

Prepared By:



## Stormwater Management Report

263512 Southgate Road 26, Southgate  
Proposed Wilder Lake Subdivision  
H. Bye Construction Ltd.

**GMBP File: 218173**

**November 2019**

## **TABLE OF CONTENTS**

<b>1.</b>	<b>INTRODUCTION AND BACKGROUND .....</b>	<b>1</b>
<b>2.</b>	<b>EXISTING CONDITIONS AND DRAINAGE .....</b>	<b>1</b>
2.1	Stormwater Management Criteria.....	2
<b>3.</b>	<b>POST-DEVELOPMENT CONDITIONS AND DRAINAGE .....</b>	<b>2</b>
3.1	Phased Construction .....	3
<b>4.</b>	<b>QUANTITY CONTROL CRITERIA, PARAMETERS AND MODELLING .....</b>	<b>3</b>
4.1	Design Rainfall Events.....	3
4.2	Site Soil Conditions.....	4
4.3	Existing Catchment Areas .....	4
4.4	Post-Development Catchment Areas .....	5
4.5	MIDUSS Quantity Control Modelling Results .....	5
4.6	Major Flow Routes.....	9
<b>5.</b>	<b>STORMWATER QUALITY TREATMENT .....</b>	<b>9</b>
<b>6.</b>	<b>OPERATIONS AND MAINTENANCE PLAN .....</b>	<b>11</b>
<b>7.</b>	<b>SUMMARY .....</b>	<b>12</b>

## **FIGURES**

<b>FIGURE 1</b>	<b>Site Location Map</b>
<b>FIGURE 2</b>	<b>Existing Drainage Conditions</b>
<b>FIGURE 3</b>	<b>Post-Development Drainage Conditions</b>

## **APPENDICES**

### **FIGURES:**

**APPENDIX A: MIDUSS MODELLING – EXISTING CONDITIONS**

**APPENDIX B: MIDUSS MODELLING – POST-DEVELOPMENT CONDITIONS**

**APPENDIX C: SWM FACILITY STAGE-STORAGE-DISCHARGE CALCULATIONS**

**APPENDIX D: MIDUSS MODELLING - ENHANCED GRASS SWALES**

**APPENDIX E: MAINTENANCE CHECKLIST**

**APPENDIX F: DRIVEWAY CULVERT CALCULATIONS AND STORM SEWER DESIGN SHEET**

**APPENDIX G: GEOTECHNICAL INVESTIGATION, CMT ENGINEERING INC.**

---

**STORMWATER MANAGEMENT REPORT  
PROPOSED WILDER LAKE SUBDIVISION**

**263512 SOUTHGATE ROAD 26, SOUTHGATE  
H. BYE CONSTRUCTION LTD.**

**NOVEMBER 2019**

**GMBP FILE: 218173**

---

## **1. INTRODUCTION AND BACKGROUND**

H. Bye Construction Limited proposes to construct a new 20.8 ha residential subdivision on their property located at 263512 Southgate Road 26 in the Township of Southgate, as shown in Figure 1 – Site Location map. For the purposes of this report Southgate Road 26 is assumed to travel in an east-west direction and the subject property is therefore located on the southerly side of the Southgate Road 26 roadway.

The subject property is approximately 50 ha and is currently developed as Homestead Golf Course and Winter Resort, an 18 hole golf course with a clubhouse and restaurant, as well as rental cottages along the shore of Wilder Lake. The subject property is bound by Wilder Lake to the east, forested and agricultural land to the south, agricultural land to the west, and residential and forested agricultural land to the north. Camp Creek Tributary is the outlet of Wilder Lake and flows through the northern portion of the property.

The lands associated with the proposed subdivision consist of an approximately 20 ha portion of the east side of the property. The main roadway through the proposed subdivision is accessed from Southgate Road 26 and follows the existing roadway to the existing Clubhouse, and then loops eastward, south parallel to the shore of Wilder Lake, west parallel with the property boundary, and north back to join with the entrance road just west of the Clubhouse. It includes parcels on both sides of the roadway.

Development is proposed to be completed in two phases. Phase 1 of development includes the roadway and lots up to and along Wilder Lake along with all stormwater management features, and Phase 2 is the completion of the looped roadway and the remainder of the lots.

## **2. EXISTING CONDITIONS AND DRAINAGE**

Under existing conditions, the majority of the golf course is maintained grass, with some small treed areas and some forest land near Wilder Lake and Camp Creek Tributary. Impervious areas include gravel road ways, golf cart paths, rental cottages and shed roofs.

Under existing conditions, drainage is generally overland flow to one of four (4) outlets:

1. Wilder Lake
2. Camp Creek Tributary
3. South Off-site Overland Flow
4. Infiltration in low areas

Due to the undulating topography, areas with no visible outlet were identified and infiltration is believed to occur as the primary outlet. A portion of the property also drains off-site to the treed area to the south of the property.

Local soils are known to be quite pervious. A recently conducted Geotechnical Investigation prepared by CMT Engineering Inc. provided calculations of the infiltration rate. Six (6) boreholes were drilled across the subject property, a sample was taken from each at around 1.5 to 2 m below the surface, and grain size distributions were completed for each sample. Other than topsoil encountered in the top 0.3 m of the boreholes, the remaining approximately 6.0 m of the boreholes typically consisted of gravel and sand. The average hydraulic conductivity of the samples was estimated to be  $3.62 \times 10^{-3}$  cm/s. This is approximately equivalent to 2.2 L/min/m<sup>2</sup>. The Geotechnical Investigation (Dec. 2018) prepared by CMT Engineering Inc. is included as Appendix G.

