
STANDHYD ( 0201)							
ID= 1 DT= 5.0 min	Area (ha)= 2.20	Dir. Conn. (%)= 21.00					
	Total Imp(%)= 43.00						

		IMPERVIOUS		PERVIOUS (i)	
Surface Area	(ha)= 0.95			1.25	
Dep. Storage	(mm)= 1.00			1.50	
Average Slope	(%)= 1.00			1.00	
Length	(m)= 121.11			40.00	
Mannings n	= 0.013			0.250	

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.65	6.083	1.08	12.083	7.76	18.08	0.86
0.167	0.65	6.167	1.08	12.167	7.76	18.17	0.86
0.250	0.65	6.250	1.08	12.250	7.76	18.25	0.86
0.333	0.43	6.333	0.86	12.333	4.10	18.33	1.08
0.417	0.43	6.417	0.86	12.417	4.10	18.42	1.08
0.500	0.43	6.500	0.86	12.500	4.10	18.50	1.08
0.583	0.65	6.583	1.08	12.583	3.88	18.58	0.86
0.667	0.65	6.667	1.08	12.667	3.88	18.67	0.86
0.750	0.65	6.750	1.08	12.750	3.88	18.75	0.86
0.833	0.65	6.833	1.08	12.833	3.02	18.83	1.08
0.917	0.65	6.917	1.08	12.917	3.02	18.92	1.08
1.000	0.65	7.000	1.08	13.000	3.02	19.00	1.08
1.083	0.65	7.083	1.29	13.083	2.80	19.08	0.86
1.167	0.65	7.167	1.29	13.167	2.80	19.17	0.86

## SCS.txt

1.250	0.65	7.250	1.29	13.250	2.80	19.25	0.86
1.333	0.43	7.333	1.08	13.333	2.37	19.33	1.08
1.417	0.43	7.417	1.08	13.417	2.37	19.42	1.08
1.500	0.43	7.500	1.08	13.500	2.37	19.50	1.08
1.583	0.65	7.583	1.29	13.583	2.16	19.58	0.86
1.667	0.65	7.667	1.29	13.667	2.16	19.67	0.86
1.750	0.65	7.750	1.29	13.750	2.16	19.75	0.86
1.833	0.65	7.833	1.29	13.833	1.72	19.83	0.65
1.917	0.65	7.917	1.29	13.917	1.72	19.92	0.65
2.000	0.65	8.000	1.29	14.000	1.72	20.00	0.65
2.083	0.86	8.083	1.51	14.083	1.51	20.08	0.65
2.167	0.86	8.167	1.51	14.167	1.51	20.17	0.65
2.250	0.86	8.250	1.51	14.250	1.51	20.25	0.65
2.333	0.65	8.333	1.51	14.333	1.72	20.33	0.65
2.417	0.65	8.417	1.51	14.417	1.72	20.42	0.65
2.500	0.65	8.500	1.51	14.500	1.72	20.50	0.65
2.583	0.65	8.583	1.51	14.583	1.51	20.58	0.65
2.667	0.65	8.667	1.51	14.667	1.51	20.67	0.65
2.750	0.65	8.750	1.51	14.750	1.51	20.75	0.65
2.833	0.65	8.833	1.72	14.833	1.72	20.83	0.65
2.917	0.65	8.917	1.72	14.917	1.72	20.92	0.65
3.000	0.65	9.000	1.72	15.000	1.72	21.00	0.65
3.083	0.86	9.083	1.72	15.083	1.51	21.08	0.65
3.167	0.86	9.167	1.72	15.167	1.51	21.17	0.65
3.250	0.86	9.250	1.72	15.250	1.51	21.25	0.65
3.333	0.65	9.333	1.94	15.333	1.72	21.33	0.65
3.417	0.65	9.417	1.94	15.417	1.72	21.42	0.65
3.500	0.65	9.500	1.94	15.500	1.72	21.50	0.65
3.583	0.65	9.583	1.94	15.583	1.51	21.58	0.65
3.667	0.65	9.667	1.94	15.667	1.51	21.67	0.65
3.750	0.65	9.750	1.94	15.750	1.51	21.75	0.65
3.833	0.86	9.833	2.37	15.833	1.08	21.83	0.65
3.917	0.86	9.917	2.37	15.917	1.08	21.92	0.65
4.000	0.86	10.000	2.37	16.000	1.08	22.00	0.65
4.083	0.86	10.083	2.59	16.083	0.86	22.08	0.65
4.167	0.86	10.167	2.59	16.167	0.86	22.17	0.65
4.250	0.86	10.250	2.59	16.250	0.86	22.25	0.65
4.333	0.86	10.333	3.23	16.333	1.08	22.33	0.65
4.417	0.86	10.417	3.23	16.417	1.08	22.42	0.65
4.500	0.86	10.500	3.23	16.500	1.08	22.50	0.65
4.583	0.86	10.583	3.45	16.583	0.86	22.58	0.65
4.667	0.86	10.667	3.45	16.667	0.86	22.67	0.65
4.750	0.86	10.750	3.45	16.750	0.86	22.75	0.65
4.833	0.86	10.833	5.17	16.833	1.08	22.83	0.65
4.917	0.86	10.917	5.17	16.917	1.08	22.92	0.65
5.000	0.86	11.000	5.17	17.000	1.08	23.00	0.65
5.083	0.86	11.083	5.17	17.083	0.86	23.08	0.65
5.167	0.86	11.167	5.17	17.167	0.86	23.17	0.65
5.250	0.86	11.250	5.17	17.250	0.86	23.25	0.65
5.333	0.86	11.333	15.95	17.333	1.08	23.33	0.65
5.417	0.86	11.417	15.95	17.417	1.08	23.42	0.65
5.500	0.86	11.500	15.95	17.500	1.08	23.50	0.65
5.583	0.86	11.583	65.97	17.583	0.86	23.58	0.65
5.667	0.86	11.667	65.97	17.667	0.86	23.67	0.65
5.750	0.86	11.750	65.97	17.750	0.86	23.75	0.65
5.833	0.86	11.833	7.77	17.833	1.08		
5.917	0.86	11.917	7.76	17.917	1.08		
6.000	0.86	12.000	7.76	18.000	1.08		

Max. Eff. Inten. (mm/hr)= 65.97 79.19  
 over (min)= 5.00 15.00  
 Storage Coeff. (min)= 3.38 (ii) 12.92 (ii)  
 Unit Hyd. Tpeak (min)= 5.00 15.00

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Unit Hyd. peak (cms)=	0.26	0.08	*TOTALS*
PEAK FLOW (cms)=	0.08	0.16	0.217 (iii)
TIME TO PEAK (hrs)=	11.75	11.83	11.75
RUNOFF VOLUME (mm)=	52.74	14.47	22.51
TOTAL RAINFALL (mm)=	53.74	53.74	53.74
RUNOFF COEFFICIENT =	0.98	0.27	0.42

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR( 0301)  
 IN= 2---> OUT= 1  
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

INFLOW : ID= 2 ( 0201)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 ( 0301)	2.200	0.217	11.75	22.51
	2.200	0.000	0.00	0.00

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.00  
 TIME SHIFT OF PEAK FLOW (min) = \*\*\*\*\*  
 MAXIMUM STORAGE USED (ha.m.) = 0.0219

ADD HYD ( 0059)  
 1 + 2 = 3

ID1= 1 ( 0301):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
+ ID2= 2 ( 0058):	2.20	0.000	0.00	0.00
	5.16	0.064	11.83	5.39
=====				
ID = 3 ( 0059):	7.36	0.064	11.83	3.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 STANDHYD ( 0205)  
 ID= 1 DT= 5.0 min

Area (ha)=	0.03
Total Imp(%)=	56.00
Dir. Conn. (%)=	28.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.02	0.01
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	1.00
Length (m)=	14.14	20.00
Mannings n =	0.013	0.250

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Max. Eff. Inten. (mm/hr)=	65.97	97.48
over (min)=	5.00	10.00
Storage Coeff. (min)=	0.93 (ii)	6.72 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.34	0.14

\*TOTALS\*

PEAK FLOW (cms)=	0.00	0.00	0.005 (iii)
TIME TO PEAK (hrs)=	11.75	11.75	11.75
RUNOFF VOLUME (mm)=	52.74	17.06	23.42
TOTAL RAINFALL (mm)=	53.74	53.74	53.74
RUNOFF COEFFICIENT =	0.98	0.32	0.44

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0060)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0205):	0.03	0.005	11.75	23.42
+ ID2= 2 ( 0059):	7.36	0.064	11.83	3.78
ID = 3 ( 0060):	7.39	0.067	11.83	3.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)=	0.78	Curve Number (CN)=	66.3
NASHYD ( 0207)	la	(mm)=	4.87	# of Linear Res. (N)=	3.00
ID= 1 DT= 5.0 min	U. H. Tp(hrs)=	0.19			

Unit Hyd Qpeak	(cms)=	0.157
PEAK FLOW	(cms)=	0.027 (i)
TIME TO PEAK	(hrs)=	11.833
RUNOFF VOLUME	(mm)=	13.387
TOTAL RAINFALL	(mm)=	53.738
RUNOFF COEFFICIENT	=	0.249

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0061)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0207):	0.78	0.027	11.83	13.39
+ ID2= 2 ( 0060):	7.39	0.067	11.83	3.86
ID = 3 ( 0061):	8.17	0.093	11.83	4.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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CALIB	Area	(ha)=	0.73	Dir. Conn. (%)=	13.00
STANDHYD ( 0206)	Total Imp(%)=	32.00			
ID= 1 DT= 5.0 min					

	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)= 0.23	0.50
Dep. Storage	(mm)= 1.00	1.50
Average Slope	(%)= 1.00	2.00
Length	(m)= 69.76	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)=	65.97	71.30
over (min)=	5.00	15.00
Storage Coeff. (min)=	2.43 (ii)	10.51 (ii)
Unit Hyd. Tpeak (min)=	5.00	15.00
Unit Hyd. peak (cms)=	0.30	0.09

\*TOTALS\*

PEAK FLOW (cms)=	0.02	0.06	0.067 (iii)
TIME TO PEAK (hrs)=	11.75	11.83	11.75
RUNOFF VOLUME (mm)=	52.74	13.20	18.34
TOTAL RAINFALL (mm)=	53.74	53.74	53.74
RUNOFF COEFFICIENT =	0.98	0.25	0.34

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 \*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0401)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0206):	0.73	0.067	11.75	18.34
+ ID2= 2 ( 0061):	8.17	0.093	11.83	4.77
ID = 3 ( 0401):	8.90	0.157	11.83	5.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V	V	I	SSSS	U	U	A	L
	V	I	SS	U	U	A A	L
V	V	I	SS	U	U	AAAAA	L
V	V	I	SS	U	U	A A	L
VV		I	SSSS	UUUUU	A	A	LLLLL

000	TTTT	TTTT	H	H	Y	Y	M	M	000	TM
0	0	T	T	H	H	Y Y	MM	MM	0	0
0	0	T	T	H	H	Y	M	M	0	0

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SCS.txt  
000 T T H H Y M M 000  
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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\V02\voir.n.dat

Output filename:  
C:\Users\aschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\7f5e0  
c8e-c343-4098-8160-ff3683dfafae\scen  
Summary filename:  
C:\Users\aschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\7f5e0  
c8e-c343-4098-8160-ff3683dfafae\scen

DATE: 10/09/2020 TIME: 05:31:52

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 02 \*\*  
\*\*\*\*\*

MASS STORM

Ptotal = 71.80 mm

Filename: C:\Users\aschoof\AppData\Local\Temp\  
d21eaacb-4cf1-4504-bf53-d3c44604e13d\9fd102dc  
Comments: SCS Type II 24 HR MASS CURVE

Duration of storm = 23.75 hrs  
Mass curve time step = 15.00 min

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.86	6.25	1.44	12.25	10.34	18.25	1.15
0.50	0.86	6.50	1.15	12.50	5.46	18.50	1.44
0.75	0.86	6.75	1.44	12.75	5.17	18.75	1.15
1.00	0.86	7.00	1.44	13.00	4.02	19.00	1.44
1.25	0.86	7.25	1.72	13.25	3.73	19.25	1.15
1.50	0.57	7.50	1.44	13.50	3.16	19.50	1.44
1.75	0.86	7.75	1.72	13.75	2.87	19.75	1.15
2.00	0.86	8.00	1.72	14.00	2.30	20.00	0.86
2.25	1.15	8.25	2.01	14.25	2.01	20.25	0.86
2.50	0.86	8.50	2.01	14.50	2.30	20.50	0.86
2.75	0.86	8.75	2.01	14.75	2.01	20.75	0.86
3.00	0.86	9.00	2.30	15.00	2.30	21.00	0.86
3.25	1.15	9.25	2.30	15.25	2.01	21.25	0.86
3.50	0.86	9.50	2.58	15.50	2.30	21.50	0.86
3.75	0.86	9.75	2.58	15.75	2.01	21.75	0.86
4.00	1.15	10.00	3.16	16.00	1.44	22.00	0.86
4.25	1.15	10.25	3.45	16.25	1.15	22.25	0.86
4.50	1.15	10.50	4.31	16.50	1.44	22.50	0.86

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4.75	1.15	10.75	4.60	16.75	1.15	22.75	0.86
5.00	1.15	11.00	6.89	17.00	1.44	23.00	0.86
5.25	1.15	11.25	6.89	17.25	1.15	23.25	0.86
5.50	1.15	11.50	21.25	17.50	1.44	23.50	0.86
5.75	1.15	11.75	87.88	17.75	1.15	23.75	0.86
6.00	1.15	12.00	10.34	18.00	1.44		

CALIB  
NASHYD ( 0202)  
ID= 1 DT= 2.0 min  
Area (ha)= 1.63  
Curve Number (CN)= 44.1  
# of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.033	0.86	6.000	1.15	11.967	10.34	17.93	1.44
0.067	0.86	6.033	1.44	12.000	10.34	17.97	1.44
0.100	0.86	6.067	1.44	12.033	10.34	18.00	1.44
0.133	0.86	6.100	1.44	12.067	10.34	18.03	1.15
0.167	0.86	6.133	1.44	12.100	10.34	18.07	1.15
0.200	0.86	6.167	1.44	12.133	10.34	18.10	1.15
0.233	0.86	6.200	1.44	12.167	10.34	18.13	1.15
0.267	0.72	6.233	1.44	12.200	10.34	18.17	1.15
0.300	0.57	6.267	1.29	12.233	10.34	18.20	1.15
0.333	0.57	6.300	1.15	12.267	7.89	18.23	1.15
0.367	0.57	6.333	1.15	12.300	5.46	18.27	1.29
0.400	0.57	6.367	1.15	12.333	5.46	18.30	1.44
0.433	0.57	6.400	1.15	12.367	5.46	18.33	1.44
0.467	0.57	6.433	1.15	12.400	5.46	18.37	1.44
0.500	0.57	6.467	1.15	12.433	5.46	18.40	1.44
0.533	0.86	6.500	1.15	12.467	5.46	18.43	1.44
0.567	0.86	6.533	1.44	12.500	5.46	18.47	1.44
0.600	0.86	6.567	1.44	12.533	5.17	18.50	1.44
0.633	0.86	6.600	1.44	12.567	5.17	18.53	1.15
0.667	0.86	6.633	1.44	12.600	5.17	18.57	1.15
0.700	0.86	6.667	1.44	12.633	5.17	18.60	1.15
0.733	0.86	6.700	1.44	12.667	5.17	18.63	1.15
0.767	0.86	6.733	1.44	12.700	5.17	18.67	1.15
0.800	0.86	6.767	1.44	12.733	5.17	18.70	1.15
0.833	0.86	6.800	1.44	12.767	4.59	18.73	1.15
0.867	0.86	6.833	1.44	12.800	4.02	18.77	1.29
0.900	0.86	6.867	1.44	12.833	4.02	18.80	1.44
0.933	0.86	6.900	1.44	12.867	4.02	18.83	1.44
0.967	0.86	6.933	1.44	12.900	4.02	18.87	1.44
1.000	0.86	6.967	1.44	12.933	4.02	18.90	1.44
1.033	0.86	7.000	1.44	12.967	4.02	18.93	1.44
1.067	0.86	7.033	1.72	13.000	4.02	18.97	1.44
1.100	0.86	7.067	1.72	13.033	3.73	19.00	1.44
1.133	0.86	7.100	1.72	13.067	3.73	19.03	1.15
1.167	0.86	7.133	1.72	13.100	3.73	19.07	1.15
1.200	0.86	7.167	1.72	13.133	3.73	19.10	1.15
1.233	0.86	7.200	1.72	13.167	3.73	19.13	1.15
1.267	0.72	7.233	1.72	13.200	3.73	19.17	1.15
1.300	0.57	7.267	1.58	13.233	3.73	19.20	1.15
1.333	0.57	7.300	1.44	13.267	3.45	19.23	1.15
1.367	0.57	7.333	1.44	13.300	3.16	19.27	1.29
1.400	0.57	7.367	1.44	13.333	3.16	19.30	1.44
1.433	0.57	7.400	1.44	13.367	3.16	19.33	1.44

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SCS. txt							
1.467	0.57	7.433	1.44	13.400	3.16	19.37	1.44
1.500	0.57	7.467	1.44	13.433	3.16	19.40	1.44
1.533	0.86	7.500	1.44	13.467	3.16	19.43	1.44
1.567	0.86	7.533	1.72	13.500	3.16	19.47	1.44
1.600	0.86	7.567	1.72	13.533	2.87	19.50	1.44
1.633	0.86	7.600	1.72	13.567	2.87	19.53	1.15
1.667	0.86	7.633	1.72	13.600	2.87	19.57	1.15
1.700	0.86	7.667	1.72	13.633	2.87	19.60	1.15
1.733	0.86	7.700	1.72	13.667	2.87	19.63	1.15
1.767	0.86	7.733	1.72	13.700	2.87	19.67	1.15
1.800	0.86	7.767	1.72	13.733	2.87	19.70	1.15
1.833	0.86	7.800	1.72	13.767	2.58	19.73	1.15
1.867	0.86	7.833	1.72	13.800	2.30	19.77	1.00
1.900	0.86	7.867	1.72	13.833	2.30	19.80	0.86
1.933	0.86	7.900	1.72	13.867	2.30	19.83	0.86
1.967	0.86	7.933	1.72	13.900	2.30	19.87	0.86
2.000	0.86	7.967	1.72	13.933	2.30	19.90	0.86
2.033	1.15	8.000	1.72	13.967	2.30	19.93	0.86
2.067	1.15	8.033	2.01	14.000	2.30	19.97	0.86
2.100	1.15	8.067	2.01	14.033	2.01	20.00	0.86
2.133	1.15	8.100	2.01	14.067	2.01	20.03	0.86
2.167	1.15	8.133	2.01	14.100	2.01	20.07	0.86
2.200	1.15	8.167	2.01	14.133	2.01	20.10	0.86
2.233	1.15	8.200	2.01	14.167	2.01	20.13	0.86
2.267	1.01	8.233	2.01	14.200	2.01	20.17	0.86
2.300	0.86	8.267	2.01	14.233	2.01	20.20	0.86
2.333	0.86	8.300	2.01	14.267	2.15	20.23	0.86
2.367	0.86	8.333	2.01	14.300	2.30	20.27	0.86
2.400	0.86	8.367	2.01	14.333	2.30	20.30	0.86
2.433	0.86	8.400	2.01	14.367	2.30	20.33	0.86
2.467	0.86	8.433	2.01	14.400	2.30	20.37	0.86
2.500	0.86	8.467	2.01	14.433	2.30	20.40	0.86
2.533	0.86	8.500	2.01	14.467	2.30	20.43	0.86
2.567	0.86	8.533	2.01	14.500	2.30	20.47	0.86
2.600	0.86	8.567	2.01	14.533	2.01	20.50	0.86
2.633	0.86	8.600	2.01	14.567	2.01	20.53	0.86
2.667	0.86	8.633	2.01	14.600	2.01	20.57	0.86
2.700	0.86	8.667	2.01	14.633	2.01	20.60	0.86
2.733	0.86	8.700	2.01	14.667	2.01	20.63	0.86
2.767	0.86	8.733	2.01	14.700	2.01	20.67	0.86
2.800	0.86	8.767	2.15	14.733	2.01	20.70	0.86
2.833	0.86	8.800	2.30	14.767	2.15	20.73	0.86
2.867	0.86	8.833	2.30	14.800	2.30	20.77	0.86
2.900	0.86	8.867	2.30	14.833	2.30	20.80	0.86
2.933	0.86	8.900	2.30	14.867	2.30	20.83	0.86
2.967	0.86	8.933	2.30	14.900	2.30	20.87	0.86
3.000	0.86	8.967	2.30	14.933	2.30	20.90	0.86
3.033	1.15	9.000	2.30	14.967	2.30	20.93	0.86
3.067	1.15	9.033	2.30	15.000	2.30	20.97	0.86
3.100	1.15	9.067	2.30	15.033	2.01	21.00	0.86
3.133	1.15	9.100	2.30	15.067	2.01	21.03	0.86
3.167	1.15	9.133	2.30	15.100	2.01	21.07	0.86
3.200	1.15	9.167	2.30	15.133	2.01	21.10	0.86
3.233	1.15	9.200	2.30	15.167	2.01	21.13	0.86
3.267	1.01	9.233	2.30	15.200	2.01	21.17	0.86
3.300	0.86	9.267	2.44	15.233	2.01	21.20	0.86
3.333	0.86	9.300	2.58	15.267	2.15	21.23	0.86
3.367	0.86	9.333	2.58	15.300	2.30	21.27	0.86
3.400	0.86	9.367	2.58	15.333	2.30	21.30	0.86
3.433	0.86	9.400	2.58	15.367	2.30	21.33	0.86
3.467	0.86	9.433	2.58	15.400	2.30	21.37	0.86
3.500	0.86	9.467	2.58	15.433	2.30	21.40	0.86
3.533	0.86	9.500	2.58	15.467	2.30	21.43	0.86