## 2.1 Stormwater Management Criteria

Based on the existing drainage conditions and the requirements of the Saugeen Valley Conservation Authority, the Stormwater Management (SWM) criteria used to develop the appropriate SWM approach for the proposed development are as follows:

1. Total post-development peak flow rates discharging from the subject property to each of the drainage outlets as well as the property as a whole, are to be attenuated to less than, or equal to, the total existing conditions peak flow rates.
2. Existing drainage outlets from the property should be used as much as possible
3. An "Enhanced" level of water quality treatment (80% TSS Removal) is to be provided for runoff draining from the subject property prior to discharging from the site.

## 3. POST-DEVELOPMENT CONDITIONS AND DRAINAGE

The post-development drainage conditions of the subdivision are generally maintained from existing conditions with runoff to each of the four (4) outlets. The majority of the developed portion of the subdivision ultimately drains to one of three SWM ponds within the subdivision. Each of the SWM ponds encourages infiltration prior to draining to the surface outlets, Wilder Lake or Camp Creek Tributary.

The subdivision development includes approximately 1.4 km of two-lane paved road way, and 29 lots. To be conservative when establishing the impervious area post-development, each lot is assumed to have approximately 300 m<sup>2</sup> of rooftop area and 550 m<sup>2</sup> of driveway and sheds for modelling purposes. The remainder of the property including the back portion of the majority of the lots are proposed to drain uncontrolled to their pre-development outlets.

Three (3) Stormwater Management Ponds are proposed to provide stormwater runoff attenuation of peak flows and quality treatment of runoff draining to it. Two of the SWM Ponds ultimately discharge to Wilder Lake and one discharges to Camp Creek Tributary.

The proposed SWM facilities in Block 30 and Block 31 are designed as infiltration basins with overflow outlets to Wilder Lake. The proposed top of grate elevation (low side) of the outlet ditch inlet catchbasins (DICB) are designed to achieve the required infiltration volume for storm event water quality treatment purposes as well as to infiltrate the majority of the 2 year return frequency storm. The outlets are proposed to be 150 mm Ø and 300 mm Ø respectively, outletting from the DICB with an orifice plate. The outlet diameter of the orifice plate are 100 mm Ø and 125 mm Ø respectively.



The proposed SWM facility in Block 32 utilizes an existing wet ponding area for quantity control, with quality treatment provided upstream of the wet pond. The proposed top of grate elevation of the outlet DICB is designed to allow existing water levels and flooding areas to be maintained as well as meet the required storm water quantity control. The outlet elevation was determined in consultation with the Environmental Consultant preparing the Environmental Impact Study (EIS). Stormwater quality treatment is proposed to be provided by Enhanced Grassed Swales along both sides of the roadway for the entire catchment area upstream of the wet pond. The outlet from the wet pond is proposed to be a 250 mm Ø HDPE pipe and to outlet to an existing watercourse.

Post-development drainage conditions of the undeveloped portions of the subject property are expected to remain unchanged from existing drainage conditions.

### 3.1 Phased Construction

The subdivision is proposed to be constructed in two phases. Phase 1 of construction includes the roadway along lots 1 to 13 and 26 to 29 and from Southgate Road 26 in to the site. All Stormwater Management Ponds are to be constructed as part of Phase 1. Therefore both quality and quantity treatment will be fully provided at the conclusion of Phase 1 development.

## 4. QUANTITY CONTROL CRITERIA, PARAMETERS AND MODELLING

### 4.1 Design Rainfall Events

Rainfall data collected by Environment Canada for the years 1962-2016, 54 years total, was used to prepare Intensity-Duration-Frequency (IDF) statistical data for Mount Forest and the surrounding area. The IDF data was used to determine the total rainfall depth for various storm events. The rainfall intensity was calculated using MIDUSS and the SCS 6 hour Type II curve, as requested by the Saugeen Valley Conservation Authority (SVCA). The maximum rainfall intensity was identified by MIDUSS and is recorded below.

**Table 1: Design Rainfall Events – Mount Forest IDF 6-hour SCS Type-II Parameters**

COEFFICIENT	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
Duration (min)	360	360	360	360	360	360
Total Depth (mm)	38.8	49.4	56.4	65.3	71.9	78.4
Max. Intensity (mm/hr)	61.9	78.9	90.0	104.3	114.8	125.2

The Hurricane Hazel Regional Storm is the Regional design storm event for lands within the jurisdiction of the Saugeen Valley Conservation Authority. As per Ontario Regulation 169/06, the rainfall depths for the Hurricane Hazel Regional Storm are summarized in Table 2 below.

**Table 2: Regional Design Storm Event – Hurricane Hazel Regional Storm**

Hour	First 36 hours		37	38	39	40	41
Rainfall Depth (mm)	73		6	4	6	13	17
Hour	42	43	44	45	46	47	48
Rainfall Depth (mm)	13	23	13	13	53	38	13

## 4.2 Site Soil Conditions

The soil type within the developed subject property is generally characterized as Pike Lake Loam, as per the Grey County Soils Map (Ontario Soil Survey Report No. 17) published by the Department of Agriculture. Pike Lake Loam is known to be of the Hydrological Soil Group 'B'.

With consideration of the developed portion of the site's existing and post-development pervious ground cover, which could be defined as "Crop and Other Improved Lands", a Group B soil is represented with an SCS Curve Number of 74, as per the MTO Drainage Manual's Design Chart 1.09.

The impervious areas within the subject property are associated with an SCS Curve Number of 98.