SCS. txt							
3.567	0.86	9.533	2.58	15.500	2.30	21.47	0.86
3.600	0.86	9.567	2.58	15.533	2.01	21.50	0.86
3.633	0.86	9.600	2.58	15.567	2.01	21.53	0.86
3.667	0.86	9.633	2.58	15.600	2.01	21.57	0.86
3.700	0.86	9.667	2.58	15.633	2.01	21.60	0.86
3.733	0.86	9.700	2.58	15.667	2.01	21.63	0.86
3.767	1.01	9.733	2.58	15.700	2.01	21.67	0.86
3.800	1.15	9.767	2.87	15.733	2.01	21.70	0.86
3.833	1.15	9.800	3.16	15.767	1.72	21.73	0.86
3.867	1.15	9.833	3.16	15.800	1.44	21.77	0.86
3.900	1.15	9.867	3.16	15.833	1.44	21.80	0.86
3.933	1.15	9.900	3.16	15.867	1.44	21.83	0.86
3.967	1.15	9.933	3.16	15.900	1.44	21.87	0.86
4.000	1.15	9.967	3.16	15.933	1.44	21.90	0.86
4.033	1.15	10.000	3.16	15.967	1.44	21.93	0.86
4.067	1.15	10.033	3.45	16.000	1.44	21.97	0.86
4.100	1.15	10.067	3.45	16.033	1.15	22.00	0.86
4.133	1.15	10.100	3.45	16.067	1.15	22.03	0.86
4.167	1.15	10.133	3.45	16.100	1.15	22.07	0.86
4.200	1.15	10.167	3.45	16.133	1.15	22.10	0.86
4.233	1.15	10.200	3.45	16.167	1.15	22.13	0.86
4.267	1.15	10.233	3.45	16.200	1.15	22.17	0.86
4.300	1.15	10.267	3.88	16.233	1.15	22.20	0.86
4.333	1.15	10.300	4.31	16.267	1.29	22.23	0.86
4.367	1.15	10.333	4.31	16.300	1.44	22.27	0.86
4.400	1.15	10.367	4.31	16.333	1.44	22.30	0.86
4.433	1.15	10.400	4.31	16.367	1.44	22.33	0.86
4.467	1.15	10.433	4.31	16.400	1.44	22.37	0.86
4.500	1.15	10.467	4.31	16.433	1.44	22.40	0.86
4.533	1.15	10.500	4.31	16.467	1.44	22.43	0.86
4.567	1.15	10.533	4.60	16.500	1.44	22.47	0.86
4.600	1.15	10.567	4.60	16.533	1.15	22.50	0.86
4.633	1.15	10.600	4.60	16.567	1.15	22.53	0.86
4.667	1.15	10.633	4.60	16.600	1.15	22.57	0.86
4.700	1.15	10.667	4.60	16.633	1.15	22.60	0.86
4.733	1.15	10.700	4.60	16.667	1.15	22.63	0.86
4.767	1.15	10.733	4.60	16.700	1.15	22.67	0.86
4.800	1.15	10.767	5.75	16.733	1.15	22.70	0.86
4.833	1.15	10.800	6.89	16.767	1.29	22.73	0.86
4.867	1.15	10.833	6.89	16.800	1.44	22.77	0.86
4.900	1.15	10.867	6.89	16.833	1.44	22.80	0.86
4.933	1.15	10.900	6.89	16.867	1.44	22.83	0.86
4.967	1.15	10.933	6.89	16.900	1.44	22.87	0.86
5.000	1.15	10.967	6.89	16.933	1.44	22.90	0.86
5.033	1.15	11.000	6.89	16.967	1.44	22.93	0.86
5.067	1.15	11.033	6.89	17.000	1.44	22.97	0.86
5.100	1.15	11.067	6.89	17.033	1.15	23.00	0.86
5.133	1.15	11.100	6.89	17.067	1.15	23.03	0.86
5.167	1.15	11.133	6.89	17.100	1.15	23.07	0.86
5.200	1.15	11.167	6.89	17.133	1.15	23.10	0.86
5.233	1.15	11.200	6.89	17.167	1.15	23.13	0.86
5.267	1.15	11.233	6.89	17.200	1.15	23.17	0.86
5.300	1.15	11.267	14.09	17.233	1.15	23.20	0.86
5.333	1.15	11.300	21.25	17.267	1.29	23.23	0.86
5.367	1.15	11.333	21.25	17.300	1.44	23.27	0.86
5.400	1.15	11.367	21.25	17.333	1.44	23.30	0.86
5.433	1.15	11.400	21.25	17.367	1.44	23.33	0.86
5.467	1.15	11.433	21.25	17.400	1.44	23.37	0.86
5.500	1.15	11.467	21.25	17.433	1.44	23.40	0.86
5.533	1.15	11.500	21.33	17.467	1.44	23.43	0.86
5.567	1.15	11.533	87.88	17.500	1.44	23.47	0.86
5.600	1.15	11.567	87.88	17.533	1.15	23.50	0.86
5.633	1.15	11.600	87.88	17.567	1.15	23.53	0.86

SCS: (X)							
5.667	1.15	11.633	87.88	17.600	1.15	23.57	0.86
5.700	1.15	11.667	87.88	17.633	1.15	23.60	0.86
5.733	1.15	11.700	87.88	17.667	1.15	23.63	0.86
5.767	1.15	11.733	87.88	17.700	1.15	23.67	0.86
5.800	1.15	11.767	49.01	17.733	1.15	23.70	0.86
5.833	1.15	11.800	10.34	17.767	1.29	23.73	0.86
5.867	1.15	11.833	10.34	17.800	1.44	23.77	0.43
5.900	1.15	11.867	10.34	17.833	1.44		
5.933	1.15	11.900	10.34	17.867	1.44		
5.967	1.15	11.933	10.34	17.900	1.44		

Unit Hyd Qpeak (cms) = 0.366

PEAK FLOW	(cms)	=	0.043	(i)
TIME TO PEAK	(hrs)	=	11.833	
RUNOFF VOLUME	(mm)	=	10.008	
TOTAL RAINFALL	(mm)	=	71.584	
RUNOFF COEFFICIENT		=	0.140	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALI B	Area (ha)=	2.89	Curve Number (CN)=	42.9
NASHYD ( 0203)	I a (mm)=	8.98	# of Linear Res. (N)=	3.00
ID= 1 DT= 2.0 min	U. H. Tp(hrs)=	0.18		

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW	(cms)	=	0.071	(i)
TIME TO PEAK	(hrs)	=	11.833	
RUNOFF VOLUME	(mm)	=	9.791	
TOTAL RAINFALL	(mm)	=	71.584	
RUNOFF COEFFICIENT		=	0.137	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057)		AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0202):		1.63	0.043	11.83	10.01
+ ID2= 2 ( 0203):		2.89	0.071	11.83	9.79
=====					
ID = 3 ( 0057):		4.52	0.114	11.83	9.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALI B	Area (ha)=	0.64	Curve Number (CN)=	48.1
NASHYD ( 0204)	Ia (mm)=	9.20	# of Linear Res. (N)=	3.00
ID= 1 DT= 2.0 min	U. H. Tp(hrs)=	0.33		

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW	{cms}=	0.012	(i)
TIME TO PEAK	{hrs}=	12.000	
RUNOFF VOLUME	{mm}=	11.558	

SCS. txt

TOTAL RAINFALL (mm) = 71.584  
 RUNOFF COEFFICIENT = 0.161

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD 1 +	HYD 2 =	( 3	0058)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
	ID1=	1	( 0204):	0.64	0.012	12.00	11.56
	ID2=	2	( 0057):	4.52	0.114	11.83	9.87
	ID	3	( 0058):	5.16	0.124	11.83	10.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0201) ID= 1 DT= 5.0 min	Area Total	(ha)= 2.20 Imp(%)= 43.00	Dir. Conn. (%)= 21.00
		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.95	1.25
Dep. Storage	(mm)=	1.00	1.50
Average Slope	(%)=	1.00	1.00
Length	(m)=	121.11	40.00
Manning's n	=	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.86	6.083	1.44	12.083	10.34	18.08	1.15
0.167	0.86	6.167	1.44	12.167	10.34	18.17	1.15
0.250	0.86	6.250	1.44	12.250	10.34	18.25	1.15
0.333	0.57	6.333	1.15	12.333	5.46	18.33	1.44
0.417	0.57	6.417	1.15	12.417	5.46	18.42	1.44
0.500	0.57	6.500	1.15	12.500	5.46	18.50	1.44
0.583	0.86	6.583	1.44	12.583	5.17	18.58	1.15
0.667	0.86	6.667	1.44	12.667	5.17	18.67	1.15
0.750	0.86	6.750	1.44	12.750	5.17	18.75	1.15
0.833	0.86	6.833	1.44	12.833	4.02	18.83	1.44
0.917	0.86	6.917	1.44	12.917	4.02	18.92	1.44
1.000	0.86	7.000	1.44	13.000	4.02	19.00	1.44
1.083	0.86	7.083	1.72	13.083	3.73	19.08	1.15
1.167	0.86	7.167	1.72	13.167	3.73	19.17	1.15
1.250	0.86	7.250	1.72	13.250	3.73	19.25	1.15
1.333	0.57	7.333	1.44	13.333	3.16	19.33	1.44
1.417	0.57	7.417	1.44	13.417	3.16	19.42	1.44
1.500	0.57	7.500	1.44	13.500	3.16	19.50	1.44
1.583	0.86	7.583	1.72	13.583	2.87	19.58	1.15
1.667	0.86	7.667	1.72	13.667	2.87	19.67	1.15
1.750	0.86	7.750	1.72	13.750	2.87	19.75	1.15
1.833	0.86	7.833	1.72	13.833	2.30	19.83	0.86
1.917	0.86	7.917	1.72	13.917	2.30	19.92	0.86
2.000	0.86	8.000	1.72	14.000	2.30	20.00	0.86
2.083	1.15	8.083	2.01	14.083	2.01	20.08	0.86
2.167	1.15	8.167	2.01	14.167	2.01	20.17	0.86

## SCS.txt

2.250	1.15	8.250	2.01	14.250	2.01	20.25	0.86
2.333	0.86	8.333	2.01	14.333	2.30	20.33	0.86
2.417	0.86	8.417	2.01	14.417	2.30	20.42	0.86
2.500	0.86	8.500	2.01	14.500	2.30	20.50	0.86
2.583	0.86	8.583	2.01	14.583	2.01	20.58	0.86
2.667	0.86	8.667	2.01	14.667	2.01	20.67	0.86
2.750	0.86	8.750	2.01	14.750	2.01	20.75	0.86
2.833	0.86	8.833	2.30	14.833	2.30	20.83	0.86
2.917	0.86	8.917	2.30	14.917	2.30	20.92	0.86
3.000	0.86	9.000	2.30	15.000	2.30	21.00	0.86
3.083	1.15	9.083	2.30	15.083	2.01	21.08	0.86
3.167	1.15	9.167	2.30	15.167	2.01	21.17	0.86
3.250	1.15	9.250	2.30	15.250	2.01	21.25	0.86
3.333	0.86	9.333	2.58	15.333	2.30	21.33	0.86
3.417	0.86	9.417	2.58	15.417	2.30	21.42	0.86
3.500	0.86	9.500	2.58	15.500	2.30	21.50	0.86
3.583	0.86	9.583	2.58	15.583	2.01	21.58	0.86
3.667	0.86	9.667	2.58	15.667	2.01	21.67	0.86
3.750	0.86	9.750	2.58	15.750	2.01	21.75	0.86
3.833	1.15	9.833	3.16	15.833	1.44	21.83	0.86
3.917	1.15	9.917	3.16	15.917	1.44	21.92	0.86
4.000	1.15	10.000	3.16	16.000	1.44	22.00	0.86
4.083	1.15	10.083	3.45	16.083	1.15	22.08	0.86
4.167	1.15	10.167	3.45	16.167	1.15	22.17	0.86
4.250	1.15	10.250	3.45	16.250	1.15	22.25	0.86
4.333	1.15	10.333	4.31	16.333	1.44	22.33	0.86
4.417	1.15	10.417	4.31	16.417	1.44	22.42	0.86
4.500	1.15	10.500	4.31	16.500	1.44	22.50	0.86
4.583	1.15	10.583	4.60	16.583	1.15	22.58	0.86
4.667	1.15	10.667	4.60	16.667	1.15	22.67	0.86
4.750	1.15	10.750	4.60	16.750	1.15	22.75	0.86
4.833	1.15	10.833	6.89	16.833	1.44	22.83	0.86
4.917	1.15	10.917	6.89	16.917	1.44	22.92	0.86
5.000	1.15	11.000	6.89	17.000	1.44	23.00	0.86
5.083	1.15	11.083	6.89	17.083	1.15	23.08	0.86
5.167	1.15	11.167	6.89	17.167	1.15	23.17	0.86
5.250	1.15	11.250	6.89	17.250	1.15	23.25	0.86
5.333	1.15	11.333	21.25	17.333	1.44	23.33	0.86
5.417	1.15	11.417	21.25	17.417	1.44	23.42	0.86
5.500	1.15	11.500	21.25	17.500	1.44	23.50	0.86
5.583	1.15	11.583	87.88	17.583	1.15	23.58	0.86
5.667	1.15	11.667	87.88	17.667	1.15	23.67	0.86
5.750	1.15	11.750	87.88	17.750	1.15	23.75	0.86
5.833	1.15	11.833	10.35	17.833	1.44		
5.917	1.15	11.917	10.34	17.917	1.44		
6.000	1.15	12.000	10.34	18.000	1.44		

Max. Eff. Inten. (mm/hr)= 87.88 112.48  
over (min)= 5.00 15.00  
Storage Coeff. (min)= 3.02 (ii) 11.31 (ii)  
Unit Hyd. Tpeak (min)= 5.00 15.00  
Unit Hyd. peak (cms)= 0.27 0.09

PEAK FLOW (cms)= 0.11 0.25 \*TOTALS\*  
TIME TO PEAK (hrs)= 11.75 11.83 0.336 (iii)  
RUNOFF VOLUME (mm)= 70.58 24.66 34.30  
TOTAL RAINFALL (mm)= 71.58 71.58 71.58  
RUNOFF COEFFICIENT = 0.99 0.34 0.48

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00

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## SCS.txt

Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR( 0301)  
IN= 2--> OUT= 1  
DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

INFLOW : ID= 2 ( 0201)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 ( 0301)	2.200	0.336	11.75	34.30
	2.200	0.000	0.00	0.00

PEAK FLOW REDUCTION [Qout/Qi n](%)= 0.00  
TIME SHIFT OF PEAK FLOW (min)=\*\*\*\*\*  
MAXIMUM STORAGE USED (ha. m.)= 0.0339

ADD HYD ( 0059)  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0301):	2.20	0.000	0.00	0.00
+ ID2= 2 ( 0058):	5.16	0.124	11.83	10.08
ID = 3 ( 0059):	7.36	0.124	11.83	7.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0205)  
ID= 1 DT= 5.0 min

Area Total	(ha)=	Imp(%)=	Dir. Conn. (%)=
	0.03	56.00	28.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.02	0.01
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	1.00
Length (m)=	14.14	20.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 87.88 135.56  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 0.83 (ii) 5.91 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.34 0.15

PEAK FLOW (cms)= 0.00 0.00 \*TOTALS\*  
TIME TO PEAK (hrs)= 11.75 11.75 0.006 (iii)  
RUNOFF VOLUME (mm)= 70.58 27.10 11.75  
TOTAL RAINFALL (mm)= 71.58 71.58 71.58  
RUNOFF COEFFICIENT = 0.99 0.38 0.49

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\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0060 ) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0205 ):	0.03	0.006	11.75	34.87
+ ID2= 2 ( 0059 ):	7.36	0.124	11.83	7.07
=====				
ID = 3 ( 0060 ):	7.39	0.128	11.83	7.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ( 0207 ) ID= 1 DT= 5.0 min	Area (ha)= Ia (mm)= U.H. Tp(hrs)=	0.78 4.87 0.19	Curve Number (CN)= 66.3 # of Linear Res. (N)= 3.00
---	---	----------------------	---

Unit Hyd Qpeak (cms)=	0.157
PEAK FLOW (cms)=	0.046 (i)
TIME TO PEAK (hrs)=	11.833
RUNOFF VOLUME (mm)=	22.676
TOTAL RAINFALL (mm)=	71.585
RUNOFF COEFFICIENT	= 0.317

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0061 ) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0207 ):	0.78	0.046	11.83	22.68
+ ID2= 2 ( 0060 ):	7.39	0.128	11.83	7.18
=====				
ID = 3 ( 0061 ):	8.17	0.173	11.83	8.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0206 ) ID= 1 DT= 5.0 min	Area (ha)= Total Imp(%)=	0.73 32.00	Dir. Conn. (%)= 13.00
---	-----------------------------	---------------	-----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.23	0.50
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	69.76	40.00

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	SCS.txt	
Mannings n	=	0.013 0.250
Max. Eff. Inten. (mm/hr)=	87.88	102.37
over (min)	5.00	10.00
Storage Coeff. (min)=	2.17 (ii)	9.16 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.31	0.12
*TOTALS*		
PEAK FLOW (cms)=	0.02	0.11 0.129 (iii)
TIME TO PEAK (hrs)=	11.75	11.75 11.75
RUNOFF VOLUME (mm)=	70.58	23.43 29.56
TOTAL RAINFALL (mm)=	71.58	71.58 71.58
RUNOFF COEFFICIENT	=	0.99 0.33 0.41

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0401 ) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0206 ):	0.73	0.129	11.75	29.56
+ ID2= 2 ( 0061 ):	8.17	0.173	11.83	8.66
=====				
ID = 3 ( 0401 ):	8.90	0.281	11.80	10.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLL
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000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y MM MM 0 0
0 0 T T H H Y M M 0 0
000 T T H H Y M M 000
```

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\V02\vo1n.dat

Output filename:

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SCS.txt  
C:\Users\aschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\e11a2  
aee-0f12-477c-b4d8-201ce77718da\scen  
Summary file name:  
C:\Users\aschoof\AppData\Local\Ci vi ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\e11a2  
aee-0f12-477c-b4d8-201ce77718da\scen

DATE: 10/09/2020 TIME: 05:31:52

USER:

COMMENTS:

\*\*\*\*\*  
\*\* SIMULATION : Run 03 \*\*  
\*\*\*\*\*

MASS STORM

Filename: C:\Users\aschoof\AppData\Local\Temp\  
d21eaacb-4cf1-4504-bf53-d3c44604e13d\7057d6bc  
Comments: SCS Type II 24 HR MASS CURVE

Duration of storm = 23.75 hrs  
Mass curve time step = 15.00 min

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	1.01	6.25	1.68	12.25	12.07	18.25	1.34
0.50	0.67	6.50	1.34	12.50	6.37	18.50	1.68
0.75	1.01	6.75	1.68	12.75	6.03	18.75	1.34
1.00	1.01	7.00	1.68	13.00	4.69	19.00	1.68
1.25	1.01	7.25	2.01	13.25	4.36	19.25	1.34
1.50	0.67	7.50	1.68	13.50	3.69	19.50	1.68
1.75	1.01	7.75	2.01	13.75	3.35	19.75	1.34
2.00	1.01	8.00	2.01	14.00	2.68	20.00	1.01
2.25	1.34	8.25	2.35	14.25	2.35	20.25	1.01
2.50	1.01	8.50	2.35	14.50	2.68	20.50	1.01
2.75	1.01	8.75	2.35	14.75	2.35	20.75	1.01
3.00	1.01	9.00	2.68	15.00	2.68	21.00	1.01
3.25	1.34	9.25	2.68	15.25	2.35	21.25	1.01
3.50	1.01	9.50	3.02	15.50	2.68	21.50	1.01
3.75	1.01	9.75	3.02	15.75	2.35	21.75	1.01
4.00	1.34	10.00	3.69	16.00	1.68	22.00	1.01
4.25	1.34	10.25	4.02	16.25	1.34	22.25	1.01
4.50	1.34	10.50	5.03	16.50	1.68	22.50	1.01
4.75	1.34	10.75	5.36	16.75	1.34	22.75	1.01
5.00	1.34	11.00	8.04	17.00	1.68	23.00	1.01
5.25	1.34	11.25	8.04	17.25	1.34	23.25	1.01
5.50	1.34	11.50	24.80	17.50	1.68	23.50	1.01
5.75	1.34	11.75	102.57	17.75	1.34	23.75	1.01
6.00	1.34	12.00	12.07	18.00	1.68		

CALIB  
NASHYD ( 0202)  
ID= 1 DT= 2.0 min

Area (ha)= 1.63 Curve Number (CN)= 44.1  
la (mm)= 9.62 # of Linear Res. (N)= 3.00  
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SCS.txt  
U. H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----							
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.033	1.01	6.000	1.34	11.967	12.07	17.93	1.68
0.067	1.01	6.033	1.68	12.000	12.07	17.97	1.68
0.100	1.01	6.067	1.68	12.033	12.07	18.00	1.68
0.133	1.01	6.100	1.68	12.067	12.07	18.03	1.34
0.167	1.01	6.133	1.68	12.100	12.07	18.07	1.34
0.200	1.01	6.167	1.68	12.133	12.07	18.10	1.34
0.233	1.01	6.200	1.68	12.167	12.07	18.13	1.34
0.267	0.84	6.233	1.68	12.200	12.07	18.17	1.34
0.300	0.67	6.267	1.51	12.233	12.07	18.20	1.34
0.333	0.67	6.300	1.34	12.267	9.21	18.23	1.34
0.367	0.67	6.333	1.34	12.300	6.37	18.27	1.51
0.400	0.67	6.367	1.34	12.333	6.37	18.30	1.68
0.433	0.67	6.400	1.34	12.367	6.37	18.33	1.68
0.467	0.67	6.433	1.34	12.400	6.37	18.37	1.68
0.500	0.67	6.467	1.34	12.433	6.37	18.40	1.68
0.533	1.01	6.500	1.34	12.467	6.37	18.43	1.68
0.567	1.01	6.533	1.68	12.500	6.37	18.47	1.68
0.600	1.01	6.567	1.68	12.533	6.03	18.50	1.68
0.633	1.01	6.600	1.68	12.567	6.03	18.53	1.34
0.667	1.01	6.633	1.68	12.600	6.03	18.57	1.34
0.700	1.01	6.667	1.68	12.633	6.03	18.60	1.34
0.733	1.01	6.700	1.68	12.667	6.03	18.63	1.34
0.767	1.01	6.733	1.68	12.700	6.03	18.67	1.34
0.800	1.01	6.767	1.68	12.733	6.03	18.70	1.34
0.833	1.01	6.800	1.68	12.767	5.36	18.73	1.34
0.867	1.01	6.833	1.68	12.800	4.69	18.77	1.51
0.900	1.01	6.867	1.68	12.833	4.69	18.80	1.68
0.933	1.01	6.900	1.68	12.867	4.69	18.83	1.68
0.967	1.01	6.933	1.68	12.900	4.69	18.87	1.68
1.000	1.01	6.967	1.68	12.933	4.69	18.90	1.68
1.033	1.01	7.000	1.68	12.967	4.69	18.93	1.68
1.067	1.01	7.033	2.01	13.000	4.69	18.97	1.68
1.100	1.01	7.067	2.01	13.033	4.36	19.00	1.68
1.133	1.01	7.100	2.01	13.067	4.36	19.03	1.34
1.167	1.01	7.133	2.01	13.100	4.36	19.07	1.34
1.200	1.01	7.167	2.01	13.133	4.36	19.10	1.34
1.233	1.01	7.200	2.01	13.167	4.36	19.13	1.34
1.267	0.84	7.233	2.01	13.200	4.36	19.17	1.34
1.300	0.67	7.267	1.84	13.233	4.36	19.20	1.34
1.333	0.67	7.300	1.68	13.267	4.02	19.23	1.34
1.367	0.67	7.333	1.68	13.300	3.69	19.27	1.51
1.400	0.67	7.367	1.68	13.333	3.69	19.30	1.68
1.433	0.67	7.400	1.68	13.367	3.69	19.33	1.68
1.467	0.67	7.433	1.68	13.400	3.69	19.37	1.68
1.500	0.67	7.467	1.68	13.433	3.69	19.40	1.68
1.533	1.01	7.500	1.68	13.467	3.69	19.43	1.68
1.567	1.01	7.533	2.01	13.500	3.69	19.47	1.68
1.600	1.01	7.567	2.01	13.533	3.35	19.50	1.68
1.633	1.01	7.600	2.01	13.567	3.35	19.53	1.34
1.667	1.01	7.633	2.01	13.600	3.35	19.57	1.34
1.700	1.01	7.667	2.01	13.633	3.35	19.60	1.34
1.733	1.01	7.700	2.01	13.667	3.35	19.63	1.34
1.767	1.01	7.733	2.01	13.700	3.35	19.67	1.34
1.800	1.01	7.767	2.01	13.733	3.35	19.70	1.34
1.833	1.01	7.800	2.01	13.767	3.02	19.73	1.34

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SCS.txt							
1.867	1.01	7.833	2.01	13.800	2.68	19.77	1.17
1.900	1.01	7.867	2.01	13.833	2.68	19.80	1.01
1.933	1.01	7.900	2.01	13.867	2.68	19.83	1.01
1.967	1.01	7.933	2.01	13.900	2.68	19.87	1.01
2.000	1.01	7.967	2.01	13.933	2.68	19.90	1.01
2.033	1.34	8.000	2.01	13.967	2.68	19.93	1.01
2.067	1.34	8.033	2.35	14.000	2.68	19.97	1.01
2.100	1.34	8.067	2.35	14.033	2.35	20.00	1.01
2.133	1.34	8.100	2.35	14.067	2.35	20.03	1.01
2.167	1.34	8.133	2.35	14.100	2.35	20.07	1.01
2.200	1.34	8.167	2.35	14.133	2.35	20.10	1.01
2.233	1.34	8.200	2.35	14.167	2.35	20.13	1.01
2.267	1.17	8.233	2.35	14.200	2.35	20.17	1.01
2.300	1.01	8.267	2.35	14.233	2.35	20.20	1.01
2.333	1.01	8.300	2.35	14.267	2.51	20.23	1.01
2.367	1.01	8.333	2.35	14.300	2.68	20.27	1.01
2.400	1.01	8.367	2.35	14.333	2.68	20.30	1.01
2.433	1.01	8.400	2.35	14.367	2.68	20.33	1.01
2.467	1.01	8.433	2.35	14.400	2.68	20.37	1.01
2.500	1.01	8.467	2.35	14.433	2.68	20.40	1.01
2.533	1.01	8.500	2.35	14.467	2.68	20.43	1.01
2.567	1.01	8.533	2.35	14.500	2.68	20.47	1.01
2.600	1.01	8.567	2.35	14.533	2.35	20.50	1.01
2.633	1.01	8.600	2.35	14.567	2.35	20.53	1.01
2.667	1.01	8.633	2.35	14.600	2.35	20.57	1.01
2.700	1.01	8.667	2.35	14.633	2.35	20.60	1.01
2.733	1.01	8.700	2.35	14.667	2.35	20.63	1.01
2.767	1.01	8.733	2.35	14.700	2.35	20.67	1.01
2.800	1.01	8.767	2.51	14.733	2.35	20.70	1.01
2.833	1.01	8.800	2.68	14.767	2.51	20.73	1.01
2.867	1.01	8.833	2.68	14.800	2.68	20.77	1.01
2.900	1.01	8.867	2.68	14.833	2.68	20.80	1.01
2.933	1.01	8.900	2.68	14.867	2.68	20.83	1.01
2.967	1.01	8.933	2.68	14.900	2.68	20.87	1.01
3.000	1.01	8.967	2.68	14.933	2.68	20.90	1.01
3.033	1.34	9.000	2.68	14.967	2.68	20.93	1.01
3.067	1.34	9.033	2.68	15.000	2.68	20.97	1.01
3.100	1.34	9.067	2.68	15.033	2.35	21.00	1.01
3.133	1.34	9.100	2.68	15.067	2.35	21.03	1.01
3.167	1.34	9.133	2.68	15.100	2.35	21.07	1.01
3.200	1.34	9.167	2.68	15.133	2.35	21.10	1.01
3.233	1.34	9.200	2.68	15.167	2.35	21.13	1.01
3.267	1.17	9.233	2.68	15.200	2.35	21.17	1.01
3.300	1.01	9.267	2.85	15.233	2.35	21.20	1.01
3.333	1.01	9.300	3.02	15.267	2.51	21.23	1.01
3.367	1.01	9.333	3.02	15.300	2.68	21.27	1.01
3.400	1.01	9.367	3.02	15.333	2.68	21.30	1.01
3.433	1.01	9.400	3.02	15.367	2.68	21.33	1.01
3.467	1.01	9.433	3.02	15.400	2.68	21.37	1.01
3.500	1.01	9.467	3.02	15.433	2.68	21.40	1.01
3.533	1.01	9.500	3.02	15.467	2.68	21.43	1.01
3.567	1.01	9.533	3.02	15.500	2.68	21.47	1.01
3.600	1.01	9.567	3.02	15.533	2.35	21.50	1.01
3.633	1.01	9.600	3.02	15.567	2.35	21.53	1.01
3.667	1.01	9.633	3.02	15.600	2.35	21.57	1.01
3.700	1.01	9.667	3.02	15.633	2.35	21.60	1.01
3.733	1.01	9.700	3.02	15.667	2.35	21.63	1.01
3.767	1.17	9.733	3.02	15.700	2.35	21.67	1.01
3.800	1.34	9.767	3.35	15.733	2.35	21.70	1.01
3.833	1.34	9.800	3.69	15.767	2.01	21.73	1.01
3.867	1.34	9.833	3.69	15.800	1.68	21.77	1.01
3.900	1.34	9.867	3.69	15.833	1.68	21.80	1.01
3.933	1.34	9.900	3.69	15.867	1.68	21.83	1.01

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SCS.txt							
3.967	1.34	9.933	3.69	15.900	1.68	21.87	1.01
4.000	1.34	9.967	3.69	15.933	1.68	21.90	1.01
4.033	1.34	10.000	3.69	15.967	1.68	21.93	1.01
4.067	1.34	10.033	4.02	16.000	1.68	21.97	1.01
4.100	1.34	10.067	4.02	16.033	1.34	22.00	1.01
4.133	1.34	10.100	4.02	16.067	1.34	22.03	1.01
4.167	1.34	10.133	4.02	16.100	1.34	22.07	1.01
4.200	1.34	10.167	4.02	16.133	1.34	22.10	1.01
4.233	1.34	10.200	4.02	16.167	1.34	22.13	1.01
4.267	1.34	10.233	4.02	16.200	1.34	22.17	1.01
4.300	1.34	10.267	4.53	16.233	1.34	22.20	1.01
4.333	1.34	10.300	5.03	16.267	1.51	22.23	1.01
4.367	1.34	10.333	5.03	16.300	1.68	22.27	1.01
4.400	1.34	10.367	5.03	16.333	1.68	22.30	1.01
4.433	1.34	10.400	5.03	16.367	1.68	22.33	1.01
4.467	1.34	10.433	5.03	16.400	1.68	22.37	1.01
4.500	1.34	10.467	5.03	16.433	1.68	22.40	1.01
4.533	1.34	10.500	5.03	16.467	1.68	22.43	1.01
4.567	1.34	10.533	5.36	16.500	1.68	22.47	1.01
4.600	1.34	10.567	5.36	16.533	1.34	22.50	1.01
4.633	1.34	10.600	5.36	16.567	1.34	22.53	1.01
4.667	1.34	10.633	5.36	16.600	1.34	22.57	1.01
4.700	1.34	10.667	5.36	16.633	1.34	22.60	1.01
4.733	1.34	10.700	5.36	16.667	1.34	22.63	1.01
4.767	1.34	10.733	5.36	16.700	1.34	22.67	1.01
4.800	1.34	10.767	6.71	16.733	1.34	22.70	1.01
4.833	1.34	10.800	8.04	16.767	1.51	22.73	1.01
4.867	1.34	10.833	8.04	16.800	1.68	22.77	1.01
4.900	1.34	10.867	8.04	16.833	1.68	22.80	1.01
4.933	1.34	10.900	8.04	16.867	1.68	22.83	1.01
4.967	1.34	10.933	8.04	16.900	1.68	22.87	1.01
5.000	1.34	10.967	8.04	16.933	1.68	22.90	1.01
5.033	1.34	11.000	8.04	16.967	1.68	22.93	1.01
5.067	1.34	11.033	8.04	17.000	1.68	22.97	1.01
5.100	1.34	11.067	8.04	17.033	1.34	23.00	1.01
5.133	1.34	11.100	8.04	17.067	1.34	23.03	1.01
5.167	1.34	11.133	8.04	17.100	1.34	23.07	1.01
5.200	1.34	11.167	8.04	17.133	1.34	23.10	1.01
5.233	1.34	11.200	8.04	17.167	1.34	23.13	1.01
5.267	1.34	11.233	8.04	17.200	1.34	23.17	1.01
5.300	1.34	11.267	16.44	17.233	1.34	23.20	1.01
5.333	1.34	11.300	24.80	17.267	1.51	23.23	1.01
5.367	1.34	11.333	24.80	17.300	1.68	23.27	1.01
5.400	1.34	11.367	24.80	17.333	1.68	23.30	1.01
5.433	1.34	11.400	24.80	17.367	1.68	23.33	1.01
5.467	1.34	11.433	24.80	17.400	1.68	23.37	1.01
5.500	1.34	11.467	24.80	17.433	1.68	23.40	1.01
5.533	1.34	11.500	24.90	17.467	1.68	23.43	1.01
5.567	1.34	11.533	102.57	17.500	1.68	23.47	1.01
5.600	1.34	11.567	102.57	17.533	1.34	23.50	1.01
5.633	1.34	11.600	102.57	17.567	1.34	23.53	1.01
5.667	1.34	11.633	102.57	17.600	1.34	23.57	1.01
5.700	1.34	11.667	102.57	17.633	1.34	23.60	1.01
5.733	1.34	11.700	102.57	17.667	1.34	23.63	1.01
5.767	1.34	11.733	102.57	17.700	1.34	23.67	1.01
5.800	1.34	11.767	57.20	17.733	1.34	23.70	1.01
5.833	1.34	11.800	12.07	17.767	1.51	23.73	1.01
5.867	1.34	11.833	12.07	17.800	1.68	23.77	0.50
5.900	1.34	11.867	12.07	17.833	1.68		
5.933	1.34	11.900	12.07	17.867	1.68		
5.967	1.34	11.933	12.07	17.900	1.68		

Unit Hyd Qpeak (cms)= 0.366

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## SCS.txt

PEAK FLOW (cms)= 0.059 (i)  
 TIME TO PEAK (hrs)= 11.833  
 RUNOFF VOLUME (mm)= 13.815  
 TOTAL RAINFALL (mm)= 83.548  
 RUNOFF COEFFICIENT = 0.165

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0203) ID= 1 DT= 2.0 min	Area (ha)= 2.89 la (mm)= 8.98 U. H. Tp(hrs)= 0.18	Curve Number (CN)= 42.9 # of Linear Res. (N)= 3.00
--	---	---

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.099 (i)  
 TIME TO PEAK (hrs)= 11.833  
 RUNOFF VOLUME (mm)= 13.487  
 TOTAL RAINFALL (mm)= 83.548  
 RUNOFF COEFFICIENT = 0.161

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057) 1 + 2 = 3	AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)	1.63 0.059 11.83 13.82	2.89 0.099 11.83 13.49
------------------------------	---	---------------------------------	---------------------------------

ID1= 1 ( 0202):  
 + ID2= 2 ( 0203):  
 ID = 3 ( 0057):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ( 0204) ID= 1 DT= 2.0 min	Area (ha)= 0.64 la (mm)= 9.20 U. H. Tp(hrs)= 0.33	Curve Number (CN)= 48.1 # of Linear Res. (N)= 3.00
--	---	---

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.017 (i)  
 TIME TO PEAK (hrs)= 12.000  
 RUNOFF VOLUME (mm)= 15.854  
 TOTAL RAINFALL (mm)= 83.548  
 RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0058) 1 + 2 = 3	AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)	0.64 0.017 12.00 15.85
------------------------------	---	---------------------------------

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## SCS.txt

+ ID2= 2 ( 0057): 4.52 0.159 11.83 13.61  
 ID = 3 ( 0058): 5.16 0.173 11.83 13.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0201) ID= 1 DT= 5.0 min	Area (ha)= 2.20 Total Imp(%)= 43.00	Dir. Conn. (%)= 21.00
--	--	-----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.95	1.25
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	1.00
Length (m)=	121.11	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.01	6.083	1.68
0.167	1.01	6.167	1.68
0.250	1.01	6.250	1.68
0.333	0.67	6.333	1.34
0.417	0.67	6.417	1.34
0.500	0.67	6.500	1.34
0.583	1.01	6.583	1.68
0.667	1.01	6.667	1.68
0.750	1.01	6.750	1.68
0.833	1.01	6.833	1.68
0.917	1.01	6.917	1.68
1.000	1.01	7.000	1.68
1.083	1.01	7.083	2.01
1.167	1.01	7.167	2.01
1.250	1.01	7.250	2.01
1.333	0.67	7.333	1.68
1.417	0.67	7.417	1.68
1.500	0.67	7.500	1.68
1.583	1.01	7.583	2.01
1.667	1.01	7.667	2.01
1.750	1.01	7.750	2.01
1.833	1.01	7.833	2.01
1.917	1.01	7.917	2.01
2.000	1.01	8.000	2.01
2.083	1.34	8.083	2.35
2.167	1.34	8.167	2.35
2.250	1.34	8.250	2.35
2.333	1.01	8.333	2.35
2.417	1.01	8.417	2.35
2.500	1.01	8.500	2.35
2.583	1.01	8.583	2.35
2.667	1.01	8.667	2.35
2.750	1.01	8.750	2.35
2.833	1.01	8.833	2.68
2.917	1.01	8.917	2.68
3.000	1.01	9.000	2.68
3.083	1.34	9.083	2.68
3.167	1.34	9.167	2.68

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## SCS.txt

3.250	1.34	9.250	2.68	15.250	2.35	21.25	1.01
3.333	1.01	9.333	3.02	15.333	2.68	21.33	1.01
3.417	1.01	9.417	3.02	15.417	2.68	21.42	1.01
3.500	1.01	9.500	3.02	15.500	2.68	21.50	1.01
3.583	1.01	9.583	3.02	15.583	2.35	21.58	1.01
3.667	1.01	9.667	3.02	15.667	2.35	21.67	1.01
3.750	1.01	9.750	3.02	15.750	2.35	21.75	1.01
3.833	1.34	9.833	3.69	15.833	1.68	21.83	1.01
3.917	1.34	9.917	3.69	15.917	1.68	21.92	1.01
4.000	1.34	10.000	3.69	16.000	1.68	22.00	1.01
4.083	1.34	10.083	4.02	16.083	1.34	22.08	1.01
4.167	1.34	10.167	4.02	16.167	1.34	22.17	1.01
4.250	1.34	10.250	4.02	16.250	1.34	22.25	1.01
4.333	1.34	10.333	5.03	16.333	1.68	22.33	1.01
4.417	1.34	10.417	5.03	16.417	1.68	22.42	1.01
4.500	1.34	10.500	5.03	16.500	1.68	22.50	1.01
4.583	1.34	10.583	5.36	16.583	1.34	22.58	1.01
4.667	1.34	10.667	5.36	16.667	1.34	22.67	1.01
4.750	1.34	10.750	5.36	16.750	1.34	22.75	1.01
4.833	1.34	10.833	8.04	16.833	1.68	22.83	1.01
4.917	1.34	10.917	8.04	16.917	1.68	22.92	1.01
5.000	1.34	11.000	8.04	17.000	1.68	23.00	1.01
5.083	1.34	11.083	8.04	17.083	1.34	23.08	1.01
5.167	1.34	11.167	8.04	17.167	1.34	23.17	1.01
5.250	1.34	11.250	8.04	17.250	1.34	23.25	1.01
5.333	1.34	11.333	24.80	17.333	1.68	23.33	1.01
5.417	1.34	11.417	24.80	17.417	1.68	23.42	1.01
5.500	1.34	11.500	24.80	17.500	1.68	23.50	1.01
5.583	1.34	11.583	102.56	17.583	1.34	23.58	1.01
5.667	1.34	11.667	102.57	17.667	1.34	23.67	1.01
5.750	1.34	11.750	102.57	17.750	1.34	23.75	1.01
5.833	1.34	11.833	12.08	17.833	1.68		
5.917	1.34	11.917	12.07	17.917	1.68		
6.000	1.34	12.000	12.07	18.000	1.68		

Max. Eff. Inten. (mm/hr)= 102.57 133.87  
over (min)= 5.00 15.00  
Storage Coeff. (min)= 2.84 (ii) 10.57 (ii)  
Unit Hyd. Tpeak (min)= 5.00 15.00  
Unit Hyd. peak (cms)= 0.28 0.09

\*TOTALS\*  
0.414 (iii)  
11.75  
42.15  
83.55  
0.50

PEAK FLOW (cms)= 0.13 0.31  
TIME TO PEAK (hrs)= 11.75 11.83  
RUNOFF VOLUME (mm)= 82.55 31.41  
TOTAL RAINFALL (mm)= 83.55 83.55  
RUNOFF COEFFICIENT = 0.99 0.38

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0301)  
IN= 2----> OUT= 1  
DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.1680	0.1590	0.5570

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## SCS.txt

0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
2.200	0.414	11.75	42.15
2.200	0.000	0.00	0.00

INFLOW : ID= 2 ( 0201)  
OUTFLOW: ID= 1 ( 0301)

PEAK FLOW REDUCTION [Qout/Qin](%)= 0.00  
TIME SHIFT OF PEAK FLOW (min)=\*\*\*\*\*  
MAXIMUM STORAGE USED (ha. m.)= 0.0422

ADD HYD ( 0059)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
2.20	0.000	0.00	0.00
5.16	0.173	11.83	13.88
ID = 3 ( 0059):	7.36	0.173	11.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0205)  
ID= 1 DT= 5.0 min

Area (ha)= 0.03  
Total Imp(%)= 56.00 Dir. Conn.(%)= 28.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 0.02	0.01
Dep. Storage (mm)= 1.00	1.50
Average Slope (%)= 1.00	1.00
Length (m)= 14.14	20.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr)= 102.57 160.03  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 0.78 (ii) 5.53 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.34 0.16

PEAK FLOW (cms)= 0.00 0.01  
TIME TO PEAK (hrs)= 11.75 11.75  
RUNOFF VOLUME (mm)= 82.55 34.35  
TOTAL RAINFALL (mm)= 83.55 83.55  
RUNOFF COEFFICIENT = 0.99 0.41

\*TOTALS\*  
0.008 (iii)  
11.75  
43.13  
83.55  
0.52

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.



```

SCS.txt
| ADD HYD ( 0060) |
| 1 + 2 = 3 |
-----
ID1= 1 ( 0205): 0.03 0.008 11.75 43.13
+ ID2= 2 ( 0059): 7.36 0.173 11.83 9.73
=====
ID = 3 ( 0060): 7.39 0.177 11.83 9.87

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD ( 0207) |
ID= 1 DT= 5.0 min |
-----
Area (ha)= 0.78 Curve Number (CN)= 66.3
Ia (mm)= 4.87 # of Linear Res. (N)= 3.00
U. H. Tp(hrs)= 0.19

```

```

Unit Hyd Qpeak (cms)= 0.157
PEAK FLOW (cms)= 0.060 (i)
TIME TO PEAK (hrs)= 11.833
RUNOFF VOLUME (mm)= 29.722
TOTAL RAINFALL (mm)= 83.549
RUNOFF COEFFICIENT = 0.356

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

| ADD HYD ( 0061) |
| 1 + 2 = 3 |
-----
ID1= 1 ( 0207): 0.78 0.060 11.83 29.72
+ ID2= 2 ( 0060): 7.39 0.177 11.83 9.87
=====
ID = 3 ( 0061): 8.17 0.237 11.83 11.76

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
STANDHYD ( 0206) |
ID= 1 DT= 5.0 min |
-----
Area (ha)= 0.73
Total Imp(%)= 32.00 Dir. Conn. (%)= 13.00

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.23 0.50
Dep. Storage (mm)= 1.00 1.50
Average Slope (%)= 1.00 2.00
Length (m)= 69.76 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 102.57 122.50
over (min)= 5.00 10.00
Storage Coeff. (min)= 2.04 (ii) 8.54 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.12

```

```

*TOTALS*
PEAK FLOW (cms)= 0.03 0.13 0.158 (iii)
TIME TO PEAK (hrs)= 11.75 11.75 11.75
RUNOFF VOLUME (mm)= 82.55 29.97 36.80
TOTAL RAINFALL (mm)= 83.55 83.55 83.55

```

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```

SCS.txt
RUNOFF COEFFICIENT = 0.99 0.36 0.44

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 \*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

| ADD HYD ( 0401) |
| 1 + 2 = 3 |
-----
ID1= 1 ( 0206): 0.73 0.158 11.75 36.80
+ ID2= 2 ( 0061): 8.17 0.237 11.83 11.76
=====
ID = 3 ( 0401): 8.90 0.370 11.80 13.82

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