A recently conducted Geotechnical Investigation prepared by CMT Engineering Inc. determined that the soil between 0.25 m and 7.0 m below the ground surface consisted mainly of sand and gravel with an average hydraulic conductivity of  $3.62 \times 10^{-3}$  cm/s. The Geotechnical Investigation (Dec 2018) prepared by CMT Engineering Inc. is included as Appendix G To be conservative when calculating the expected drawdown time of the infiltration ponds, a hydraulic conductivity of  $1.0 \times 10^{-3}$  cm/s will be used.

## 4.3 Existing Catchment Areas

For existing analysis purposes, the subdivision area is modelled as eight (8) drainage catchments, described in Table 2 below and as shown on Figure 2 – Existing Drainage Conditions. The existing conditions MIDUSS computer modelling is attached as Appendix A.

**Table 2: Existing Conditions Catchments**

Catchment	Outlet	Description	Area (ha)	Impervious Level (%)
10	Outlet 1 – Wilder Lake	Overland Flow – grass and trees, includes rental cottages	5.26	3
20	Outlet 2 – Camp Creek Tributary	Overland Flow – grass and trees, includes roadway	5.50	6
30	Outlet 3 – Infiltration On-Site	Infiltration Area – East Section	3.06	0
40	Outlet 4 – South Off-Site	Overland Flow Off-Site, South East Section – grassed, some trees	1.07	0
41	Outlet 4 – South Off-Site	Overland Flow Off-Site – South West Section – grassed area	3.18	0
42	Outlet 4 – South Off-Site	Overland Flow Off-Site – West Section – grassed, some trees	0.69	0
50	Outlet 3 – Infiltration On-Site	West Section – grassed area, includes Clubhouse and parking lot	2.01	7
60	Outlet 2/3 – Infiltration On-Site / Spill to Camp Creek Tributary	Overland Flow to Wet Pond on Golf Course	0.55	0

The results of the existing conditions routing analysis are summarized in Section 4.5.

#### 4.4 Post-Development Catchment Areas

For post-development analysis purposes, the subdivision area is modelled as nine (9) drainage catchments described in Table 3 below and as shown on Figure 3 – Post-Development Drainage Conditions. The post-development conditions MIDUSS computer modelling is attached as Appendix B.

**Table 3: Post-Development Conditions Catchments**

Catchment	Outlet	Description	Area (ha)	Impervious Level (%)
100	Outlet 1 – Wilder Lake	Overland flow directly to Wilder Lake	4.25	10
101	Outlet 1 – Wilder Lake	South developed portion of the subdivision, directed to Block 30 SWM Facility	4.89	30
102	Outlet 1 – Wilder Lake	Northeast developed portion of the subdivision, directed to Block 31 SWM Facility	0.90	30
200	Outlet 2 – Camp Creek Tributary	Uncontrolled overland flow to Camp Creek Tributary, including one lot	4.21	11
201	Outlet 2 – Camp Creek Tributary	Northwest developed portion of the subdivision, directed to SWM Facility 600	2.15	20
400	Outlet 4 – South Off-Site	Uncontrolled overland flow off-site – Southeast portion	0.99	8
401	Outlet 4 – South Off-Site	Uncontrolled overland flow off-site – Southwest portion	1.69	8
500	Outlet 2 – Camp Creek Tributary	West developed portion of the subdivision, directed to SWM Facility 600	1.69	25
600	Outlet 2/3 – Camp Creek Tributary /Infiltrate on site	Overland flow to SWM Facility 600.	0.55	0

The results of the post-development conditions routing analysis are summarized in Section 4.5.

#### 4.5 MIDUSS Quantity Control Modelling Results

MIDUSS modelling software was used to model the expected existing conditions and post-development conditions stormwater runoff from the subject property under the various design storms. Results from the models are summarized in the following Tables, and the modelling is provided for reference in Appendix A and B. The stage-storage-discharge calculations for the SWM facilities under existing and proposed conditions are included as Appendix C.

Table 4 below provides the total peak flow rates discharging from the subject property under existing and post-development conditions from the modelled catchments.

**Table 4: Summary of Peak Runoff Flow Results**

Catchments		Return Storm Frequency (yr)						Regional
		2	5	10	25	50	100	
<b>Total to Wilder Lake (m<sup>3</sup>/s)</b>	Existing Condition	0.068	0.139	0.196	0.278	0.349	0.429	0.737
	<b>Post-development</b>	<b>0.065</b>	<b>0.125</b>	<b>0.178</b>	<b>0.260</b>	<b>0.339</b>	<b>0.411</b>	<b>1.380</b>
<b>Total to Camp Creek Tributary (m<sup>3</sup>/s)</b>	Existing Condition	0.081	0.163	0.220	0.321	0.402	0.487	0.754
	<b>Post-development</b>	<b>0.081</b>	<b>0.152</b>	<b>0.197</b>	<b>0.275</b>	<b>0.339</b>	<b>0.405</b>	<b>1.148</b>
<b>Total Infiltrating on-site (m<sup>3</sup>/s)</b>	Existing Condition	0.078	0.154	0.212	0.312	0.395	0.482	0.774
	<b>Post-development</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>	<b>0.000</b>
<b>Total Off-site to South (m<sup>3</sup>/s)</b>	Existing Condition	0.047	0.097	0.144	0.204	0.261	0.325	0.677
	<b>Post-development</b>	<b>0.035</b>	<b>0.069</b>	<b>0.097</b>	<b>0.136</b>	<b>0.170</b>	<b>0.209</b>	<b>0.372</b>
<b>Total From Site (m<sup>3</sup>/s)</b>	Existing Condition	0.274	0.553	0.772	1.115	1.407	1.723	2.942
	<b>Post-development</b>	<b>0.181</b>	<b>0.346</b>	<b>0.472</b>	<b>0.671</b>	<b>0.848</b>	<b>1.025</b>	<b>2.900</b>

Under existing conditions, a large portion of the site fully infiltrates in low-lying areas on the golf course. Under post-development conditions, no undrained low-lying areas will exist on site, so all runoff is directed to either the Wilder Lake outlet or the Camp Creek Tributary outlet, while maintaining pre-development peak flow rates. However, the proposed SWM Ponds are designed to provide infiltration for all or most of the 2 year design storm events.

As shown in Table 4 above, under post-development conditions, the peak flow rates to be directed each of the four outlets as well as the total from the site, are expected to be less than, or equal to, existing conditions for all design storm events up to, and including, the 100 year return period.

Table 5 below summarizes the capacity available at the various stages in the Block 31 SWM Facility and provides a comparison to the capacity that is expected to be used during the various design storm events.

**Table 5: Conceptual Stage-Storage-Discharge Capacities for Block 31 SWM Facility (Catchment 101)**

	Available Capacity in SWM Facility Design				Capacity Used During Various Design Storm Events			
	Peak Flow (m <sup>3</sup> /s)	Available Storage Volume		Storage Elevation (m)	Peak Flow (m <sup>3</sup> /s)	Infiltration Storage Volume (m <sup>3</sup> )	Active Storage Volume (m <sup>3</sup> )	Storage Elevation (m)
		Infiltration (m <sup>3</sup> )	Active (m <sup>3</sup> )					
Inv. 125 mm Ø Orifice	0.00	0	0	423.25	---	---	---	---
Bottom of Pond	0.00	0	0	423.00	---	---	---	---
T/G Outlet DICB	0.00	625	0	424.50	---	---	---	---
2 year Design Storm	---	---	---	---	0.008	625	49	424.56
5 year Design Storm	---	---	---	---	0.028	625	171	424.69
10 year Design Storm	---	---	---	---	0.036	625	290	424.81
25 year Design Storm	---	---	---	---	0.040	625	517	425.03
50 year Design Storm	---	---	---	---	0.042	625	707	425.20
100 year Design Storm	---	---	---	---	0.044	625	906	425.35
Overflow Weir	0.048	625	1106	425.50	---	---	---	---
Regional Storm	---	---	---	---	0.686	625	1333	425.65
Top of Bank	1.117	625	1483	425.75	---	---	---	---

As shown in Table 5 above, the first 625 m<sup>3</sup> of runoff will drain via infiltration. During all design storm events up to, and including, the 100 year return period, runoff draining to the proposed SWM facility is expected to discharge from the proposed SWM facility solely via infiltration or the 125 mm Ø orifice with a 300 mm Ø outlet pipe from the outlet DICB. For design storm events greater than the 100 year return period, runoff draining to the SWM facility is expected to discharge via a combination of infiltration, the 125 mm Ø orifice and the overflow weir, with all runoff that is not infiltrated draining via the treed area to Wilder Lake.

The storage volume for infiltration of the Block 31 SWM Pond is 625 m<sup>3</sup>. As noted earlier, the conservative hydraulic conductivity of the on-site soil is 1x10<sup>-3</sup> cm/s. As such, the 625 m<sup>3</sup> stormwater volume would be expected to infiltrate in 21.4 hours when the active pond area (at the top of grate – low side of the DICB) of 812 m<sup>2</sup> is considered to allow for infiltration.

Given the density of the sand and gravel material in the area of the Block 31 SWM Pond, compaction is not considered to be a concern for the facility to infiltrate the minor storm events.

Table 6 below summarizes the capacity available at the various stages in the Block 30 SWM Facility and provides a comparison to the capacity that is expected to be used during the various design storm events.



**Table 6: Conceptual Stage-Storage-Discharge Capacities for Block 30 SWM Facility (Catchment 102)**

	Available Capacity in SWM Facility Design				Capacity Used During Various Design Storm Events			
	Peak Flow (m <sup>3</sup> /s)	Available Storage Volume		Storage Elevation (m)	Peak Flow (m <sup>3</sup> /s)	Infiltration Storage Volume (m <sup>3</sup> )	Active Storage Volume (m <sup>3</sup> )	Storage Elevation (m)
		Infiltration (m <sup>3</sup> )	Active (m <sup>3</sup> )					
Inv. 250 mm Ø Outlet Pipe	0.00	0	0	423.95	---	---	---	---
Bottom of Pond	0.00	0	0	423.35	---	---	---	---
2 year Design Storm	---	---	---	---	0.000	134	0	424.21
T/G Outlet DICB	0.00	146	0	424.25	---	---	---	---
5 year Design Storm	---	---	---	---	0.004	146	15	424.30
10 year Design Storm	---	---	---	---	0.008	146	27	424.34
25 year Design Storm	---	---	---	---	0.013	146	49	424.40
50 year Design Storm	---	---	---	---	0.014	146	74	424.47
100 year Design Storm	---	---	---	---	0.015	146	101	424.55
Overflow Weir	0.017	146	312	424.70	---	---	---	---
Regional Storm	---	---	---	---	0.124	146	182	424.73
Top of Bank	0.514	146	384	424.85	---	---	---	---

As shown in Table 6 above, during a 2 year design storm event, all runoff from the SWM Facility will drain via infiltration. During design storm events greater than a 2 year design storm, up to, and including, the 100 year return period, runoff draining to the proposed SWM facility is expected to discharge from the proposed SWM facility solely via infiltration and the 100 mm Ø orifice with a 250 mm Ø outlet pipe from the outlet DICB. For design storm events greater than the 100 year return period, runoff draining to the SWM facility is expected to discharge via a combination of infiltration, the 100 mm Ø orifice and the overflow weir, with all runoff that is not infiltrated draining via the low-lying wet area to Wilder Lake.