```

V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLL

```

```

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y MM MM 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

```

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\VO2\voinput.dat

Output filename:  
 C:\Users\Naschoof\AppData\Local\Civica\58194ef53-adad-4f15-90f7-c4eafb4675c3\6a9678e7-13ef-4779-b86c-b151dfec042d\scen  
 Summary filename:  
 C:\Users\Naschoof\AppData\Local\Civica\58194ef53-adad-4f15-90f7-c4eafb4675c3\6a9678e7-13ef-4779-b86c-b151dfec042d\scen

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DATE: 10/09/2020 SCS.txt  
TIME: 05:31:52

USER:

COMMENTS:

\*\*\*\*\*  
\*\* SIMULATION : Run 04 \*\*  
\*\*\*\*\*

MASS STORM

Ptotal = 98.90 mm

Filename: C:\Users\aschoof\AppData  
ata\Local\Temp\  
d21eaacb-4cf1-4504-bf53-d3c44604e13d\8ae18c54  
Comments: SCS Type II 24 HR MASS CURVE

Duration of storm = 23.75 hrs  
Mass curve time step = 15.00 min

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.19	6.25	1.98	12.25	14.24	18.25	1.58
0.50	0.79	6.50	1.58	12.50	7.52	18.50	1.98
0.75	1.19	6.75	1.98	12.75	7.12	18.75	1.58
1.00	1.19	7.00	1.98	13.00	5.54	19.00	1.98
1.25	1.19	7.25	2.37	13.25	5.14	19.25	1.58
1.50	0.79	7.50	1.98	13.50	4.35	19.50	1.98
1.75	1.19	7.75	2.37	13.75	3.96	19.75	1.58
2.00	1.19	8.00	2.37	14.00	3.16	20.00	1.19
2.25	1.58	8.25	2.77	14.25	2.77	20.25	1.19
2.50	1.19	8.50	2.77	14.50	3.16	20.50	1.19
2.75	1.19	8.75	2.77	14.75	2.77	20.75	1.19
3.00	1.19	9.00	3.16	15.00	3.16	21.00	1.19
3.25	1.58	9.25	3.16	15.25	2.77	21.25	1.19
3.50	1.19	9.50	3.56	15.50	3.16	21.50	1.19
3.75	1.19	9.75	3.56	15.75	2.77	21.75	1.19
4.00	1.58	10.00	4.35	16.00	1.98	22.00	1.19
4.25	1.58	10.25	4.75	16.25	1.58	22.25	1.19
4.50	1.58	10.50	5.93	16.50	1.98	22.50	1.19
4.75	1.58	10.75	6.33	16.75	1.58	22.75	1.19
5.00	1.58	11.00	9.49	17.00	1.98	23.00	1.19
5.25	1.58	11.25	9.49	17.25	1.58	23.25	1.19
5.50	1.58	11.50	29.27	17.50	1.98	23.50	1.19
5.75	1.58	11.75	121.05	17.75	1.58	23.75	1.19
6.00	1.58	12.00	14.24	18.00	1.98		

CALIB  
NASHYD ( 0202) Area (ha)= 1.63 Curve Number (CN)= 44.1  
ID= 1 DT= 2.0 min Ia (mm)= 9.62 # of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----  
TIME RAIN | TIME RAIN | TIME RAIN |  
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SCS.txt

hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.033	1.19	6.000	1.58	11.967	14.24	17.93	1.98
0.067	1.19	6.033	1.98	12.000	14.24	17.97	1.98
0.100	1.19	6.067	1.98	12.033	14.24	18.00	1.98
0.133	1.19	6.100	1.98	12.067	14.24	18.03	1.58
0.167	1.19	6.133	1.98	12.100	14.24	18.07	1.58
0.200	1.19	6.167	1.98	12.133	14.24	18.10	1.58
0.233	1.19	6.200	1.98	12.167	14.24	18.13	1.58
0.267	0.99	6.233	1.98	12.200	14.24	18.17	1.58
0.300	0.79	6.267	1.78	12.233	14.24	18.20	1.58
0.333	0.79	6.300	1.58	12.267	10.87	18.23	1.58
0.367	0.79	6.333	1.58	12.300	7.52	18.27	1.78
0.400	0.79	6.367	1.58	12.333	7.52	18.30	1.98
0.433	0.79	6.400	1.58	12.367	7.52	18.33	1.98
0.467	0.79	6.433	1.58	12.400	7.52	18.37	1.98
0.500	0.79	6.467	1.58	12.433	7.52	18.40	1.98
0.533	1.19	6.500	1.58	12.467	7.52	18.43	1.98
0.567	1.19	6.533	1.98	12.500	7.52	18.47	1.98
0.600	1.19	6.567	1.98	12.533	7.12	18.50	1.98
0.633	1.19	6.600	1.98	12.567	7.12	18.53	1.58
0.667	1.19	6.633	1.98	12.600	7.12	18.57	1.58
0.700	1.19	6.667	1.98	12.633	7.12	18.60	1.58
0.733	1.19	6.700	1.98	12.667	7.12	18.63	1.58
0.767	1.19	6.733	1.98	12.700	7.12	18.67	1.58
0.800	1.19	6.767	1.98	12.733	7.12	18.70	1.58
0.833	1.19	6.800	1.98	12.767	6.33	18.73	1.58
0.867	1.19	6.833	1.98	12.800	5.54	18.77	1.78
0.900	1.19	6.867	1.98	12.833	5.54	18.80	1.98
0.933	1.19	6.900	1.98	12.867	5.54	18.83	1.98
0.967	1.19	6.933	1.98	12.900	5.54	18.87	1.98
1.000	1.19	6.967	1.98	12.933	5.54	18.90	1.98
1.033	1.19	7.000	1.98	12.967	5.54	18.93	1.98
1.067	1.19	7.033	2.37	13.000	5.54	18.97	1.98
1.100	1.19	7.067	2.37	13.033	5.14	19.00	1.98
1.133	1.19	7.100	2.37	13.067	5.14	19.03	1.58
1.167	1.19	7.133	2.37	13.100	5.14	19.07	1.58
1.200	1.19	7.167	2.37	13.133	5.14	19.10	1.58
1.233	1.19	7.200	2.37	13.167	5.14	19.13	1.58
1.267	0.99	7.233	2.37	13.200	5.14	19.17	1.58
1.300	0.79	7.267	2.18	13.233	5.14	19.20	1.58
1.333	0.79	7.300	1.98	13.267	4.75	19.23	1.58
1.367	0.79	7.333	1.98	13.300	4.35	19.27	1.78
1.400	0.79	7.367	1.98	13.333	4.35	19.30	1.98
1.433	0.79	7.400	1.98	13.367	4.35	19.33	1.98
1.467	0.79	7.433	1.98	13.400	4.35	19.37	1.98
1.500	0.79	7.467	1.98	13.433	4.35	19.40	1.98
1.533	1.19	7.500	1.98	13.467	4.35	19.43	1.98
1.567	1.19	7.533	2.37	13.500	4.35	19.47	1.98
1.600	1.19	7.567	2.37	13.533	3.96	19.50	1.98
1.633	1.19	7.600	2.37	13.567	3.96	19.53	1.58
1.667	1.19	7.633	2.37	13.600	3.96	19.57	1.58
1.700	1.19	7.667	2.37	13.633	3.96	19.60	1.58
1.733	1.19	7.700	2.37	13.667	3.96	19.63	1.58
1.767	1.19	7.733	2.37	13.700	3.96	19.67	1.58
1.800	1.19	7.767	2.37	13.733	3.96	19.70	1.58
1.833	1.19	7.800	2.37	13.767	3.56	19.73	1.58
1.867	1.19	7.833	2.37	13.800	3.16	19.77	1.38
1.900	1.19	7.867	2.37	13.833	3.16	19.80	1.19
1.933	1.19	7.900	2.37	13.867	3.16	19.83	1.19
1.967	1.19	7.933	2.37	13.900	3.16	19.87	1.19
2.000	1.19	7.967	2.37	13.933	3.16	19.90	1.19
2.033	1.58	8.000	2.37	13.967	3.16	19.93	1.19
2.067	1.58	8.033	2.77	14.000	3.16	19.97	1.19

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2.100	1.58	8.067	2.77	14.033	2.77	20.00	1.19		
2.133	1.58	8.100	2.77	14.067	2.77	20.03	1.19		
2.167	1.58	8.133	2.77	14.100	2.77	20.07	1.19		
2.200	1.58	8.167	2.77	14.133	2.77	20.10	1.19		
2.233	1.58	8.200	2.77	14.167	2.77	20.13	1.19		
2.267	1.38	8.233	2.77	14.200	2.77	20.17	1.19		
2.300	1.19	8.267	2.77	14.233	2.77	20.20	1.19		
2.333	1.19	8.300	2.77	14.267	2.97	20.23	1.19		
2.367	1.19	8.333	2.77	14.300	3.16	20.27	1.19		
2.400	1.19	8.367	2.77	14.333	3.16	20.30	1.19		
2.433	1.19	8.400	2.77	14.367	3.16	20.33	1.19		
2.467	1.19	8.433	2.77	14.400	3.16	20.37	1.19		
2.500	1.19	8.467	2.77	14.433	3.16	20.40	1.19		
2.533	1.19	8.500	2.77	14.467	3.16	20.43	1.19		
2.567	1.19	8.533	2.77	14.500	3.16	20.47	1.19		
2.600	1.19	8.567	2.77	14.533	2.77	20.50	1.19		
2.633	1.19	8.600	2.77	14.567	2.77	20.53	1.19		
2.667	1.19	8.633	2.77	14.600	2.77	20.57	1.19		
2.700	1.19	8.667	2.77	14.633	2.77	20.60	1.19		
2.733	1.19	8.700	2.77	14.667	2.77	20.63	1.19		
2.767	1.19	8.733	2.77	14.700	2.77	20.67	1.19		
2.800	1.19	8.767	2.97	14.733	2.77	20.70	1.19		
2.833	1.19	8.800	3.16	14.767	2.97	20.73	1.19		
2.867	1.19	8.833	3.16	14.800	3.16	20.77	1.19		
2.900	1.19	8.867	3.16	14.833	3.16	20.80	1.19		
2.933	1.19	8.900	3.16	14.867	3.16	20.83	1.19		
2.967	1.19	8.933	3.16	14.900	3.16	20.87	1.19		
3.000	1.19	8.967	3.16	14.933	3.16	20.90	1.19		
3.033	1.58	9.000	3.16	14.967	3.16	20.93	1.19		
3.067	1.58	9.033	3.16	15.000	3.16	20.97	1.19		
3.100	1.58	9.067	3.16	15.033	2.77	21.00	1.19		
3.133	1.58	9.100	3.16	15.067	2.77	21.03	1.19		
3.167	1.58	9.133	3.16	15.100	2.77	21.07	1.19		
3.200	1.58	9.167	3.16	15.133	2.77	21.10	1.19		
3.233	1.58	9.200	3.16	15.167	2.77	21.13	1.19		
3.267	1.38	9.233	3.16	15.200	2.77	21.17	1.19		
3.300	1.19	9.267	3.36	15.233	2.77	21.20	1.19		
3.333	1.19	9.300	3.56	15.267	2.97	21.23	1.19		
3.367	1.19	9.333	3.56	15.300	3.16	21.27	1.19		
3.400	1.19	9.367	3.56	15.333	3.16	21.30	1.19		
3.433	1.19	9.400	3.56	15.367	3.16	21.33	1.19		
3.467	1.19	9.433	3.56	15.400	3.16	21.37	1.19		
3.500	1.19	9.467	3.56	15.433	3.16	21.40	1.19		
3.533	1.19	9.500	3.56	15.467	3.16	21.43	1.19		
3.567	1.19	9.533	3.56	15.500	3.16	21.47	1.19		
3.600	1.19	9.567	3.56	15.533	2.77	21.50	1.19		
3.633	1.19	9.600	3.56	15.567	2.77	21.53	1.19		
3.667	1.19	9.633	3.56	15.600	2.77	21.57	1.19		
3.700	1.19	9.667	3.56	15.633	2.77	21.60	1.19		
3.733	1.19	9.700	3.56	15.667	2.77	21.63	1.19		
3.767	1.38	9.733	3.56	15.700	2.77	21.67	1.19		
3.800	1.58	9.767	3.96	15.733	2.77	21.70	1.19		
3.833	1.58	9.800	4.35	15.767	2.37	21.73	1.19		
3.867	1.58	9.833	4.35	15.800	1.98	21.77	1.19		
3.900	1.58	9.867	4.35	15.833	1.98	21.80	1.19		
3.933	1.58	9.900	4.35	15.867	1.98	21.83	1.19		
3.967	1.58	9.933	4.35	15.900	1.98	21.87	1.19		
4.000	1.58	9.967	4.35	15.933	1.98	21.90	1.19		
4.033	1.58	10.000	4.35	15.967	1.98	21.93	1.19		
4.067	1.58	10.033	4.75	16.000	1.98	21.97	1.19		
4.100	1.58	10.067	4.75	16.033	1.58	22.00	1.19		
4.133	1.58	10.100	4.75	16.067	1.58	22.03	1.19		
4.167	1.58	10.133	4.75	16.100	1.58	22.07	1.19		

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4.200	1.58	10.167	4.75	16.133	1.58	22.10	1.19		
4.233	1.58	10.200	4.75	16.167	1.58	22.13	1.19		
4.267	1.58	10.233	4.75	16.200	1.58	22.17	1.19		
4.300	1.58	10.267	5.34	16.233	1.58	22.20	1.19		
4.333	1.58	10.300	5.93	16.267	1.78	22.23	1.19		
4.367	1.58	10.333	5.93	16.300	1.98	22.27	1.19		
4.400	1.58	10.367	5.93	16.333	1.98	22.30	1.19		
4.433	1.58	10.400	5.93	16.367	1.98	22.33	1.19		
4.467	1.58	10.433	5.93	16.400	1.98	22.37	1.19		
4.500	1.58	10.467	5.93	16.433	1.98	22.40	1.19		
4.533	1.58	10.500	5.93	16.467	1.98	22.43	1.19		
4.567	1.58	10.533	6.33	16.500	1.98	22.47	1.19		
4.600	1.58	10.567	6.33	16.533	1.58	22.50	1.19		
4.633	1.58	10.600	6.33	16.567	1.58	22.53	1.19		
4.667	1.58	10.633	6.33	16.600	1.58	22.57	1.19		
4.700	1.58	10.667	6.33	16.633	1.58	22.60	1.19		
4.733	1.58	10.700	6.33	16.667	1.58	22.63	1.19		
4.767	1.58	10.733	6.33	16.700	1.58	22.67	1.19		
4.800	1.58	10.767	7.91	16.733	1.58	22.70	1.19		
4.833	1.58	10.800	9.49	16.767	1.78	22.73	1.19		
4.867	1.58	10.833	9.49	16.800	1.98	22.77	1.19		
4.900	1.58	10.867	9.49	16.833	1.98	22.80	1.19		
4.933	1.58	10.900	9.49	16.867	1.98	22.83	1.19		
4.967	1.58	10.933	9.49	16.900	1.98	22.87	1.19		
5.000	1.58	10.967	9.49	16.933	1.98	22.90	1.19		
5.033	1.58	11.000	9.49	16.967	1.98	22.93	1.19		
5.067	1.58	11.033	9.49	17.000	1.98	22.97	1.19		
5.100	1.58	11.067	9.49	17.033	1.58	23.00	1.19		
5.133	1.58	11.100	9.49	17.067	1.58	23.03	1.19		
5.167	1.58	11.133	9.49	17.100	1.58	23.07	1.19		
5.200	1.58	11.167	9.49	17.133	1.58	23.10	1.19		
5.233	1.58	11.200	9.49	17.167	1.58	23.13	1.19		
5.267	1.58	11.233	9.49	17.200	1.58	23.17	1.19		
5.300	1.58	11.267	19.41	17.233	1.58	23.20	1.19		
5.333	1.58	11.300	29.27	17.267	1.78	23.23	1.19		
5.367	1.58	11.333	29.27	17.300	1.98	23.27	1.19		
5.400	1.58	11.367	29.27	17.333	1.98	23.30	1.19		
5.433	1.58	11.400	29.27	17.367	1.98	23.33	1.19		
5.467	1.58	11.433	29.27	17.400	1.98	23.37	1.19		
5.500	1.58	11.467	29.27	17.433	1.98	23.40	1.19		
5.533	1.58	11.500	29.39	17.467	1.98	23.43	1.19		
5.567	1.58	11.533	121.05	17.500	1.98	23.47	1.19		
5.600	1.58	11.567	121.05	17.533	1.58	23.50	1.19		
5.633	1.58	11.600	121.05	17.567	1.58	23.53	1.19		
5.667	1.58	11.633	121.05	17.600	1.58	23.57	1.19		
5.700	1.58	11.667	121.05	17.633	1.58	23.60	1.19		
5.733	1.58	11.700	121.05	17.667	1.58	23.63	1.19		
5.767	1.58	11.733	121.05	17.700	1.58	23.67	1.19		
5.800	1.58	11.767	67.51	17.733	1.58	23.70	1.19		
5.833	1.58	11.800	14.24	17.767	1.78	23.73	1.19		
5.867	1.58	11.833	14.24	17.800	1.98	23.77	0.59		
5.900	1.58	11.867	14.24	17.833	1.98				
5.933	1.58	11.900	14.24	17.867	1.98				
5.967	1.58	11.933	14.24	17.900	1.98				

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.084 (i)

TIME TO PEAK (hrs)= 11.833

RUNOFF VOLUME (mm)= 19.282

TOTAL RAINFALL (mm)= 98.603

RUNOFF COEFFICIENT = 0.196

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(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0203) ID= 1 DT= 2.0 min	Area (ha)= 2.89 la (mm)= 8.98 U. H. Tp(hrs)= 0.18	Curve Number (CN)= 42.9 # of Linear Res. (N)= 3.00
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Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.140 (i)

TIME TO PEAK (hrs)= 11.833

RUNOFF VOLUME (mm)= 18.797

TOTAL RAINFALL (mm)= 98.603

RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0202):	1.63	0.084	11.83	19.28
+ ID2= 2 ( 0203):	2.89	0.140	11.83	18.80
=====				
ID = 3 ( 0057):	4.52	0.223	11.83	18.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ( 0204) ID= 1 DT= 2.0 min	Area (ha)= 0.64 la (mm)= 9.20 U. H. Tp(hrs)= 0.33	Curve Number (CN)= 48.1 # of Linear Res. (N)= 3.00
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Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.024 (i)

TIME TO PEAK (hrs)= 12.000

RUNOFF VOLUME (mm)= 21.976

TOTAL RAINFALL (mm)= 98.603

RUNOFF COEFFICIENT = 0.223

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0058) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0204):	0.64	0.024	12.00	21.98
+ ID2= 2 ( 0057):	4.52	0.223	11.83	18.97
=====				
ID = 3 ( 0058):	5.16	0.243	11.83	19.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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CALIB STANDHYD ( 0201) ID= 1 DT= 5.0 min	Area (ha)= 2.20 Total Imp(%)= 43.00	Dir. Conn. (%)= 21.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.95	1.25
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	1.00
Length (m)=	121.11	40.00
Mannings n =	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----					
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.19	6.083	1.98	12.083	14.24
0.167	1.19	6.167	1.98	12.167	14.24
0.250	1.19	6.250	1.98	12.250	14.24
0.333	0.79	6.333	1.58	12.333	7.52
0.417	0.79	6.417	1.58	12.417	7.52
0.500	0.79	6.500	1.58	12.500	7.52
0.583	1.19	6.583	1.98	12.583	7.12
0.667	1.19	6.667	1.98	12.667	7.12
0.750	1.19	6.750	1.98	12.750	7.12
0.833	1.19	6.833	1.98	12.833	5.54
0.917	1.19	6.917	1.98	12.917	5.54
1.000	1.19	7.000	1.98	13.000	5.54
1.083	1.19	7.083	2.37	13.083	5.14
1.167	1.19	7.167	2.37	13.167	5.14
1.250	1.19	7.250	2.37	13.250	5.14
1.333	0.79	7.333	1.98	13.333	4.35
1.417	0.79	7.417	1.98	13.417	4.35
1.500	0.79	7.500	1.98	13.500	4.35
1.583	1.19	7.583	2.37	13.583	3.96
1.667	1.19	7.667	2.37	13.667	3.96
1.750	1.19	7.750	2.37	13.750	3.96
1.833	1.19	7.833	2.37	13.833	3.16
1.917	1.19	7.917	2.37	13.917	3.16
2.000	1.19	8.000	2.37	14.000	3.16
2.083	1.58	8.083	2.77	14.083	2.77
2.167	1.58	8.167	2.77	14.167	2.77
2.250	1.58	8.250	2.77	14.250	2.77
2.333	1.19	8.333	2.77	14.333	3.16
2.417	1.19	8.417	2.77	14.417	3.16
2.500	1.19	8.500	2.77	14.500	3.16
2.583	1.19	8.583	2.77	14.583	2.77
2.667	1.19	8.667	2.77	14.667	2.77
2.750	1.19	8.750	2.77	14.750	2.77
2.833	1.19	8.833	3.16	14.833	3.16
2.917	1.19	8.917	3.16	14.917	3.16
3.000	1.19	9.000	3.16	15.000	3.16
3.083	1.58	9.083	3.16	15.083	2.77
3.167	1.58	9.167	3.16	15.167	2.77
3.250	1.58	9.250	3.16	15.250	2.77
3.333	1.19	9.333	3.56	15.333	3.16
3.417	1.19	9.417	3.56	15.417	3.16
3.500	1.19	9.500	3.56	15.500	3.16
3.583	1.19	9.583	3.56	15.583	2.77
3.667	1.19	9.667	3.56	15.667	2.77
3.750	1.19	9.750	3.56	15.750	2.77

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## SCS.txt

3.833	1.58	9.833	4.35	15.833	1.98	21.83	1.19
3.917	1.58	9.917	4.35	15.917	1.98	21.92	1.19
4.000	1.58	10.000	4.35	16.000	1.98	22.00	1.19
4.083	1.58	10.083	4.75	16.083	1.58	22.08	1.19
4.167	1.58	10.167	4.75	16.167	1.58	22.17	1.19
4.250	1.58	10.250	4.75	16.250	1.58	22.25	1.19
4.333	1.58	10.333	5.93	16.333	1.98	22.33	1.19
4.417	1.58	10.417	5.93	16.417	1.98	22.42	1.19
4.500	1.58	10.500	5.93	16.500	1.98	22.50	1.19
4.583	1.58	10.583	6.33	16.583	1.58	22.58	1.19
4.667	1.58	10.667	6.33	16.667	1.58	22.67	1.19
4.750	1.58	10.750	6.33	16.750	1.58	22.75	1.19
4.833	1.58	10.833	9.49	16.833	1.98	22.83	1.19
4.917	1.58	10.917	9.49	16.917	1.98	22.92	1.19
5.000	1.58	11.000	9.49	17.000	1.98	23.00	1.19
5.083	1.58	11.083	9.49	17.083	1.58	23.08	1.19
5.167	1.58	11.167	9.49	17.167	1.58	23.17	1.19
5.250	1.58	11.250	9.49	17.250	1.58	23.25	1.19
5.333	1.58	11.333	29.27	17.333	1.98	23.33	1.19
5.417	1.58	11.417	29.27	17.417	1.98	23.42	1.19
5.500	1.58	11.500	29.27	17.500	1.98	23.50	1.19
5.583	1.58	11.583	121.04	17.583	1.58	23.58	1.19
5.667	1.58	11.667	121.05	17.667	1.58	23.67	1.19
5.750	1.58	11.750	121.05	17.750	1.58	23.75	1.19
5.833	1.58	11.833	14.25	17.833	1.98		
5.917	1.58	11.917	14.24	17.917	1.98		
6.000	1.58	12.000	14.24	18.000	1.98		

Max. Eff. Inten. (mm/hr)= 121.05 159.96  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 2.65 (ii) 9.86 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.29 0.11

PEAK FLOW (cms)= 0.15 0.42 \*TOTALS\*  
TIME TO PEAK (hrs)= 11.75 11.75 0.571 (iii)  
RUNOFF VOLUME (mm)= 97.60 40.54 52.52  
TOTAL RAINFALL (mm)= 98.60 98.60 98.60  
RUNOFF COEFFICIENT = 0.99 0.41 0.53

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0301)  
IN= 2--> OUT= 1  
DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

INFLOW : ID= 2 ( 0201) 2.200 0.571 11.75 52.52

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## SCS.txt

OUTFLOW: ID= 1 ( 0301) 2.200 0.000 0.00 0.00

PEAK FLOW REDUCTION [Qout/Qi n](%)= 0.00  
TIME SHIFT OF PEAK FLOW (min)=\*\*\*\*\*  
MAXIMUM STORAGE USED (ha. m.)= 0.0614

ADD HYD ( 0059)  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0301):	2.20	0.000	0.00	0.00
+ ID2= 2 ( 0058):	5.16	0.243	11.83	19.34
ID = 3 ( 0059):	7.36	0.243	11.83	13.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0205)  
ID= 1 DT= 5.0 min

Area (ha)= 0.03  
Total Imp(%)= 56.00 Dir. Conn. (%)= 28.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.02 0.01  
Dep. Storage (mm)= 1.00 1.50  
Average Slope (%)= 1.00 1.00  
Length (m)= 14.14 20.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 121.05 190.49  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 0.73 (ii) 5.16 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.34 0.16

PEAK FLOW (cms)= 0.00 0.01 \*TOTALS\*  
TIME TO PEAK (hrs)= 11.75 11.75 0.009 (iii)  
RUNOFF VOLUME (mm)= 97.60 43.67 53.51  
TOTAL RAINFALL (mm)= 98.60 98.60 98.60  
RUNOFF COEFFICIENT = 0.99 0.44 0.54

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0060)  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0205):	0.03	0.009	11.75	53.51
+ ID2= 2 ( 0059):	7.36	0.243	11.83	13.56
ID = 3 ( 0060):	7.39	0.249	11.83	13.72

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## SCS.txt

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ( 0207) ID= 1 DT= 5.0 min	Area (ha)= 0.78 Ia (mm)= 4.87 U.H. Tp(hrs)= 0.19	Curve Number (CN)= 66.3 # of Linear Res. (N)= 3.00
--	--	---

Unit Hyd Qpeak (cms)= 0.157

PEAK FLOW (cms)= 0.080 (i)  
TIME TO PEAK (hrs)= 11.833  
RUNOFF VOLUME (mm)= 39.335  
TOTAL RAINFALL (mm)= 98.603  
RUNOFF COEFFICIENT = 0.399

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0061) 1 + 2 = 3	AREA (ha) 0.78 7.39	OPEAK (cms) 0.080 0.249	TPEAK (hrs) 11.83 11.83	R. V. (mm) 39.33 13.72
------------------------------	---------------------------	-------------------------------	-------------------------------	------------------------------

ID1= 1 ( 0207):  
+ ID2= 2 ( 0060):  
ID = 3 ( 0061):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0206) ID= 1 DT= 5.0 min	Area (ha)= 0.73 Total Imp(%)= 32.00	Dir. Conn. (%)= 13.00
--	--	-----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.23	0.50
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	2.00
Length (m)=	69.76	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	121.05	146.86
over (min)=	5.00	10.00
Storage Coeff. (min)=	1.91 (ii)	7.96 (ii)
Unit Hyd. Tpeak (min)=	5.00	10.00
Unit Hyd. peak (cms)=	0.32	0.13
PEAK FLOW (cms)=	0.03	0.16
TIME TO PEAK (hrs)=	11.75	11.75
RUNOFF VOLUME (mm)=	97.60	38.99
TOTAL RAINFALL (mm)=	98.60	98.60
RUNOFF COEFFICIENT =	0.99	0.40

\*TOTALS\*  
0.195 (iii)  
11.75  
46.61  
98.60  
0.47

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
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## SCS.txt

Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0401) 1 + 2 = 3	AREA (ha) 0.73 8.17	OPEAK (cms) 0.195 0.329	TPEAK (hrs) 11.75 11.83	R. V. (mm) 46.61 16.17
------------------------------	---------------------------	-------------------------------	-------------------------------	------------------------------

ID1= 1 ( 0206):  
+ ID2= 2 ( 0061):  
ID = 3 ( 0401):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
VV I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y MM MM 0 0
0 0 T T H H Y M M 0 0
000 T T H H Y M M 000

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## \*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\VO2\voinput.dat

Output filename:

C:\Users\aschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\08796f1a-d53a-4d1f-8f1d-5a50c28eda5a\scen

Summary filename:

C:\Users\aschoof\AppData\Local\Civica\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\08796f1a-d53a-4d1f-8f1d-5a50c28eda5a\scen

DATE: 10/09/2020

TIME: 05:31:52

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
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SCS.txt  
\*\* SIMULATION : Run 05 \*\*  
\*\*\*\*\*

MASS STORM

Ptotal=109.80 mm

Filename: C:\Users\aschoof\AppData\Local\Temp\d21eaacb-4cf1-4504-bf53-d3c44604e13d\00a83143  
Comments: SCS Type II 24 HR MASS CURVE

Duration of storm = 23.75 hrs  
Mass curve time step = 15.00 min

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.32	6.25	2.20	12.25	15.81	18.25	1.76
0.50	0.88	6.50	1.76	12.50	8.34	18.50	2.20
0.75	1.32	6.75	2.20	12.75	7.91	18.75	1.76
1.00	1.32	7.00	2.20	13.00	6.15	19.00	2.20
1.25	1.32	7.25	2.64	13.25	5.71	19.25	1.76
1.50	0.88	7.50	2.20	13.50	4.83	19.50	2.20
1.75	1.32	7.75	2.64	13.75	4.39	19.75	1.76
2.00	1.32	8.00	2.64	14.00	3.51	20.00	1.32
2.25	1.76	8.25	3.07	14.25	3.07	20.25	1.32
2.50	1.32	8.50	3.07	14.50	3.51	20.50	1.32
2.75	1.32	8.75	3.07	14.75	3.07	20.75	1.32
3.00	1.32	9.00	3.51	15.00	3.51	21.00	1.32
3.25	1.76	9.25	3.51	15.25	3.07	21.25	1.32
3.50	1.32	9.50	3.95	15.50	3.51	21.50	1.32
3.75	1.32	9.75	3.95	15.75	3.07	21.75	1.32
4.00	1.76	10.00	4.83	16.00	2.20	22.00	1.32
4.25	1.76	10.25	5.27	16.25	1.76	22.25	1.32
4.50	1.76	10.50	6.59	16.50	2.20	22.50	1.32
4.75	1.76	10.75	7.03	16.75	1.76	22.75	1.32
5.00	1.76	11.00	10.54	17.00	2.20	23.00	1.32
5.25	1.76	11.25	10.54	17.25	1.76	23.25	1.32
5.50	1.76	11.50	32.50	17.50	2.20	23.50	1.32
5.75	1.76	11.75	134.40	17.75	1.76	23.75	1.32
6.00	1.76	12.00	15.81	18.00	2.20		

CALIB  
NASHYD ( 0202)  
ID= 1 DT= 2.0 min

Area (ha)= 1.63 Curve Number (CN)= 44.1  
Ia (mm)= 9.62 # of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.17

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.033	1.32	6.000	1.76	11.967	15.81	17.93	2.20
0.067	1.32	6.033	2.20	12.000	15.81	17.97	2.20
0.100	1.32	6.067	2.20	12.033	15.81	18.00	2.20
0.133	1.32	6.100	2.20	12.067	15.81	18.03	1.76
0.167	1.32	6.133	2.20	12.100	15.81	18.07	1.76
0.200	1.32	6.167	2.20	12.133	15.81	18.10	1.76
0.233	1.32	6.200	2.20	12.167	15.81	18.13	1.76
0.267	1.10	6.233	2.20	12.200	15.81	18.17	1.76
0.300	0.88	6.267	1.98	12.233	15.81	18.20	1.76
0.333	0.88	6.300	1.76	12.267	12.07	18.23	1.76
0.367	0.88	6.333	1.76	12.300	8.34	18.27	1.98

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0.400	0.88	6.367	1.76	12.333	8.34	18.30	2.20
0.433	0.88	6.400	1.76	12.367	8.34	18.33	2.20
0.467	0.88	6.433	1.76	12.400	8.34	18.37	2.20
0.500	0.88	6.467	1.76	12.433	8.34	18.40	2.20
0.533	1.32	6.500	1.76	12.467	8.34	18.43	2.20
0.567	1.32	6.533	2.20	12.500	8.34	18.47	2.20
0.600	1.32	6.567	2.20	12.533	7.91	18.50	2.20
0.633	1.32	6.600	2.20	12.567	7.91	18.53	1.76
0.667	1.32	6.633	2.20	12.600	7.91	18.57	1.76
0.700	1.32	6.667	2.20	12.633	7.91	18.60	1.76
0.733	1.32	6.700	2.20	12.667	7.91	18.63	1.76
0.767	1.32	6.733	2.20	12.700	7.91	18.67	1.76
0.800	1.32	6.767	2.20	12.733	7.91	18.70	1.76
0.833	1.32	6.800	2.20	12.767	7.02	18.73	1.76
0.867	1.32	6.833	2.20	12.800	6.15	18.77	1.98
0.900	1.32	6.867	2.20	12.833	6.15	18.80	2.20
0.933	1.32	6.900	2.20	12.867	6.15	18.83	2.20
0.967	1.32	6.933	2.20	12.900	6.15	18.87	2.20
1.000	1.32	6.967	2.20	12.933	6.15	18.90	2.20
1.033	1.32	7.000	2.20	12.967	6.15	18.93	2.20
1.067	1.32	7.033	2.64	13.000	6.15	18.97	2.20
1.100	1.32	7.067	2.64	13.033	5.71	19.00	2.20
1.133	1.32	7.100	2.64	13.067	5.71	19.03	1.76
1.167	1.32	7.133	2.64	13.100	5.71	19.07	1.76
1.200	1.32	7.167	2.64	13.133	5.71	19.10	1.76
1.233	1.32	7.200	2.64	13.167	5.71	19.13	1.76
1.267	1.10	7.233	2.64	13.200	5.71	19.17	1.76
1.300	0.88	7.267	2.42	13.233	5.71	19.20	1.76
1.333	0.88	7.300	2.20	13.267	5.27	19.23	1.76
1.367	0.88	7.333	2.20	13.300	4.83	19.27	1.98
1.400	0.88	7.367	2.20	13.333	4.83	19.30	2.20
1.433	0.88	7.400	2.20	13.367	4.83	19.33	2.20
1.467	0.88	7.433	2.20	13.400	4.83	19.37	2.20
1.500	0.88	7.467	2.20	13.433	4.83	19.40	2.20
1.533	1.32	7.500	2.20	13.467	4.83	19.43	2.20
1.567	1.32	7.533	2.64	13.500	4.83	19.47	2.20
1.600	1.32	7.567	2.64	13.533	4.39	19.50	2.20
1.633	1.32	7.600	2.64	13.567	4.39	19.53	1.76
1.667	1.32	7.633	2.64	13.600	4.39	19.57	1.76
1.700	1.32	7.667	2.64	13.633	4.39	19.60	1.76
1.733	1.32	7.700	2.64	13.667	4.39	19.63	1.76
1.767	1.32	7.733	2.64	13.700	4.39	19.67	1.76
1.800	1.32	7.767	2.64	13.733	4.39	19.70	1.76
1.833	1.32	7.800	2.64	13.767	3.95	19.73	1.76
1.867	1.32	7.833	2.64	13.800	3.51	19.77	1.54
1.900	1.32	7.867	2.64	13.833	3.51	19.80	1.32
1.933	1.32	7.900	2.64	13.867	3.51	19.83	1.32
1.967	1.32	7.933	2.64	13.900	3.51	19.87	1.32
2.000	1.32	7.967	2.64	13.933	3.51	19.90	1.32
2.033	1.76	8.000	2.64	13.967	3.51	19.93	1.32
2.067	1.76	8.033	3.07	14.000	3.51	19.97	1.32
2.100	1.76	8.067	3.07	14.033	3.07	20.00	1.32
2.133	1.76	8.100	3.07	14.067	3.07	20.03	1.32
2.167	1.76	8.133	3.07	14.100	3.07	20.07	1.32
2.200	1.76	8.167	3.07	14.133	3.07	20.10	1.32
2.233	1.76	8.200	3.07	14.167	3.07	20.13	1.32
2.267	1.54	8.233	3.07	14.200	3.07	20.17	1.32
2.300	1.32	8.267	3.07	14.233	3.07	20.20	1.32
2.333	1.32	8.300	3.07	14.267	3.30	20.23	1.32
2.367	1.32	8.333	3.07	14.300	3.51	20.27	1.32
2.400	1.32	8.367	3.07	14.333	3.51	20.30	1.32
2.433	1.32	8.400	3.07	14.367	3.51	20.33	1.32
2.467	1.32	8.433	3.07	14.400	3.51	20.37	1.32

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SCS.txt									
2.500	1.32	8.467	3.07	14.433	3.51	20.40	1.32		
2.533	1.32	8.500	3.07	14.467	3.51	20.43	1.32		
2.567	1.32	8.533	3.07	14.500	3.51	20.47	1.32		
2.600	1.32	8.567	3.07	14.533	3.07	20.50	1.32		
2.633	1.32	8.600	3.07	14.567	3.07	20.53	1.32		
2.667	1.32	8.633	3.07	14.600	3.07	20.57	1.32		