The storage volume for infiltration of the Block 30 SWM Pond is 146 m<sup>3</sup>. As noted earlier, the conservative hydraulic conductivity of the on-site soil is 1x10<sup>-3</sup> cm/s. As such, the 146 m<sup>3</sup> stormwater volume would be expected to infiltrate in 13.9 hours when the active pond area (at the top of grate – low side of the DICB) of 292 m<sup>2</sup> is considered to allow for infiltration.

Given the density of the sand and gravel material in the area of the Block 31 SWM Pond, compaction is not considered to be a concern for the facility to infiltrate the minor storm events.

Table 7 below summarizes the capacity available at the various stages in SWM Facility 600 and provides a comparison to the capacity that is expected to be used during the various design storm events. SWM Facility 600 is an existing pond on site and the inlets and outlets are designed to allow it to act as a wet pond for quantity control only. Pond configuration and side slopes are to remain as existing.

**Table 7: Conceptual Stage-Storage-Discharge Capacities for Block 32 SWM Facility (Catchment 600)**

	Available Capacity in SWM Facility Design				Capacity Used During Various Design Storm Events			
	Peak Flow (m <sup>3</sup> /s)	Available Storage Volume		Storage Elevation (m)	Peak Flow (m <sup>3</sup> /s)	Permanent Pool Volume (m <sup>3</sup> )	Active Storage Volume (m <sup>3</sup> )	Storage Elevation (m)
		Permanent Pool (m <sup>3</sup> )	Active Storage (m <sup>3</sup> )					
Bottom of Pond	0.000	0	0	422.71	---	---	---	---
Inv. 200 mm Ø Outlet Pipe	0.000	703	0	423.30	---	---	---	---
2 year Design Storm	---	---	---	---	0.023	703	307	423.48
5 year Design Storm	---	---	---	---	0.033	703	465	423.56
10 year Design Storm	---	---	---	---	0.039	703	586	423.62
25 year Design Storm	---	---	---	---	0.046	703	759	423.71
50 year Design Storm	---	---	---	---	0.051	703	898	423.77
100 year Design Storm	---	---	---	---	0.055	703	1043	423.84
Overflow Weir	0.059	703	1170	423.90	---	---	---	---
Regional Storm	---	---	---	---	0.584	703	1518	424.05
Top of Bank	0.832	703	1636	424.10	---	---	---	---

As shown in Table 7 above, during all design storm events up to, and including, the 100 year return period, runoff draining to the proposed SWM facility is expected to discharge from the proposed SWM facility solely via the 200 mm Ø outlet pipe from the outlet DICB, with all runoff draining via the existing pond to Camp Creek Tributary.

#### 4.6 Major Flow Routes

Overland flow during a major storm such as a 100 year or design storm event can be conveyed via the ditches to the SWM facilities. The driveway culverts are designed to convey the 10 year storm. The driveways are designed to have low points above the culverts to allow storms up to the 100 year design storm event to be conveyed over the driveways and in the ditches without ponding on private property. The storm sewers have been designed to convey the 100 year storm event. A summary of the conveyance calculations and the storm sewer design sheet is included as Appendix F.

The road profiles have been designed to ensure that during a major storm event, stormwater will spill toward the appropriate outlet.

### 5. STORMWATER QUALITY TREATMENT

Stormwater quality treatment is proposed to be provided in a variety of ways, including Enhanced Grassed Swales, infiltration facilities, or directing runoff over grassed surfaces.

It is also worth noting that this a rural subdivision with low imperviousness. The highest imperviousness in any catchment is 30%, and much of that is rooftop area, which yields clean runoff.



Runoff from the developed portions of the subdivision are directed to grass surfaces where possible to allow sediment to settle out as runoff is conveyed overland to the proposed grassed ditches and ultimately to the SWM ponds.

Stormwater quality treatment for runoff from Catchments 101 and 102 is provided by the proposed SWM facilities generally designed as infiltration basins. The Ministry of Environment and Parks (MOEP) guidelines recommend that, for infiltration ponds to provide 80% TSS removal, the infiltration pond should provide 24 m<sup>3</sup>/ha of storage for catchments with 30% imperviousness. The required infiltration storage volumes to provide 80% TSS removal for ponds 101 and 102, as well as provided storage volumes, are listed in Table 8.

**Table 8: Summary Table of Infiltration Pond Water Quality Design**

Catchment	Area (ha)	Imp. Level	Required Storage Volume Ratio	Required Storage Volume	Provided Storage Volume
101	4.89	30%	24 m <sup>3</sup> /ha	117 m <sup>3</sup>	625 m <sup>3</sup>
102	0.90	30%	24 m <sup>3</sup> /ha	22 m <sup>3</sup>	146 m <sup>3</sup>

Overall, given the stormwater quality treatment provided by directing runoff along the grassed surfaces where possible, and the further polishing provided by the infiltration ponds' infiltration volume, it is expected that the proposed SWM system will provide an Enhanced water quality treatment level (80% TSS removal) prior to discharging from the SWM facilities to Wilder Lake.

Enhanced Grass Swales (EGS) are proposed to provide stormwater quality treatment in catchments 201 and 500, for storm water draining to the Block 32 SWM Pond. They are designed as per the Low Impact Development Stormwater Management Planning and Design Guide (LIDSWMP Design Guide) published by the Credit Valley Conservation Authority (CVCA) and the Toronto and Regional Conservation Authority (TRCA). Table 15 below compares the flow characteristics through the Enhanced Grass Swales during a 4 hour, 25 mm Chicago storm event in comparison with the requirements set by the LIDSWMP Design Guide; MIDUSS modelling for the results are attached as Appendix D.