2.700	1.32	8.667	3.07	14.633	3.07	20.60	1.32		
2.733	1.32	8.700	3.07	14.667	3.07	20.63	1.32		
2.767	1.32	8.733	3.07	14.700	3.07	20.67	1.32		
2.800	1.32	8.767	3.29	14.733	3.07	20.70	1.32		
2.833	1.32	8.800	3.51	14.767	3.30	20.73	1.32		
2.867	1.32	8.833	3.51	14.800	3.51	20.77	1.32		
2.900	1.32	8.867	3.51	14.833	3.51	20.80	1.32		
2.933	1.32	8.900	3.51	14.867	3.51	20.83	1.32		
2.967	1.32	8.933	3.51	14.900	3.51	20.87	1.32		
3.000	1.32	8.967	3.51	14.933	3.51	20.90	1.32		
3.033	1.76	9.000	3.51	14.967	3.51	20.93	1.32		
3.067	1.76	9.033	3.51	15.000	3.51	20.97	1.32		
3.100	1.76	9.067	3.51	15.033	3.07	21.00	1.32		
3.133	1.76	9.100	3.51	15.067	3.07	21.03	1.32		
3.167	1.76	9.133	3.51	15.100	3.07	21.07	1.32		
3.200	1.76	9.167	3.51	15.133	3.07	21.10	1.32		
3.233	1.76	9.200	3.51	15.167	3.07	21.13	1.32		
3.267	1.54	9.233	3.51	15.200	3.07	21.17	1.32		
3.300	1.32	9.267	3.73	15.233	3.07	21.20	1.32		
3.333	1.32	9.300	3.95	15.267	3.30	21.23	1.32		
3.367	1.32	9.333	3.95	15.300	3.51	21.27	1.32		
3.400	1.32	9.367	3.95	15.333	3.51	21.30	1.32		
3.433	1.32	9.400	3.95	15.367	3.51	21.33	1.32		
3.467	1.32	9.433	3.95	15.400	3.51	21.37	1.32		
3.500	1.32	9.467	3.95	15.433	3.51	21.40	1.32		
3.533	1.32	9.500	3.95	15.467	3.51	21.43	1.32		
3.567	1.32	9.533	3.95	15.500	3.51	21.47	1.32		
3.600	1.32	9.567	3.95	15.533	3.07	21.50	1.32		
3.633	1.32	9.600	3.95	15.567	3.07	21.53	1.32		
3.667	1.32	9.633	3.95	15.600	3.07	21.57	1.32		
3.700	1.32	9.667	3.95	15.633	3.07	21.60	1.32		
3.733	1.32	9.700	3.95	15.667	3.07	21.63	1.32		
3.767	1.54	9.733	3.95	15.700	3.07	21.67	1.32		
3.800	1.76	9.767	4.39	15.733	3.07	21.70	1.32		
3.833	1.76	9.800	4.83	15.767	2.63	21.73	1.32		
3.867	1.76	9.833	4.83	15.800	2.20	21.77	1.32		
3.900	1.76	9.867	4.83	15.833	2.20	21.80	1.32		
3.933	1.76	9.900	4.83	15.867	2.20	21.83	1.32		
3.967	1.76	9.933	4.83	15.900	2.20	21.87	1.32		
4.000	1.76	9.967	4.83	15.933	2.20	21.90	1.32		
4.033	1.76	10.000	4.83	15.967	2.20	21.93	1.32		
4.067	1.76	10.033	5.27	16.000	2.19	21.97	1.32		
4.100	1.76	10.067	5.27	16.033	1.76	22.00	1.32		
4.133	1.76	10.100	5.27	16.067	1.76	22.03	1.32		
4.167	1.76	10.133	5.27	16.100	1.76	22.07	1.32		
4.200	1.76	10.167	5.27	16.133	1.76	22.10	1.32		
4.233	1.76	10.200	5.27	16.167	1.76	22.13	1.32		
4.267	1.76	10.233	5.27	16.200	1.76	22.17	1.32		
4.300	1.76	10.267	5.93	16.233	1.76	22.20	1.32		
4.333	1.76	10.300	6.59	16.267	1.98	22.23	1.32		
4.367	1.76	10.333	6.59	16.300	2.20	22.27	1.32		
4.400	1.76	10.367	6.59	16.333	2.20	22.30	1.32		
4.433	1.76	10.400	6.59	16.367	2.20	22.33	1.32		
4.467	1.76	10.433	6.59	16.400	2.20	22.37	1.32		
4.500	1.76	10.467	6.59	16.433	2.20	22.40	1.32		
4.533	1.76	10.500	6.59	16.467	2.20	22.43	1.32		
4.567	1.76	10.533	7.03	16.500	2.19	22.47	1.32		

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SCS.txt									
4.600	1.76	10.567	7.03	16.533	1.76	22.50	1.32		
4.633	1.76	10.600	7.03	16.567	1.76	22.53	1.32		
4.667	1.76	10.633	7.03	16.600	1.76	22.57	1.32		
4.700	1.76	10.667	7.03	16.633	1.76	22.60	1.32		
4.733	1.76	10.700	7.03	16.667	1.76	22.63	1.32		
4.767	1.76	10.733	7.03	16.700	1.76	22.67	1.32		
4.800	1.76	10.767	8.79	16.733	1.76	22.70	1.32		
4.833	1.76	10.800	10.54	16.767	1.98	22.73	1.32		
4.867	1.76	10.833	10.54	16.800	2.20	22.77	1.32		
4.900	1.76	10.867	10.54	16.833	2.20	22.80	1.32		
4.933	1.76	10.900	10.54	16.867	2.20	22.83	1.32		
4.967	1.76	10.933	10.54	16.900	2.20	22.87	1.32		
5.000	1.76	10.967	10.54	16.933	2.20	22.90	1.32		
5.033	1.76	11.000	10.54	16.967	2.20	22.93	1.32		
5.067	1.76	11.033	10.54	17.000	2.19	22.97	1.32		
5.100	1.76	11.067	10.54	17.033	1.76	23.00	1.32		
5.133	1.76	11.100	10.54	17.067	1.76	23.03	1.32		
5.167	1.76	11.133	10.54	17.100	1.76	23.07	1.32		
5.200	1.76	11.167	10.54	17.133	1.76	23.10	1.32		
5.233	1.76	11.200	10.54	17.167	1.76	23.13	1.32		
5.267	1.76	11.233	10.54	17.200	1.76	23.17	1.32		
5.300	1.76	11.267	21.55	17.233	1.76	23.20	1.32		
5.333	1.76	11.300	32.50	17.267	1.98	23.23	1.32		
5.367	1.76	11.333	32.50	17.300	2.20	23.27	1.32		
5.400	1.76	11.367	32.50	17.333	2.20	23.30	1.32		
5.433	1.76	11.400	32.50	17.367	2.20	23.33	1.32		
5.467	1.76	11.433	32.50	17.400	2.20	23.37	1.32		
5.500	1.76	11.467	32.50	17.433	2.20	23.40	1.32		
5.533	1.76	11.500	32.62	17.467	2.20	23.43	1.32		
5.567	1.76	11.533	134.40	17.500	2.19	23.47	1.32		
5.600	1.76	11.567	134.40	17.533	1.76	23.50	1.32		
5.633	1.76	11.600	134.40	17.567	1.76	23.53	1.32		
5.667	1.76	11.633	134.40	17.600	1.76	23.57	1.32		
5.700	1.76	11.667	134.40	17.633	1.76	23.60	1.32		
5.733	1.76	11.700	134.40	17.667	1.76	23.63	1.32		
5.767	1.76	11.733	134.40	17.700	1.76	23.67	1.32		
5.800	1.76	11.767	74.95	17.733	1.76	23.70	1.32		
5.833	1.76	11.800	15.81	17.767	1.98	23.73	1.32		
5.867	1.76	11.833	15.81	17.800	2.20	23.77	0.66		
5.900	1.76	11.867	15.81	17.833	2.20				
5.933	1.76	11.900	15.81	17.867	2.20				
5.967	1.76	11.933	15.81	17.900	2.20				

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.103 (i)

TIME TO PEAK (hrs)= 11.833

RUNOFF VOLUME (mm)= 23.653

TOTAL RAINFALL (mm)= 109.470

RUNOFF COEFFICIENT = 0.216

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
NASHYD ( 0203)			
ID= 1 DT= 2.0 min			
Area	(ha)=	2.89	Curve Number (CN)= 42.9
Ia	(mm)=	8.98	# of Linear Res. (N)= 3.00
U. H. Tp	(hrs)=	0.18	

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.172 (i)

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## SCS.txt

TIME TO PEAK (hrs)= 11.833  
 RUNOFF VOLUME (mm)= 23.045  
 TOTAL RAINFALL (mm)= 109.470  
 RUNOFF COEFFICIENT = 0.211

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057)				
1	2	3		
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0202):	1.63	0.103	11.83	23.65
+ ID2= 2 ( 0203):	2.89	0.172	11.83	23.05
=====				
ID = 3 ( 0057):	4.52	0.275	11.83	23.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ( 0204)				
ID= 1 DT= 2.0 min	Area (ha)=	0.64	Curve Number (CN)=	48.1
	la (mm)=	9.20	# of Linear Res. (N)=	3.00
	U. H. Tp(hrs)=	0.33		

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.030 (i)  
 TIME TO PEAK (hrs)= 11.967  
 RUNOFF VOLUME (mm)= 26.841  
 TOTAL RAINFALL (mm)= 109.470  
 RUNOFF COEFFICIENT = 0.245

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0058)				
1	2	3		
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0204):	0.64	0.030	11.97	26.84
+ ID2= 2 ( 0057):	4.52	0.275	11.83	23.26
=====				
ID = 3 ( 0058):	5.16	0.300	11.83	23.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0201)				
ID= 1 DT= 5.0 min	Area (ha)=	2.20		
	Total Imp(%)=	43.00	Dir. Conn. (%)=	21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.95	1.25
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	1.00
Length (m)=	121.11	40.00
Mannings n =	0.013	0.250

## SCS.txt

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----									
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.32	6.083	2.20	12.083	15.81	18.08	1.76		
0.167	1.32	6.167	2.20	12.167	15.81	18.17	1.76		
0.250	1.32	6.250	2.20	12.250	15.81	18.25	1.76		
0.333	0.88	6.333	1.76	12.333	8.35	18.33	2.20		
0.417	0.88	6.417	1.76	12.417	8.34	18.42	2.20		
0.500	0.88	6.500	1.76	12.500	8.34	18.50	2.20		
0.583	1.32	6.583	2.20	12.583	7.91	18.58	1.76		
0.667	1.32	6.667	2.20	12.667	7.91	18.67	1.76		
0.750	1.32	6.750	2.20	12.750	7.91	18.75	1.76		
0.833	1.32	6.833	2.20	12.833	6.15	18.83	2.20		
0.917	1.32	6.917	2.20	12.917	6.15	18.92	2.20		
1.000	1.32	7.000	2.20	13.000	6.15	19.00	2.20		
1.083	1.32	7.083	2.64	13.083	5.71	19.08	1.76		
1.167	1.32	7.167	2.64	13.167	5.71	19.17	1.76		
1.250	1.32	7.250	2.64	13.250	5.71	19.25	1.76		
1.333	0.88	7.333	2.20	13.333	4.83	19.33	2.20		
1.417	0.88	7.417	2.20	13.417	4.83	19.42	2.20		
1.500	0.88	7.500	2.20	13.500	4.83	19.50	2.20		
1.583	1.32	7.583	2.64	13.583	4.39	19.58	1.76		
1.667	1.32	7.667	2.64	13.667	4.39	19.67	1.76		
1.750	1.32	7.750	2.64	13.750	4.39	19.75	1.76		
1.833	1.32	7.833	2.64	13.833	3.51	19.83	1.32		
1.917	1.32	7.917	2.64	13.917	3.51	19.92	1.32		
2.000	1.32	8.000	2.64	14.000	3.51	20.00	1.32		
2.083	1.76	8.083	3.07	14.083	3.07	20.08	1.32		
2.167	1.76	8.167	3.07	14.167	3.07	20.17	1.32		
2.250	1.76	8.250	3.07	14.250	3.07	20.25	1.32		
2.333	1.32	8.333	3.07	14.333	3.51	20.33	1.32		
2.417	1.32	8.417	3.07	14.417	3.51	20.42	1.32		
2.500	1.32	8.500	3.07	14.500	3.51	20.50	1.32		
2.583	1.32	8.583	3.07	14.583	3.07	20.58	1.32		
2.667	1.32	8.667	3.07	14.667	3.07	20.67	1.32		
2.750	1.32	8.750	3.07	14.750	3.07	20.75	1.32		
2.833	1.32	8.833	3.51	14.833	3.51	20.83	1.32		
2.917	1.32	8.917	3.51	14.917	3.51	20.92	1.32		
3.000	1.32	9.000	3.51	15.000	3.51	21.00	1.32		
3.083	1.76	9.083	3.51	15.083	3.07	21.08	1.32		
3.167	1.76	9.167	3.51	15.167	3.07	21.17	1.32		
3.250	1.76	9.250	3.51	15.250	3.07	21.25	1.32		
3.333	1.32	9.333	3.95	15.333	3.51	21.33	1.32		
3.417	1.32	9.417	3.95	15.417	3.51	21.42	1.32		
3.500	1.32	9.500	3.95	15.500	3.51	21.50	1.32		
3.583	1.32	9.583	3.95	15.583	3.07	21.58	1.32		
3.667	1.32	9.667	3.95	15.667	3.07	21.67	1.32		
3.750	1.32	9.750	3.95	15.750	3.07	21.75	1.32		
3.833	1.76	9.833	4.83	15.833	2.20	21.83	1.32		
3.917	1.76	9.917	4.83	15.917	2.20	21.92	1.32		
4.000	1.76	10.000	4.83	16.000	2.20	22.00	1.32		
4.083	1.76	10.083	5.27	16.083	1.76	22.08	1.32		
4.167	1.76	10.167	5.27	16.167	1.76	22.17	1.32		
4.250	1.76	10.250	5.27	16.250	1.76	22.25	1.32		
4.333	1.76	10.333	6.59	16.333	2.20	22.33	1.32		
4.417	1.76	10.417	6.59	16.417	2.20	22.42	1.32		
4.500	1.76	10.500	6.59	16.500	2.20	22.50	1.32		
4.583	1.76	10.583	7.03	16.583	1.76	22.58	1.32		
4.667	1.76	10.667	7.03	16.667	1.76	22.67	1.32		
4.750	1.76	10.750	7.03	16.750	1.76	22.75	1.32		

SCS.txt							
4.833	1.76	10.833	10.54	16.833	2.20	22.83	1.32
4.917	1.76	10.917	10.54	16.917	2.20	22.92	1.32
5.000	1.76	11.000	10.54	17.000	2.20	23.00	1.32
5.083	1.76	11.083	10.54	17.083	1.76	23.08	1.32
5.167	1.76	11.167	10.54	17.167	1.76	23.17	1.32
5.250	1.76	11.250	10.54	17.250	1.76	23.25	1.32
5.333	1.76	11.333	32.50	17.333	2.20	23.33	1.32
5.417	1.76	11.417	32.50	17.417	2.20	23.42	1.32
5.500	1.76	11.500	32.50	17.500	2.20	23.50	1.32
5.583	1.76	11.583	134.39	17.583	1.76	23.58	1.32
5.667	1.76	11.667	134.40	17.667	1.76	23.67	1.32
5.750	1.76	11.750	134.40	17.750	1.76	23.75	1.32
5.833	1.76	11.833	15.82	17.833	2.20		
5.917	1.76	11.917	15.81	17.917	2.20		
6.000	1.76	12.000	15.81	18.000	2.20		

Max. Eff. Inten. (mm/hr)= 134.40 178.62  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 2.55 (ii) 9.44 (ii)  
Unit Hyd. Tpeak (min)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.29 0.12

PEAK FLOW (cms)= 0.17 0.47  
TIME TO PEAK (hrs)= 11.75 11.75  
RUNOFF VOLUME (mm)= 108.47 47.16  
TOTAL RAINFALL (mm)= 109.47 109.47  
RUNOFF COEFFICIENT = 0.99 0.43

\*TOTALS\*  
0.646 (iii)  
11.75  
60.04  
109.47  
0.55

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0301)  
IN= 2---> OUT= 1  
DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 ( 0201)	2.200	0.646	11.75	60.04
OUTFLOW: ID= 1 ( 0301)	2.200	0.000	0.00	0.00

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.00  
TIME SHIFT OF PEAK FLOW (min) = \*\*\*\*\*  
MAXIMUM STORAGE USED (ha. m.) = 0.0711

ADD HYD ( 0059)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)

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SCS.txt  
ID1= 1 ( 0301): 2.20 0.000 0.00 0.00  
+ ID2= 2 ( 0058): 5.16 0.300 11.83 23.71  
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
STANDHYD ( 0205)  
ID= 1 DT= 5.0 min

Area (ha)= 0.03  
Total Imp(%)= 56.00 Dir. Conn. (%) = 28.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.02 0.01  
Dep. Storage (mm)= 1.00 1.50  
Average Slope (%)= 1.00 1.00  
Length (m)= 14.14 20.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 134.40 212.38  
over (min)= 5.00 5.00  
Storage Coeff. (min)= 0.70 (ii) 4.94 (ii)  
Unit Hyd. Tpeak (min)= 5.00 5.00  
Unit Hyd. peak (cms)= 0.34 0.22

PEAK FLOW (cms)= 0.00 0.01  
TIME TO PEAK (hrs)= 11.75 11.75  
RUNOFF VOLUME (mm)= 108.47 50.53  
TOTAL RAINFALL (mm)= 109.47 109.47  
RUNOFF COEFFICIENT = 0.99 0.46

\*TOTALS\*  
0.011 (iii)  
11.75  
61.22  
109.47  
0.56

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0060)  
1 + 2 = 3

AREA OPEAK TPEAK R. V.  
(ha) (cms) (hrs) (mm)  
ID1= 1 ( 0205): 0.03 0.011 11.75 61.22  
+ ID2= 2 ( 0059): 7.36 0.300 11.83 16.62  
=====

ID = 3 ( 0060): 7.39 0.303 11.83 16.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
NASHYD ( 0207)  
ID= 1 DT= 5.0 min

Area (ha)= 0.78 Curve Number (CN)= 66.3  
la (mm)= 4.87 # of Linear Res. (N)= 3.00  
U. H. Tp(hrs)= 0.19

Unit Hyd Qpeak (cms)= 0.157

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SCS.txt  
PEAK FLOW (cms)= 0.095 (i)  
TIME TO PEAK (hrs)= 11.833  
RUNOFF VOLUME (mm)= 46.707  
TOTAL RAINFALL (mm)= 109.471  
RUNOFF COEFFICIENT = 0.427

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0061)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 ( 0207):	0.78	0.095	11.83	46.71
+ ID2= 2 ( 0060):	7.39	0.303	11.83	16.80
ID = 3 ( 0061):	8.17	0.398	11.83	19.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0206)	Area (ha)	IMPERVIOUS	PERVIOUS (i)
ID= 1 DT= 5.0 min	Total Imp(%)= 32.00		
Dir. Conn. (%)= 13.00			
Surface Area (ha)=	0.23		0.50
Dep. Storage (mm)=	1.00		1.50
Average Slope (%)=	1.00		2.00
Length (m)=	69.76		40.00
Mannings n =	0.013		0.250
Max. Eff. Inten. (mm/hr)=	134.40		164.18
over (min)=	5.00		10.00
Storage Coeff. (min)=	1.83 (ii)		7.62 (ii)
Unit Hyd. Tpeak (min)=	5.00		10.00
Unit Hyd. peak (cms)=	0.32		0.13
PEAK FLOW (cms)=	0.04		0.19
TIME TO PEAK (hrs)=	11.75		11.75
RUNOFF VOLUME (mm)=	108.47		53.68
TOTAL RAINFALL (mm)=	109.47		109.47
RUNOFF COEFFICIENT =	0.99		0.49

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0401)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				

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SCS.txt  
ID1= 1 ( 0206): 0.73 0.222 11.75 53.68  
+ ID2= 2 ( 0061): 8.17 0.398 11.83 19.66  
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L  
V V I SS U U A A L  
V V I SS U U A A L  
V V I SSSSS UUUUU A A LLLLL  
000 TTTT TTTT H H Y Y M M 000 TM  
O O T T H H Y Y M M O O  
O O T T H H Y Y M M O O  
000 T T H H Y Y M M 000

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.0\V02\voinput.dat

Output filename:  
C:\Users\vaschoof\AppData\Local\Civi.ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\86865  
4d5-43f7-4ece-987b-dd28b1573297\scen  
Summary filename:  
C:\Users\vaschoof\AppData\Local\Civi.ca\XH5\8194ef53-adad-4f15-90f7-c4eafb4675c3\86865  
4d5-43f7-4ece-987b-dd28b1573297\scen

DATE: 10/09/2020 TIME: 05:31:52

USER:

COMMENTS: \_\_\_\_\_

\*\*\*\*\*  
\*\* SIMULATION : Run 06 \*\*  
\*\*\*\*\*

MASS STORM	Filename: C:\Users\vaschoof\AppData\Local\Temp\
Ptotal=120.80 mm	d21eaacb-4cf1-4504-bf53-d3c44604e13d\4477bc98
	Comments: SCS Type II 24 HR MASS CURVE
	Duration of storm = 23.75 hrs
	Mass curve time step = 15.00 min

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SCS.txt							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.45	6.25	2.42	12.25	17.40	18.25	1.93
0.50	0.97	6.50	1.93	12.50	9.18	18.50	2.42
0.75	1.45	6.75	2.42	12.75	8.70	18.75	1.93
1.00	1.45	7.00	2.42	13.00	6.76	19.00	2.42
1.25	1.45	7.25	2.90	13.25	6.28	19.25	1.93
1.50	0.97	7.50	2.42	13.50	5.32	19.50	2.42
1.75	1.45	7.75	2.90	13.75	4.83	19.75	1.93
2.00	1.45	8.00	2.90	14.00	3.87	20.00	1.45
2.25	1.93	8.25	3.38	14.25	3.38	20.25	1.45
2.50	1.45	8.50	3.38	14.50	3.87	20.50	1.45
2.75	1.45	8.75	3.38	14.75	3.38	20.75	1.45
3.00	1.45	9.00	3.87	15.00	3.87	21.00	1.45
3.25	1.93	9.25	3.87	15.25	3.38	21.25	1.45
3.50	1.45	9.50	4.35	15.50	3.87	21.50	1.45
3.75	1.45	9.75	4.35	15.75	3.38	21.75	1.45
4.00	1.93	10.00	5.32	16.00	2.42	22.00	1.45
4.25	1.93	10.25	5.80	16.25	1.93	22.25	1.45
4.50	1.93	10.50	7.25	16.50	2.42	22.50	1.45
4.75	1.93	10.75	7.73	16.75	1.93	22.75	1.45
5.00	1.93	11.00	11.60	17.00	2.42	23.00	1.45
5.25	1.93	11.25	11.60	17.25	1.93	23.25	1.45
5.50	1.93	11.50	35.76	17.50	2.42	23.50	1.45
5.75	1.93	11.75	147.86	17.75	1.93	23.75	1.45
6.00	1.93	12.00	17.40	18.00	2.42		

CALIB  
 NASHYD ( 0202)  
 ID= 1 DT= 2.0 min

Area (ha)= 1.63  
 Ia (mm)= 9.62  
 U.H. Tp(hrs)= 0.17

Curve Number (CN)= 44.1  
 # of Linear Res. (N)= 3.00

NOTE: RAINFALL WAS TRANSFORMED TO 2.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.033	1.45	6.000	1.93	11.967	17.40	17.93	2.42
0.067	1.45	6.033	2.42	12.000	17.40	17.97	2.42
0.100	1.45	6.067	2.42	12.033	17.40	18.00	2.41
0.133	1.45	6.100	2.42	12.067	17.40	18.03	1.93
0.167	1.45	6.133	2.42	12.100	17.40	18.07	1.93
0.200	1.45	6.167	2.42	12.133	17.40	18.10	1.93
0.233	1.45	6.200	2.42	12.167	17.40	18.13	1.93
0.267	1.21	6.233	2.42	12.200	17.40	18.17	1.93
0.300	0.97	6.267	2.17	12.233	17.40	18.20	1.93
0.333	0.97	6.300	1.93	12.267	13.28	18.23	1.93
0.367	0.97	6.333	1.93	12.300	9.18	18.27	2.18
0.400	0.97	6.367	1.93	12.333	9.18	18.30	2.42
0.433	0.97	6.400	1.93	12.367	9.18	18.33	2.42
0.467	0.97	6.433	1.93	12.400	9.18	18.37	2.42
0.500	0.97	6.467	1.93	12.433	9.18	18.40	2.42
0.533	1.45	6.500	1.93	12.467	9.18	18.43	2.42
0.567	1.45	6.533	2.42	12.500	9.18	18.47	2.42
0.600	1.45	6.567	2.42	12.533	8.70	18.50	2.42
0.633	1.45	6.600	2.42	12.567	8.70	18.53	1.93
0.667	1.45	6.633	2.42	12.600	8.70	18.57	1.93
0.700	1.45	6.667	2.42	12.633	8.70	18.60	1.93
0.733	1.45	6.700	2.42	12.667	8.70	18.63	1.93
0.767	1.45	6.733	2.42	12.700	8.70	18.67	1.93

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SCS.txt							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.800	1.45	6.767	2.42	12.733	8.70	18.70	1.93
0.833	1.45	6.800	2.42	12.767	7.73	18.73	1.93
0.867	1.45	6.833	2.42	12.800	6.76	18.77	2.18
0.900	1.45	6.867	2.42	12.833	6.76	18.80	2.42
0.933	1.45	6.900	2.42	12.867	6.76	18.83	2.42
0.967	1.45	6.933	2.42	12.900	6.76	18.87	2.42
1.000	1.45	6.967	2.42	12.933	6.76	18.90	2.42
1.033	1.45	7.000	2.42	12.967	6.76	18.93	2.42
1.067	1.45	7.033	2.90	13.000	6.76	18.97	2.42
1.100	1.45	7.067	2.90	13.033	6.28	19.00	2.42
1.133	1.45	7.100	2.90	13.067	6.28	19.03	1.93
1.167	1.45	7.133	2.90	13.100	6.28	19.07	1.93
1.200	1.45	7.167	2.90	13.133	6.28	19.10	1.93
1.233	1.45	7.200	2.90	13.167	6.28	19.13	1.93
1.267	1.21	7.233	2.90	13.200	6.28	19.17	1.93
1.300	0.97	7.267	2.66	13.233	6.28	19.20	1.93
1.333	0.97	7.300	2.42	13.267	5.80	19.23	1.93
1.367	0.97	7.333	2.42	13.300	5.32	19.27	2.18
1.400	0.97	7.367	2.42	13.333	5.32	19.30	2.42
1.433	0.97	7.400	2.42	13.367	5.32	19.33	2.42
1.467	0.97	7.433	2.42	13.400	5.32	19.37	2.42
1.500	0.97	7.467	2.42	13.433	5.32	19.40	2.42
1.533	1.45	7.500	2.42	13.467	5.32	19.43	2.42
1.567	1.45	7.533	2.90	13.500	5.31	19.47	2.42
1.600	1.45	7.567	2.90	13.533	4.83	19.50	2.42
1.633	1.45	7.600	2.90	13.567	4.83	19.53	1.93
1.667	1.45	7.633	2.90	13.600	4.83	19.57	1.93
1.700	1.45	7.667	2.90	13.633	4.83	19.60	1.93
1.733	1.45	7.700	2.90	13.667	4.83	19.63	1.93
1.767	1.45	7.733	2.90	13.700	4.83	19.67	1.93
1.800	1.45	7.767	2.90	13.733	4.83	19.70	1.93
1.833	1.45	7.800	2.90	13.767	4.35	19.73	1.93
1.867	1.45	7.833	2.90	13.800	3.87	19.77	1.69
1.900	1.45	7.867	2.90	13.833	3.87	19.80	1.45
1.933	1.45	7.900	2.90	13.867	3.87	19.83	1.45
1.967	1.45	7.933	2.90	13.900	3.87	19.87	1.45
2.000	1.45	7.967	2.90	13.933	3.87	19.90	1.45
2.033	1.93	8.000	2.90	13.967	3.87	19.93	1.45
2.067	1.93	8.033	3.38	14.000	3.86	19.97	1.45
2.100	1.93	8.067	3.38	14.033	3.38	20.00	1.45
2.133	1.93	8.100	3.38	14.067	3.38	20.03	1.45
2.167	1.93	8.133	3.38	14.100	3.38	20.07	1.45
2.200	1.93	8.167	3.38	14.133	3.38	20.10	1.45
2.233	1.93	8.200	3.38	14.167	3.38	20.13	1.45
2.267	1.69	8.233	3.38	14.200	3.38	20.17	1.45
2.300	1.45	8.267	3.38	14.233	3.38	20.20	1.45
2.333	1.45	8.300	3.38	14.267	3.63	20.23	1.45
2.367	1.45	8.333	3.38	14.300	3.87	20.27	1.45
2.400	1.45	8.367	3.38	14.333	3.87	20.30	1.45
2.433	1.45	8.400	3.38	14.367	3.87	20.33	1.45
2.467	1.45	8.433	3.38	14.400	3.87	20.37	1.45
2.500	1.45	8.467	3.38	14.433	3.87	20.40	1.45
2.533	1.45	8.500	3.38	14.467	3.87	20.43	1.45
2.567	1.45	8.533	3.38	14.500	3.86	20.47	1.45
2.600	1.45	8.567	3.38	14.533	3.38	20.50	1.45
2.633	1.45	8.600	3.38	14.567	3.38	20.53	1.45
2.667	1.45	8.633	3.38	14.600	3.38	20.57	1.45
2.700	1.45	8.667	3.38	14.633	3.38	20.60	1.45
2.733	1.45	8.700	3.38	14.667	3.38	20.63	1.45
2.767	1.45	8.733	3.38	14.700	3.38	20.67	1.45
2.800	1.45	8.767	3.62	14.733	3.38	20.70	1.45
2.833	1.45	8.800	3.87	14.767	3.63	20.73	1.45
2.867	1.45	8.833	3.87	14.800	3.87	20.77	1.45

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2.900	1.45	8.867	3.87	14.833	3.87	20.80	1.45
2.933	1.45	8.900	3.87	14.867	3.87	20.83	1.45
2.967	1.45	8.933	3.87	14.900	3.87	20.87	1.45
3.000	1.45	8.967	3.87	14.933	3.87	20.90	1.45
3.033	1.93	9.000	3.87	14.967	3.87	20.93	1.45
3.067	1.93	9.033	3.87	15.000	3.86	20.97	1.45
3.100	1.93	9.067	3.87	15.033	3.38	21.00	1.45
3.133	1.93	9.100	3.87	15.067	3.38	21.03	1.45
3.167	1.93	9.133	3.87	15.100	3.38	21.07	1.45
3.200	1.93	9.167	3.87	15.133	3.38	21.10	1.45
3.233	1.93	9.200	3.87	15.167	3.38	21.13	1.45
3.267	1.69	9.233	3.87	15.200	3.38	21.17	1.45
3.300	1.45	9.267	4.11	15.233	3.38	21.20	1.45
3.333	1.45	9.300	4.35	15.267	3.63	21.23	1.45
3.367	1.45	9.333	4.35	15.300	3.87	21.27	1.45
3.400	1.45	9.367	4.35	15.333	3.87	21.30	1.45
3.433	1.45	9.400	4.35	15.367	3.87	21.33	1.45
3.467	1.45	9.433	4.35	15.400	3.87	21.37	1.45
3.500	1.45	9.467	4.35	15.433	3.87	21.40	1.45
3.533	1.45	9.500	4.35	15.467	3.87	21.43	1.45
3.567	1.45	9.533	4.35	15.500	3.86	21.47	1.45
3.600	1.45	9.567	4.35	15.533	3.38	21.50	1.45
3.633	1.45	9.600	4.35	15.567	3.38	21.53	1.45
3.667	1.45	9.633	4.35	15.600	3.38	21.57	1.45
3.700	1.45	9.667	4.35	15.633	3.38	21.60	1.45
3.733	1.45	9.700	4.35	15.667	3.38	21.63	1.45
3.767	1.69	9.733	4.35	15.700	3.38	21.67	1.45
3.800	1.93	9.767	4.83	15.733	3.38	21.70	1.45
3.833	1.93	9.800	5.32	15.767	2.90	21.73	1.45
3.867	1.93	9.833	5.32	15.800	2.42	21.77	1.45
3.900	1.93	9.867	5.32	15.833	2.42	21.80	1.45
3.933	1.93	9.900	5.32	15.867	2.42	21.83	1.45
3.967	1.93	9.933	5.32	15.900	2.42	21.87	1.45
4.000	1.93	9.967	5.32	15.933	2.42	21.90	1.45
4.033	1.93	10.000	5.32	15.967	2.42	21.93	1.45
4.067	1.93	10.033	5.80	16.000	2.41	21.97	1.45
4.100	1.93	10.067	5.80	16.033	1.93	22.00	1.45
4.133	1.93	10.100	5.80	16.067	1.93	22.03	1.45
4.167	1.93	10.133	5.80	16.100	1.93	22.07	1.45
4.200	1.93	10.167	5.80	16.133	1.93	22.10	1.45
4.233	1.93	10.200	5.80	16.167	1.93	22.13	1.45
4.267	1.93	10.233	5.80	16.200	1.93	22.17	1.45
4.300	1.93	10.267	6.52	16.233	1.93	22.20	1.45
4.333	1.93	10.300	7.25	16.267	2.18	22.23	1.45
4.367	1.93	10.333	7.25	16.300	2.42	22.27	1.45
4.400	1.93	10.367	7.25	16.333	2.42	22.30	1.45
4.433	1.93	10.400	7.25	16.367	2.42	22.33	1.45
4.467	1.93	10.433	7.25	16.400	2.42	22.37	1.45
4.500	1.93	10.467	7.25	16.433	2.42	22.40	1.45
4.533	1.93	10.500	7.25	16.467	2.42	22.43	1.45
4.567	1.93	10.533	7.73	16.500	2.41	22.47	1.45
4.600	1.93	10.567	7.73	16.533	1.93	22.50	1.45
4.633	1.93	10.600	7.73	16.567	1.93	22.53	1.45
4.667	1.93	10.633	7.73	16.600	1.93	22.57	1.45
4.700	1.93	10.667	7.73	16.633	1.93	22.60	1.45
4.733	1.93	10.700	7.73	16.667	1.93	22.63	1.45
4.767	1.93	10.733	7.73	16.700	1.93	22.67	1.45
4.800	1.93	10.767	9.67	16.733	1.93	22.70	1.45
4.833	1.93	10.800	11.60	16.767	2.18	22.73	1.45
4.867	1.93	10.833	11.60	16.800	2.42	22.77	1.45
4.900	1.93	10.867	11.60	16.833	2.42	22.80	1.45
4.933	1.93	10.900	11.60	16.867	2.42	22.83	1.45
4.967	1.93	10.933	11.60	16.900	2.42	22.87	1.45

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5.000	1.93	10.967	11.60	16.933	2.42	22.90	1.45
5.033	1.93	11.000	11.60	16.967	2.42	22.93	1.45
5.067	1.93	11.033	11.60	17.000	2.41	22.97	1.45
5.100	1.93	11.067	11.60	17.033	1.93	23.00	1.45
5.133	1.93	11.100	11.60	17.067	1.93	23.03	1.45
5.167	1.93	11.133	11.60	17.100	1.93	23.07	1.45
5.200	1.93	11.167	11.60	17.133	1.93	23.10	1.45
5.233	1.93	11.200	11.60	17.167	1.93	23.13	1.45
5.267	1.93	11.233	11.60	17.200	1.93	23.17	1.45
5.300	1.93	11.267	23.70	17.233	1.93	23.20	1.45
5.333	1.93	11.300	35.76	17.267	2.18	23.23	1.45
5.367	1.93	11.333	35.76	17.300	2.42	23.27	1.45
5.400	1.93	11.367	35.76	17.333	2.42	23.30	1.45
5.433	1.93	11.400	35.76	17.367	2.42	23.33	1.45
5.467	1.93	11.433	35.76	17.400	2.42	23.37	1.45
5.500	1.93	11.467	35.76	17.433	2.42	23.40	1.45
5.533	1.93	11.500	35.89	17.467	2.42	23.43	1.45
5.567	1.93	11.533	147.86	17.500	2.41	23.47	1.45
5.600	1.93	11.567	147.86	17.533	1.93	23.50	1.45
5.633	1.93	11.600	147.86	17.567	1.93	23.53	1.45
5.667	1.93	11.633	147.86	17.600	1.93	23.57	1.45
5.700	1.93	11.667	147.86	17.633	1.93	23.60	1.45
5.733	1.93	11.700	147.86	17.667	1.93	23.63	1.45
5.767	1.93	11.733	147.86	17.700	1.93	23.67	1.45
5.800	1.93	11.767	82.46	17.733	1.93	23.70	1.45
5.833	1.93	11.800	17.40	17.767	2.18	23.73	1.45
5.867	1.93	11.833	17.40	17.800	2.42	23.77	0.73
5.900	1.93	11.867	17.40	17.833	2.42		
5.933	1.93	11.900	17.40	17.867	2.42		
5.967	1.93	11.933	17.40	17.900	2.42		

Unit Hyd Qpeak (cms)= 0.366

PEAK FLOW (cms)= 0.124 (i)  
 TIME TO PEAK (hrs)= 11.833  
 RUNOFF VOLUME (mm)= 28.396  
 TOTAL RAINFALL (mm)= 120.437  
 RUNOFF COEFFICIENT = 0.236

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD ( 0203)	Area (ha)=	2.89	Curve Number (CN)=	42.9
ID= 1 DT= 2.0 min	Ia (mm)=	8.98	# of Linear Res. (N)=	3.00
	U. H. Tp(hrs)=	0.18		

Unit Hyd Qpeak (cms)= 0.613

PEAK FLOW (cms)= 0.207 (i)  
 TIME TO PEAK (hrs)= 11.833  
 RUNOFF VOLUME (mm)= 27.658  
 TOTAL RAINFALL (mm)= 120.437  
 RUNOFF COEFFICIENT = 0.230

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0057)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3				

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-----

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0202):	1.63	0.124	11.83	28.40
+ ID2= 2 ( 0203):	2.89	0.207	11.83	27.66
=====				
ID = 3 ( 0057):	4.52	0.332	11.83	27.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----

CALIB				
NASHYD ( 0204)	Area (ha)=	0.64	Curve Number (CN)=	48.1
ID= 1 DT= 2.0 min	Ia (mm)=	9.20	# of Linear Res. (N)=	3.00