**Table 9: Summary Table of Enhanced Grassed Swales Water Quality Design**

Characteristics		During a 4 hour, 25mm Chicago Storm Event			
Sub-Catchment	Bottom Width	Maximum Designed Flow Velocity	Required Maximum Flow Velocity	Maximum Designed Flow Depth	Required Maximum Flow Depth
201 – North Side	0.75 m	0.437 m/s	0.50 m/s	0.075 m	0.10 m
201 – South Side	0.75 m	0.453 m/s	0.50 m/s	0.080 m	0.10 m
500 – West Side	1.25 m	0.484 m/s	0.50 m/s	0.028 m	0.10 m
500 – East Side	1.50 m	0.482 m/s	0.50 m/s	0.028 m	0.10 m
Parking Lot	0.75 m	0.409 m/s	0.50 m/s	0.067 m	0.10 m

As shown in Table 9, the maximum design depth of flow through the EGS facilities during a 4 hour, 25 mm Chicago storm event is less than the maximum permitted depth. In addition, the design flow velocities through the SWM facilities would be less than the maximum permitted velocity.

According to the CVCA and TRCA's LIDSWMP Design Guide, an Enhanced Grass Swale provides a median TSS removal rate of 76%.

From the analysis of five soil samples taken from the near-surface as part of the geotechnical investigation completed by CMT Engineering Inc, the infiltration rate was determined to be in the range of 40 mm/h to 75 mm/hr. the CVCA and TRCA's LIDSWMP Design Guide assumes an infiltration rate around 15 mm/hr. Therefore, it is expected that the higher infiltration rate in the EGS will provide above the median TSS removal rate.

The flow from the EGS is conveyed to the wet pond by catchbasins and storm sewers. The 600 mm sump in the catchbasins will provide some additional TSS removal.

Overall, given the low imperviousness of the development, higher infiltration rate of the native soils and the proposed EGS design, it is expected that the proposed SWM system will provide an Enhanced water quality treatment level (80% TSS Removal) to runoff draining from the proposed development, prior to discharging to the wet pond.

Catchments 100, 200, 400, 401 and 600 are areas consisting of grassed or rooftop areas, which are generally considered clean runoff. As such, stormwater quality treatment does not need to be provided for these catchments.

## 6. OPERATIONS AND MAINTENANCE PLAN

To ensure that the stormwater management system continues to function as designed and constructed, we recommend that the following inspections and maintenance activities be completed by the owner on an annual basis. A checklist of the items noted below is provided as Appendix E for use during inspections and to keep record of the maintenance that has been completed.

1. Is there any noticeable damage to the facility structures (i.e. catchbasins, overflow weirs)? If yes, complete the necessary repairs and/or installation of replacement structures.
2. Inspect on-site catchbasins. Remove and dispose of accumulated sediment, trash/litter, debris (i.e. sediment, garbage, leaves, etc.) as needed.
3. Is there any indication of a spill (i.e. Frothy water, oily sheen on sump water)? If yes, investigate, inform the appropriate agencies, and complete the necessary clean-up, restoration and disposal.
4. Inspect the water level in the SWM pond. Has the pond completely drained within 48 hours after a storm? If not, inspect the downstream structures and remove and dispose of any accumulated sediment, trash/litter and debris.

Please note that any components of the structures identified during the annual inspection to be missing or damaged should be repaired or replaced in a timely manner.

## 7. SUMMARY

H. Bye Construction Limited proposes to construct a new 20.8 ha residential subdivision at their property located at 263512 Southgate Road 26 in the Township of Southgate.

Upon completion of the proposed development;

1. An approximately 20.8 ha portion of the Wilder Lake Resort and Golf Course will be developed into a subdivision with 29 lots. The subdivision will have two infiltration ponds and one wet pond with five enhanced grass swales to provide stormwater management in the form of quality and quantity controls.
2. Full stormwater quantity and quality treatment will be provided at the completion of Phase 1 of the development.
3. Total post-development peak flow rates discharging from the subject property to each of the drainage outlets, as well as the property as a whole, are expected to be attenuated to less than, or equal to, the total existing conditions peak flow rates.
4. An "Enhanced" level of water quality treatment (80% TSS Removal) is expected to be provided for runoff draining from the developed portion of the subdivision via the SWM facilities prior to discharging from the site.

All of which is respectfully submitted,

GM BLUEPLAN ENGINEERING LIMITED

Prepared By:



Jennifer L Swiger, E.I.T.

Reviewed By:



Ian E. Eriksen, P.Eng.



**FIGURES:**

218173  
Wilder Lake Subdivision  
263512 Southgate Road 26  
Township of Southgate



NOT TO SCALE  
NOVEMBER 2019

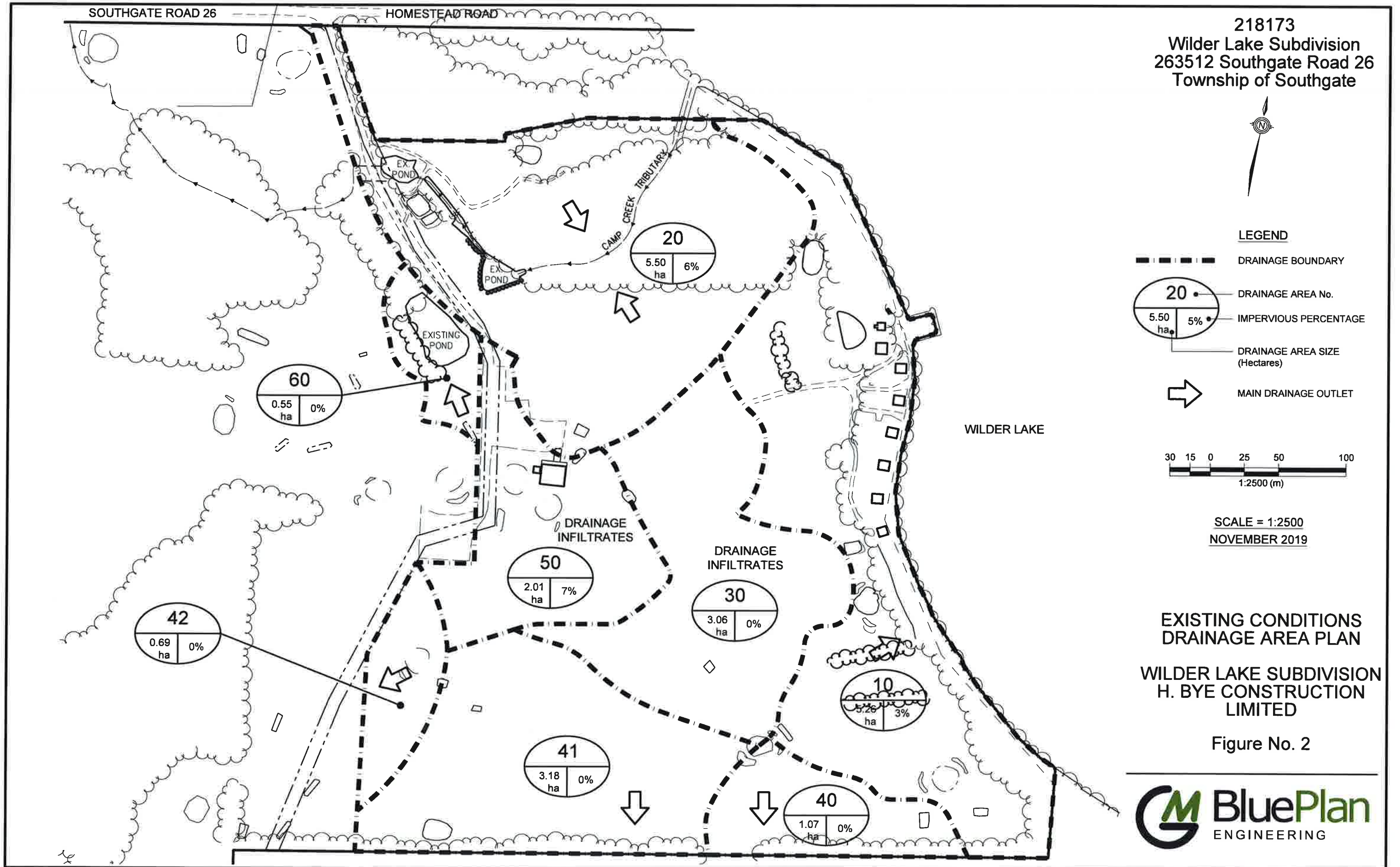
## SITE LOCATION PLAN

WILDER LAKE SUBDIVISION  
H. BYE CONSTRUCTION  
LIMITED

Figure No. 1

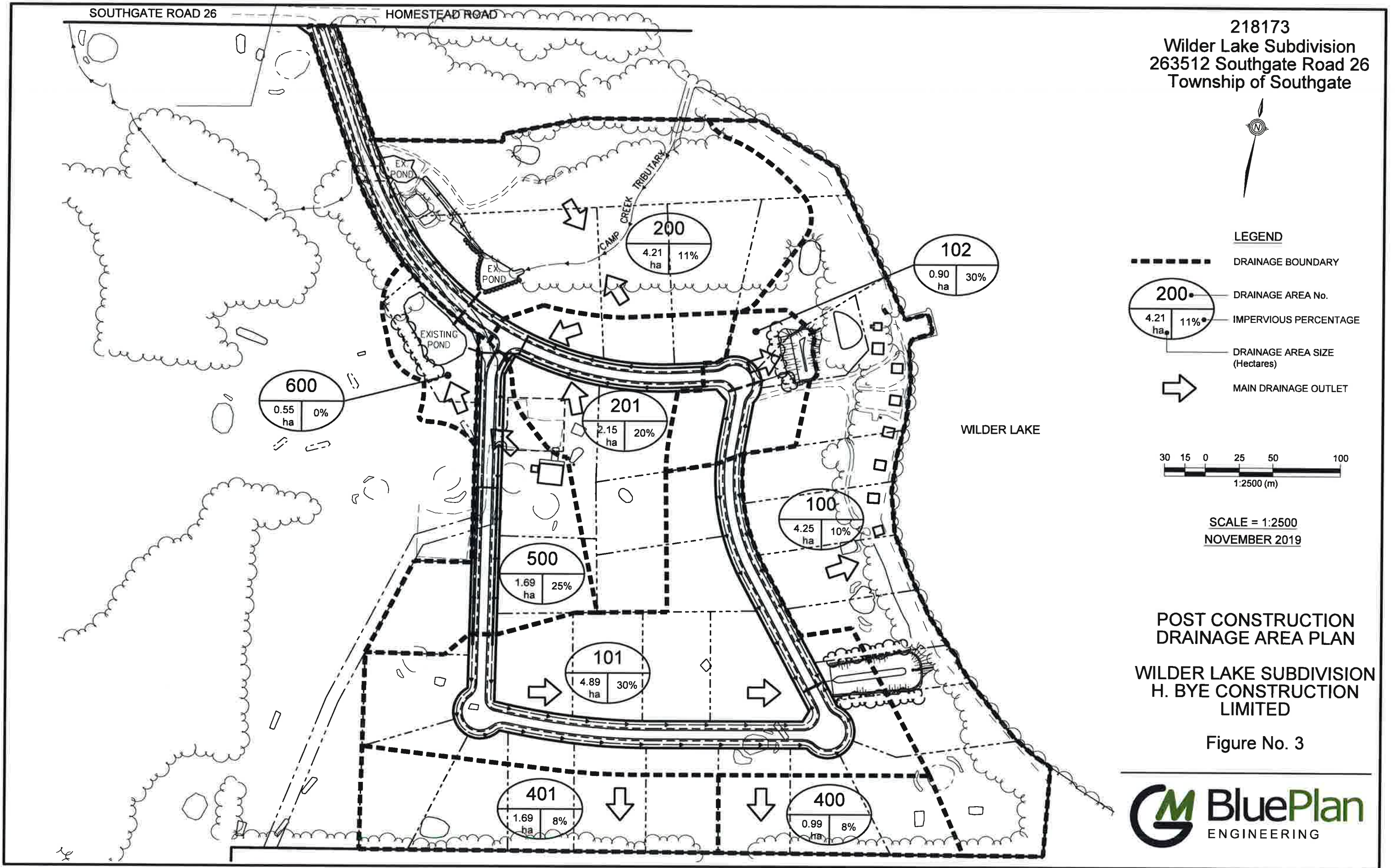


FILE:C:\Civil 3D Projects\218173 GP-K New Concept.dwg LAYOUT:Ex Drainage Areas (11x17)  
LAST SAVED BY:Kboers, 10/28/2019 4:14:48 PM PLOTTED BY:Ken Boers - GM BluePlan 11/14/2019 3:51:20 PM





FILE:C:\Civil 3D Projects\218173 GP-K New Concept.dwg LAYOUT:Drainage Areas (11x17)  
LAST SAVED BY:Kheers, 10/28/2019 4:14:48 PM PLOTTED BY:Ken Boers - GM BluePlan 11/14/2019 3:51:22 PM





**APPENDIX A:**  
**MIDUSS MODELLING – EXISTING CONDITIONS**

218173-PreDev-Oct01-2yr-SCS.out  
MIDUSS Output ----->  
MIDUSS version Version 2.25 rev. 473  
MIDUSS created Sunday, February 07, 2010  
Units used: C:\Users\jswiger\Desktop\Wilden Lake\MIDUSS  
Job folder: C:\Users\jswiger\Desktop\Wilden Lake\MIDUSS  
Output filename: 218173-PreDev-Oct01-2yr-SCS.out  
Licensee name: Hewlett-Packard Company  
Date & Time last used: 10/1/2019 at 11:37:43 AM  
TIME PARAMETERS  
31 10.000 Time Step  
360.000 Max. Storm length  
2400.000 Max. Hydrograph  
32 38.800 Mass Curve  
360.000 Duration  
47 C:\Program Files (x86)\MIDUSS\SCS\_6hr\_Type2.mrd SCS 6 hour Type  
Maximum intensity 61.945 mm/hr  
Total depth 38.800 mm  
6 002hyd Hydrograph extension used in this file  
CATCHMENT 10  
1 Triangular SCS  
1 Proportional to %  
10- Wilder Lake South  
10- Wilder Lake South  