	U. H. Tp(hrs)=	0.33		

Unit Hyd Qpeak (cms)= 0.074

PEAK FLOW (cms)= 0.036 (i)

TIME TO PEAK (hrs)= 11.967

RUNOFF VOLUME (mm)= 32.094

TOTAL RAINFALL (mm)= 120.437

RUNOFF COEFFICIENT = 0.266

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----

ADD HYD ( 0058)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0204):	0.64	0.036	11.97	32.09
+ ID2= 2 ( 0057):	4.52	0.332	11.83	27.92
=====				
ID = 3 ( 0058):	5.16	0.361	11.83	28.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----

CALIB				
STANDHYD ( 0201)	Area Total	(ha)= 2.20	Dir. Conn. (%)=	21.00
ID= 1 DT= 5.0 min	Imp(%)=	43.00		
	IMPERVIOUS	PVIOUS (i)		
Surface Area (ha)=	0.95	1.25		
Dep. Storage (mm)=	1.00	1.50		
Average Slope (%)=	1.00	1.00		
Length (m)=	121.11	40.00		
Mannings n =	0.013	0.250		

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	1.45	6.083	2.42	12.083	17.40	18.08	1.93
0.167	1.45	6.167	2.42	12.167	17.40	18.17	1.93
0.250	1.45	6.250	2.42	12.250	17.40	18.25	1.93
0.333	0.97	6.333	1.93	12.333	9.18	18.33	2.42
0.417	0.97	6.417	1.93	12.417	9.18	18.42	2.42
0.500	0.97	6.500	1.93	12.500	9.18	18.50	2.42

SCS.txt

0.583	1.45	6.583	2.42	12.583	8.70	18.58	1.93
0.667	1.45	6.667	2.42	12.667	8.70	18.67	1.93
0.750	1.45	6.750	2.42	12.750	8.70	18.75	1.93
0.833	1.45	6.833	2.42	12.833	6.77	18.83	2.42
0.917	1.45	6.917	2.42	12.917	6.76	18.92	2.42
1.000	1.45	7.000	2.42	13.000	6.76	19.00	2.42
1.083	1.45	7.083	2.90	13.083	6.28	19.08	1.93
1.167	1.45	7.167	2.90	13.167	6.28	19.17	1.93
1.250	1.45	7.250	2.90	13.250	6.28	19.25	1.93
1.333	0.97	7.333	2.42	13.333	5.32	19.33	2.42
1.417	0.97	7.417	2.42	13.417	5.32	19.42	2.42
1.500	0.97	7.500	2.42	13.500	5.32	19.50	2.42
1.583	1.45	7.583	2.90	13.583	4.83	19.58	1.93
1.667	1.45	7.667	2.90	13.667	4.83	19.67	1.93
1.750	1.45	7.750	2.90	13.750	4.83	19.75	1.93
1.833	1.45	7.833	2.90	13.833	3.87	19.83	1.45
1.917	1.45	7.917	2.90	13.917	3.87	19.92	1.45
2.000	1.45	8.000	2.90	14.000	3.87	20.00	1.45
2.083	1.93	8.083	3.38	14.083	3.38	20.08	1.45
2.167	1.93	8.167	3.38	14.167	3.38	20.17	1.45
2.250	1.93	8.250	3.38	14.250	3.38	20.25	1.45
2.333	1.45	8.333	3.38	14.333	3.87	20.33	1.45
2.417	1.45	8.417	3.38	14.417	3.87	20.42	1.45
2.500	1.45	8.500	3.38	14.500	3.87	20.50	1.45
2.583	1.45	8.583	3.38	14.583	3.38	20.58	1.45
2.667	1.45	8.667	3.38	14.667	3.38	20.67	1.45
2.750	1.45	8.750	3.38	14.750	3.38	20.75	1.45
2.833	1.45	8.833	3.87	14.833	3.87	20.83	1.45
2.917	1.45	8.917	3.87	14.917	3.87	20.92	1.45
3.000	1.45	9.000	3.87	15.000	3.87	21.00	1.45
3.083	1.93	9.083	3.87	15.083	3.38	21.08	1.45
3.167	1.93	9.167	3.87	15.167	3.38	21.17	1.45
3.250	1.93	9.250	3.87	15.250	3.38	21.25	1.45
3.333	1.45	9.333	4.35	15.333	3.87	21.33	1.45
3.417	1.45	9.417	4.35	15.417	3.87	21.42	1.45
3.500	1.45	9.500	4.35	15.500	3.87	21.50	1.45
3.583	1.45	9.583	4.35	15.583	3.38	21.58	1.45
3.667	1.45	9.667	4.35	15.667	3.38	21.67	1.45
3.750	1.45	9.750	4.35	15.750	3.38	21.75	1.45
3.833	1.93	9.833	5.32	15.833	2.42	21.83	1.45
3.917	1.93	9.917	5.32	15.917	2.42	21.92	1.45
4.000	1.93	10.000	5.32	16.000	2.42	22.00	1.45
4.083	1.93	10.083	5.80	16.083	1.93	22.08	1.45
4.167	1.93	10.167	5.80	16.167	1.93	22.17	1.45
4.250	1.93	10.250	5.80	16.250	1.93	22.25	1.45
4.333	1.93	10.333	7.25	16.333	2.42	22.33	1.45
4.417	1.93	10.417	7.25	16.417	2.42	22.42	1.45
4.500	1.93	10.500	7.25	16.500	2.42	22.50	1.45
4.583	1.93	10.583	7.73	16.583	1.93	22.58	1.45
4.667	1.93	10.667	7.73	16.667	1.93	22.67	1.45
4.750	1.93	10.750	7.73	16.750	1.93	22.75	1.45
4.833	1.93	10.833	11.60	16.833	2.42	22.83	1.45
4.917	1.93	10.917	11.60	16.917	2.42	22.92	1.45
5.000	1.93	11.000	11.60	17.000	2.42	23.00	1.45
5.083	1.93	11.083	11.60	17.083	1.93	23.08	1.45
5.167	1.93	11.167	11.60	17.167	1.93	23.17	1.45
5.250	1.93	11.250	11.60	17.250	1.93	23.25	1.45
5.333	1.93	11.333	35.75	17.333	2.42	23.33	1.45
5.417	1.93	11.417	35.76	17.417	2.42	23.42	1.45
5.500	1.93	11.500	35.76	17.500	2.42	23.50	1.45
5.583	1.93	11.583	147.85	17.583	1.93	23.58	1.45
5.667	1.93	11.667	147.86	17.667	1.93	23.67	1.45
5.750	1.93	11.750	147.86	17.750	1.93	23.75	1.45

SCS.txt  
 5.833 1.93 | 11.833 17.41 | 17.833 2.42 |  
 5.917 1.93 | 11.917 17.40 | 17.917 2.42 |  
 6.000 1.93 | 12.000 17.40 | 18.000 2.42 |

Max. Eff. Inten. (mm/hr)= 147.86 197.35  
 over (min)= 5.00 10.00  
 Storage Coeff. (min)= 2.45 (ii) 9.07 (ii)  
 Unit Hyd. Tpeak (min)= 5.00 10.00  
 Unit Hyd. peak (cms)= 0.30 0.12  
 PEAK FLOW (cms)= 0.19 0.53  
 TIME TO PEAK (hrs)= 11.75 11.75  
 RUNOFF VOLUME (mm)= 119.44 54.09  
 TOTAL RAINFALL (mm)= 120.44 120.44  
 RUNOFF COEFFICIENT = 0.99 0.45

\*TOTALS\*  
 0.722 (iii)  
 11.75  
 67.81  
 120.44  
 0.56

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0301)  
 IN= 2---> OUT= 1  
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.1680	0.1590	0.5570
0.0440	0.2080	0.3520	0.7230
0.0990	0.3020	1.2920	0.9160
0.1320	0.4180	2.0620	1.0230

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 ( 0201)	2.200	0.722	11.75	67.81
OUTFLOW: ID= 1 ( 0301)	2.200	0.000	0.00	0.00

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.00  
 TIME SHIFT OF PEAK FLOW (min) = \*\*\*\*\*  
 MAXIMUM STORAGE USED (ha.m.) = 0.0813

ADD HYD ( 0059)  
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0301):	2.20	0.000	0.00	0.00
+ ID2= 2 ( 0058):	5.16	0.361	11.83	28.44
ID = 3 ( 0059):	7.36	0.361	11.83	19.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 STANDHYD ( 0205)  
 ID= 1 DT= 5.0 min

Area (ha)= 0.03  
 Total Imp(%)= 56.00  
 Dir. Conn. (%) = 28.00  
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SCS.txt

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.02	0.01
Dep. Storage (mm)=	1.00	1.50
Average Slope (%)=	1.00	1.00
Length (m)=	14.14	20.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 147.86 234.43  
 over (min)= 5.00 5.00  
 Storage Coeff. (min)= 0.68 (ii) 4.75 (ii)  
 Unit Hyd. Tpeak (min)= 5.00 5.00  
 Unit Hyd. peak (cms)= 0.34 0.22

PEAK FLOW (cms)= 0.00 0.01  
 TIME TO PEAK (hrs)= 11.75 11.75  
 RUNOFF VOLUME (mm)= 119.44 57.66  
 TOTAL RAINFALL (mm)= 120.44 120.44  
 RUNOFF COEFFICIENT = 0.99 0.48

\*TOTALS\*  
 0.012 (iii)  
 11.75  
 68.86  
 120.44  
 0.57

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:  
 Fo (mm/hr)= 50.00 K (1/hr)= 2.00  
 Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0060)  
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0205):	0.03	0.012	11.75	68.86
+ ID2= 2 ( 0059):	7.36	0.361	11.83	19.94
ID = 3 ( 0060):	7.39	0.365	11.83	20.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB  
 NASHYD ( 0207)  
 ID= 1 DT= 5.0 min

Area (ha)= 0.78  
 la (mm)= 4.87  
 U. H. Tp(hrs)= 0.19  
 Curve Number (CN)= 66.3  
 # of Linear Res. (N)= 3.00

Unit Hyd Opeak (cms)= 0.157

PEAK FLOW (cms)= 0.111 (i)  
 TIME TO PEAK (hrs)= 11.833  
 RUNOFF VOLUME (mm)= 54.459  
 TOTAL RAINFALL (mm)= 120.438  
 RUNOFF COEFFICIENT = 0.452

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0061)

		SCS.txt			
1 + 2 = 3		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0207):		0.78	0.111	11.83	54.46
+ ID2= 2 ( 0060):		7.39	0.365	11.83	20.14
=====					
ID = 3 ( 0061):		8.17	0.477	11.83	23.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ( 0206) ID= 1 DT= 5.0 min		Area (ha)= 0.73 Total Imp(%)= 32.00	Dir. Conn. (%)= 13.00	
		IMPERVIOUS	PERVIOUS (i)	
Surface Area	(ha)=	0.23	0.50	
Dep. Storage	(mm)=	1.00	1.50	
Average Slope	(%)=	1.00	2.00	
Length	(m)=	69.76	40.00	
Mannings n	=	0.013	0.250	
Max. Eff. Inten.	(mm/hr)=	147.86	181.54	
over	(min)=	5.00	10.00	
Storage Coeff.	(min)=	1.76 (ii)	7.32 (ii)	
Unit Hyd. Tpeak	(min)=	5.00	10.00	
Unit Hyd. peak	(cms)=	0.32	0.13	
				*TOTALS*
PEAK FLOW	(cms)=	0.04	0.21	0.248 (iii)
TIME TO PEAK	(hrs)=	11.75	11.75	11.75
RUNOFF VOLUME	(mm)=	119.44	52.24	60.98
TOTAL RAINFALL	(mm)=	120.44	120.44	120.44
RUNOFF COEFFICIENT	=	0.99	0.43	0.51

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

\*\*\*\*\* WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%  
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) HORTONS EQUATION SELECTED FOR PERVIOUS LOSSES:

Fo (mm/hr)= 50.00 K (1/hr)= 2.00

Fc (mm/hr)= 7.50 Cum. Inf. (mm)= 0.00

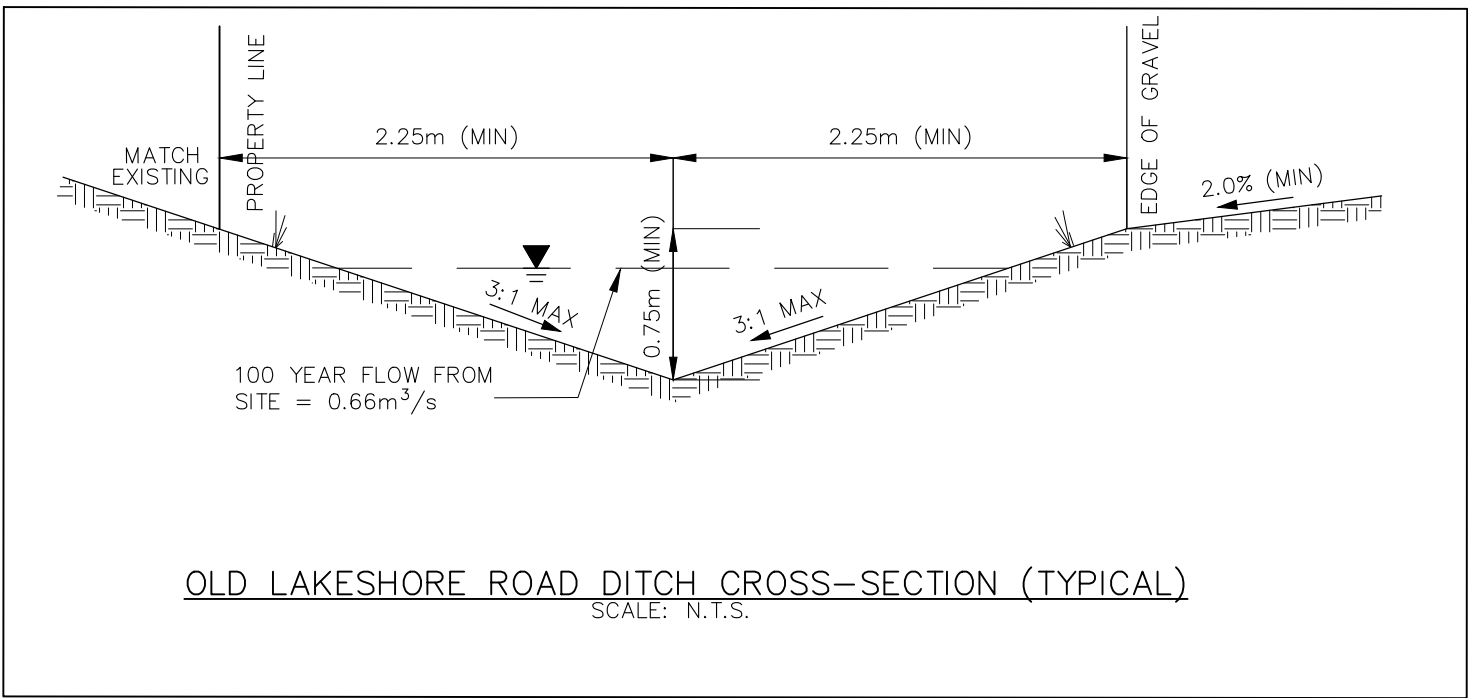
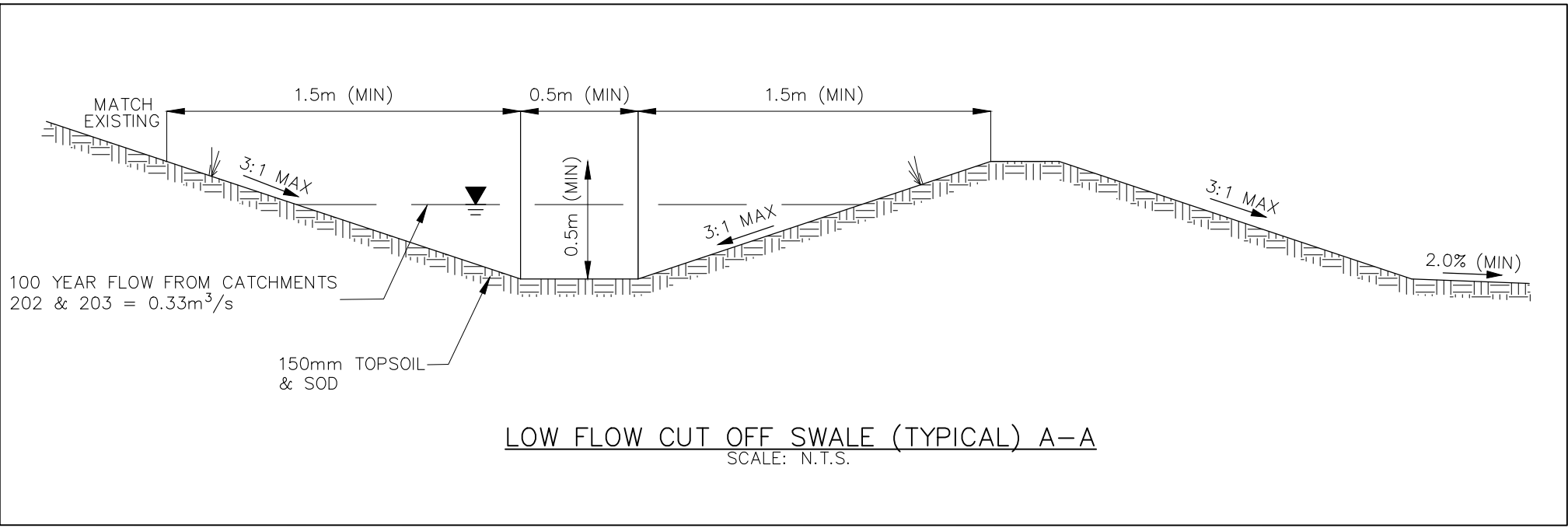
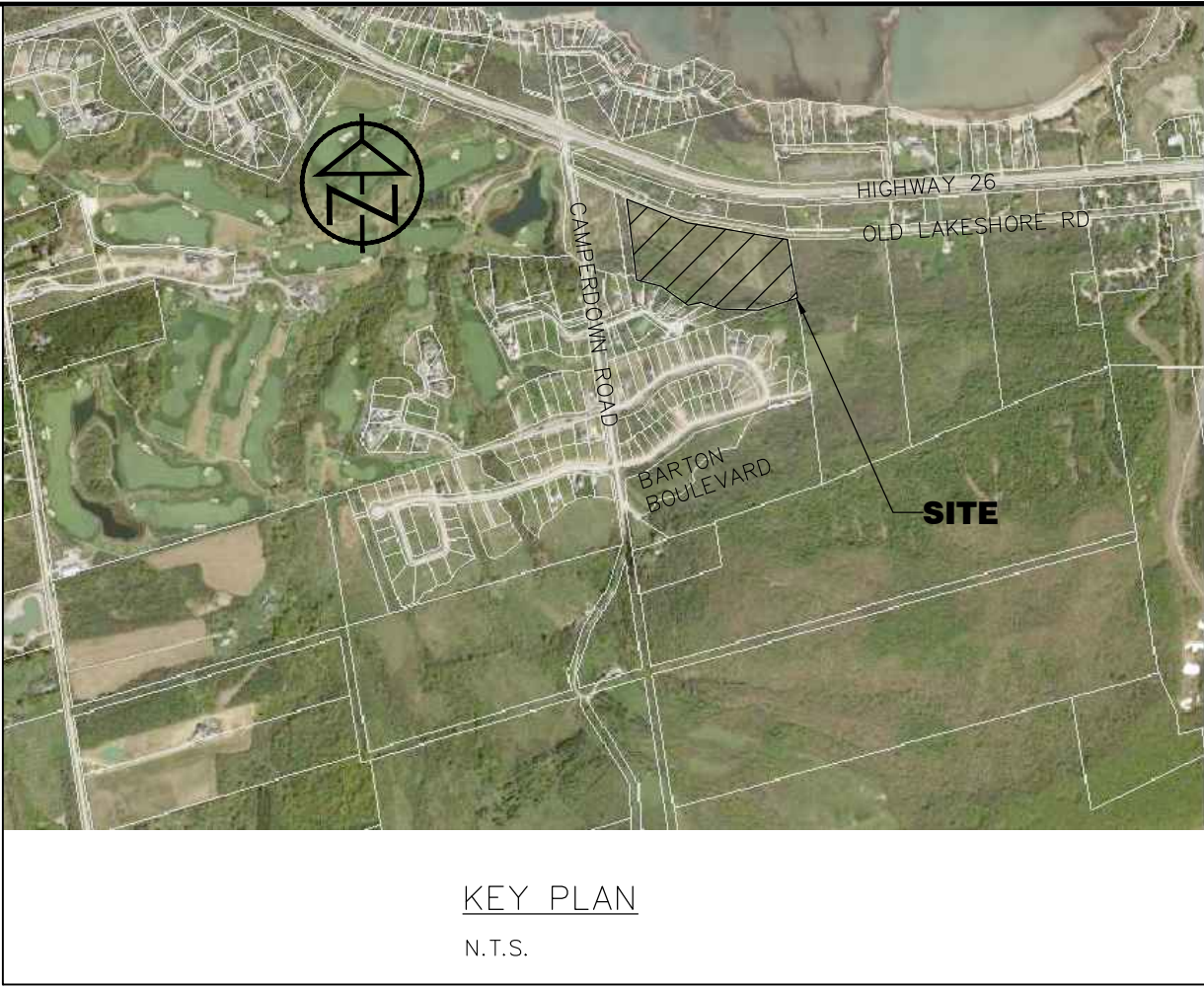
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0401) 1 + 2 = 3		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0206):		0.73	0.248	11.75	60.98
+ ID2= 2 ( 0061):		8.17	0.477	11.83	23.41
=====					
ID = 3 ( 0401):		8.90	0.682	11.80	26.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.





LEGEND		
PROPERTY LINE	---	
PROPOSED CENTERLINE ROAD	---	
EXISTING EDGE OF GRAVEL ROAD	---	
PROPOSED OVERLAND FLOW DIRECTION	➔	
PROPOSED/EXISTING ELEVATIONS	190.00 190.00	
PROPOSED SWALE	1.0%	
PROPOSED GRADE/DIRECTION	1.0%	

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**BENCHMARKS**  
TBM1 - ELEVATION 211.950  
TOP NUT FIRE HYDRANT LOCATED AT THE SOUTH WEST CORNER OF LOT 11.

**NOTES**  
LEGAL SURVEY, TOPOGRAPHIC INFORMATION AND LOT DIMENSIONS SHOWN ON THIS PLAN ARE TAKEN FROM A SURVEY PLAN PREPARED BY ZUBEK, EMO, PATTEN & THOMSEN, WHICH MAY NOT BE FINAL AND ARE NOT GUARANTEED. THE FINAL REGISTERED PLAN OF SUBDIVISION SHALL BE REFERRED TO FOR CONFIRMATION OF THE DATA.

No.	REVISION DESCRIPTION	DATE
1.	SUBMISSION FOR DRAFT PLAN APPROVAL	NOV/2019
2.	SUBMISSION FOR DRAFT PLAN APPROVAL	JUNE/2020
3.	TOWN COMMENTS	OCT/2020

ENGINEER STAMP

**CAMPERDOWN CONDOMINIUMS**  
**TOWN OF THE BLUE MOUNTAINS**

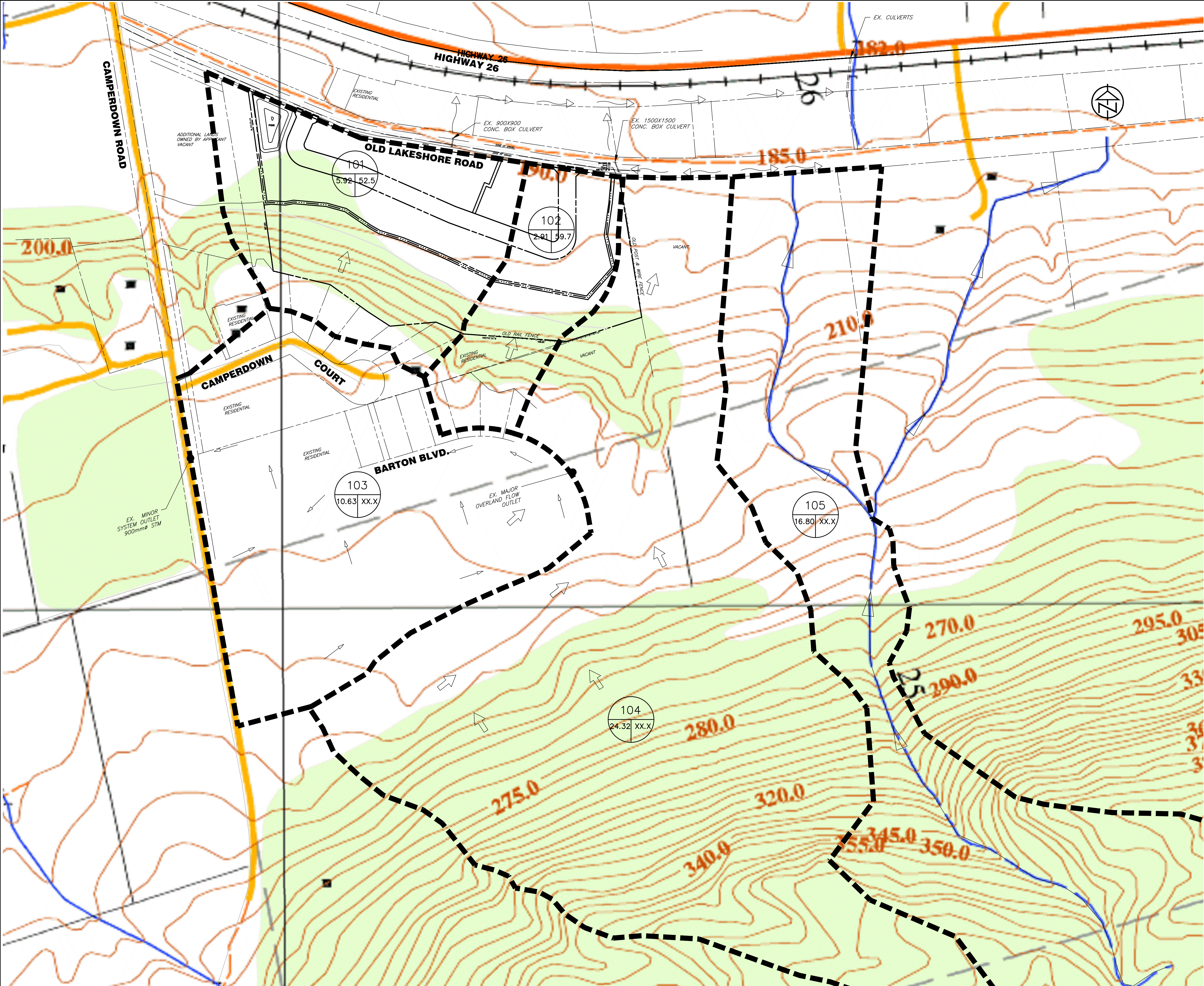
SITE GRADING PLAN

DESIGN: AS  
DRAWN: RD/AC  
CHECK: RS

FILE: 117304  
DATE: DEC., 2017  
SCALE: 1:1000

DWG: **SG-1**





KEY PLAN  
N.T.S.

LEGEND

- PROPERTY LINE
- EXISTING EDGE OF ASPHALT
- EXISTING DRAINAGE BOUNDARY
- EXISTING DRAINAGE AREA ID
- EXISTING DRAINAGE AREA (ha)
- EXISTING CN
- EXISTING MAJOR/OVERLAND FLOW DIRECTION
- EXISTING SWALE/DITCH
- EXISTING WATER COURSE
- EXISTING MINOR STORM FLOW DIRECTION

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**BENCHMARKS**  
TBM1 - ELEVATION 211.950  
TOP NUT FIRE HYDRANT LOCATED AT THE SOUTH WEST CORNER OF LOT 11.

**NOTES**  
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No.	REVISION DESCRIPTION	DATE
1.	SUBMISSION FOR DRAFT PLAN APPROVAL	NOV/2019
2.	SUBMISSION FOR DRAFT PLAN APPROVAL	JUNE/2020

ENGINEER STAMP



**CAMPERDOWN CONDOMINIUMS**  
**TOWN OF THE BLUE MOUNTAINS**

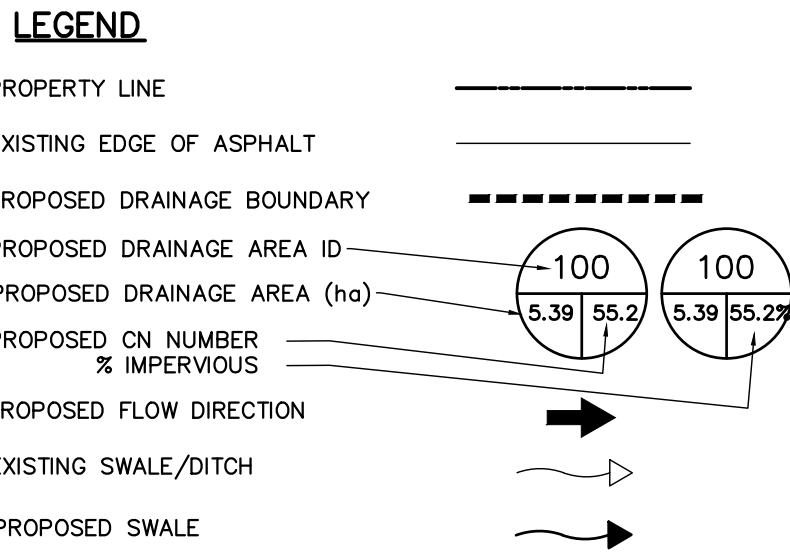
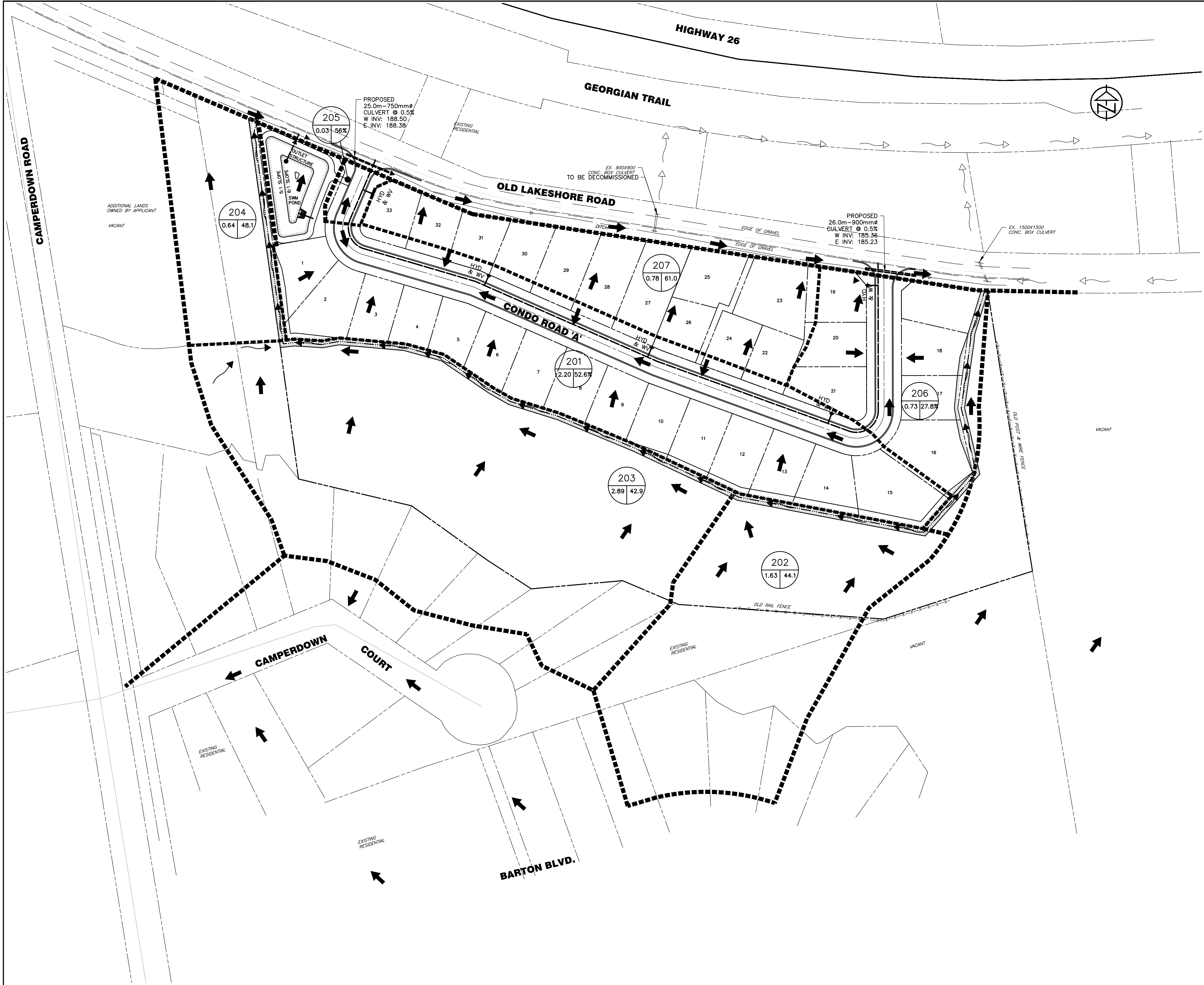
**PRE-DEVELOPMENT**  
**DRAINAGE PLAN**



DESIGN: AS	FILE: 117304	DWG:
DRAWN: RD/AC	DATE: DEC., 2017	
CHECK: RS	SCALE: 1:2000	

**DP-1**





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**BENCHMARKS**

TBM1 - ELEVATION 211.950  
TOP NUT FIRE HYDRANT LOCATED AT THE SOUTH WEST CORNER OF LOT 11.

**NOTES**

LEGAL SURVEY, TOPOGRAPHIC INFORMATION AND LOT DIMENSIONS SHOWN ON THIS PLAN ARE TAKEN FROM A SURVEY PLAN PREPARED BY ZUBEK, EMO, PATTEN & THOMSEN, WHICH MAY NOT BE FINAL AND ARE NOT GUARANTEED. THE FINAL REGISTERED PLAN OF SUBDIVISION SHALL BE REFERRED TO FOR CONFIRMATION OF THE DATA.

No.	REVISION DESCRIPTION	DATE
1.	SUBMISSION FOR DRAFT PLAN APPROVAL	NOV/2019
2.	SUBMISSION FOR DRAFT PLAN APPROVAL	JUNE/2020
3.	TOWN COMMENTS	OCT/2020

**ENGINEER STAMP**

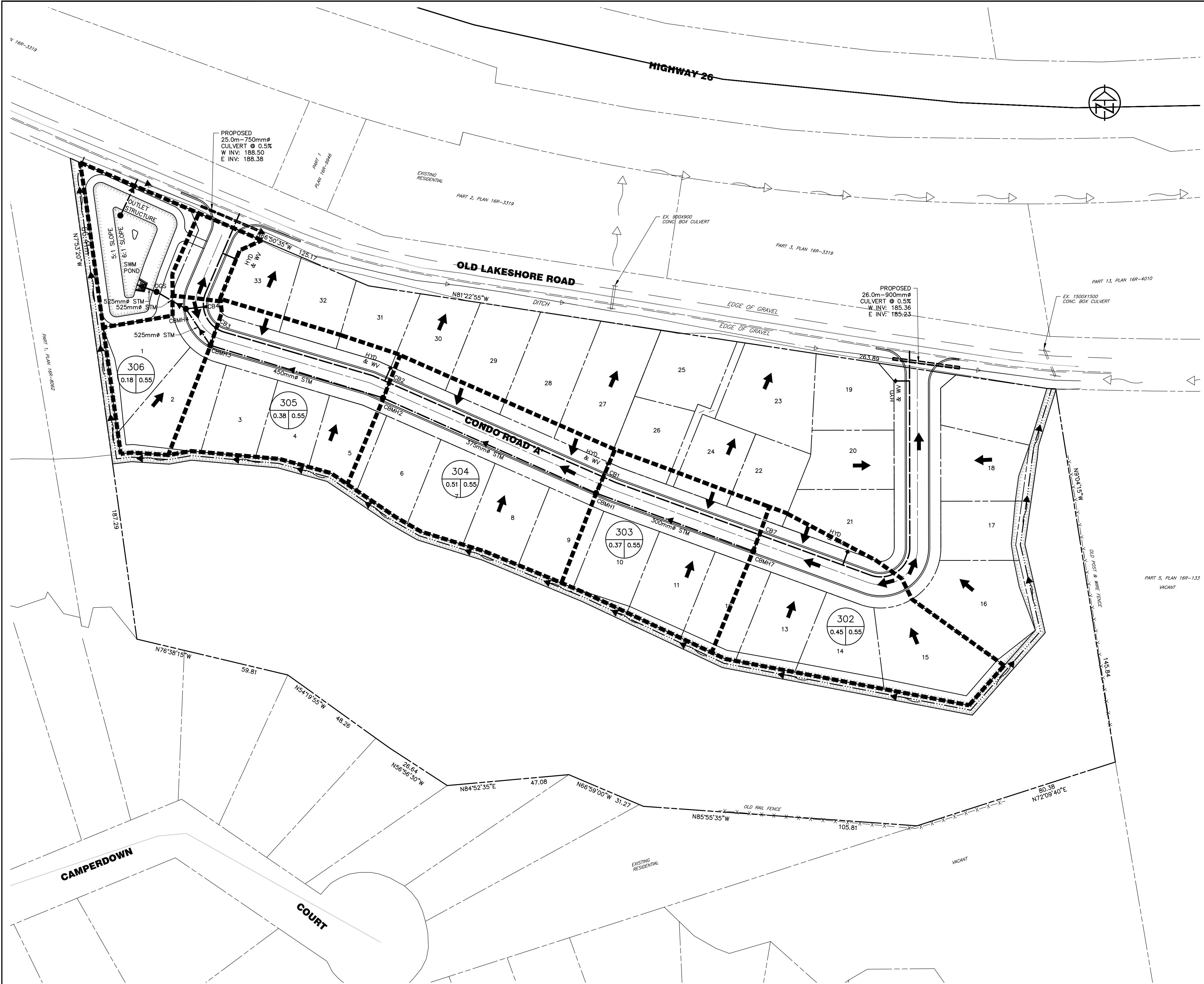
**CAMPERDOWN CONDOMINIUMS**  
**TOWN OF THE BLUE MOUNTAINS**

**POST-DEVELOPMENT DRAINAGE PLAN**

**TATHAM ENGINEERING**

DESIGN: AS	FILE: 117304	<b>DP-2</b>
DRAWN: RD/AC	DATE: DEC., 2017	
CHECK: RS	SCALE: 1:1000	





KEY PLAN  
N.T.S.

LEGEND

- PROPERTY LINE
- EXISTING EDGE OF ASPHALT
- PROPOSED DRAINAGE BOUNDARY
- PROPOSED DRAINAGE AREA ID
- PROPOSED DRAINAGE AREA (ha)
- COMPOSITE RUNOFF COEFFICIENT
- OVERLAND FLOW DIRECTION
- EXISTING SWALE/DITCH
- PROPOSED SWALE

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CONTRACTOR MUST VERIFY ALL DIMENSIONS AND BE RESPONSIBLE FOR SAME. ANY DISCREPANCIES MUST BE REPORTED TO THE ENGINEER BEFORE COMMENCING WORK. DRAWINGS ARE NOT TO BE SCALED.  
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**BENCHMARKS**  
TBM1 - ELEVATION 211.950  
TOP NUT FIRE HYDRANT LOCATED AT THE SOUTH WEST CORNER OF LOT 11.

**NOTES**  
LEGAL SURVEY, TOPOGRAPHIC INFORMATION AND LOT DIMENSIONS SHOWN ON THIS PLAN ARE TAKEN FROM A SURVEY PLAN PREPARED BY ZUBEK, EMO, PATTEN & THOMSEN, WHICH MAY NOT BE FINAL AND ARE NOT GUARANTEED. THE FINAL REGISTERED PLAN OF SUBDIVISION SHALL BE REFERRED TO FOR CONFIRMATION OF THE DATA.

No.	REVISION DESCRIPTION	DATE
1.	SUBMISSION FOR DRAFT PLAN APPROVAL	NOV/2019
2.	SUBMISSION FOR DRAFT PLAN APPROVAL	JUNE/2020
3.	TOWN COMMENTS	OCT/2020

ENGINEER STAMP



**CAMPERDOWN CONDOMINIUMS**  
**TOWN OF THE BLUE MOUNTAINS**

**STORMWATER CATCHMENT AREA**  
**PLAN**



DESIGN: AS	FILE: 117304	DWG:
DRAWN: RD/AC	DATE: DEC., 2017	
CHECK: RS	SCALE: 1:750	

**STM-1**