3.000 Total Area  
5.260 Flow length  
5.000 Overland Slope  
5.102 Pervious Area  
100.000 Pervious length  
5.000 Pervious slope  
0.158 Impervious Area  
3.093 Impervious length  
5.000 Impervious slope  
0.250 Pervious Manning 'n'  
74.000 Pervious SCS Curve No.  
0.193 Pervious Runoff coefficient  
0.100 Pervious Ia/S coefficient  
8.924 Pervious Initial abstraction  
0.015 Pervious Manning 'n'  
98.000 Impervious SCS Curve No.  
0.758 Impervious Runoff coefficient  
0.100 Impervious Ia/S coefficient  
0.518 Impervious Initial abstraction  
0.068 0.000 0.000 0.000 c.m/sec  
Catchment 10 Pervious Total Area  
Surface Area 5.102 0.158 5.260 hectare  
Time of concentration 45.566 0.534 40.688 minutes  
Rainfall depth 278.435 195.256 269.424 mm  
Rainfall volume 38.800 38.800 38.800 c.m  
Rainfall losses 1979.65 61.23 2040.88 mm  
Runoff depth 31.317 9.405 30.660 mm  
Runoff volume 7.483 29.395 8.140 c.m  
Runoff coefficient 381.79 46.39 428.17 c.m/sec  
Maximum flow 0.193 0.758 0.210  
HYDROGRAPH Add Runoff 0.067 0.022 0.068  
4 Add Runoff 0.068 0.068 0.000 0.000  
Page 1

218173-PreDev-Oct01-2yr-SCS.out  
HYDROGRAPH COPY TO OutFlow  
8 Copy to OutFlow 0.068 0.068 0.000  
HYDROGRAPH " Combine 1  
6 Combine  
1 Node # Wilder Lake Final  
Maximum flow 0.068 c.m/sec  
Hydrograph volume 428.173 c.m  
0.068 0.068  
HYDROGRAPH Start - New Tributary  
2 Start - New Tributary 0.068 0.068 0.068  
0.068 0.000  
CATCHMENT 20  
1 Triangular SCS  
2 Proportional to %  
2 SCS method  
20 - Creek  
6.000 % Impervious  
5.500 Total Area  
75.000 Flow length  
5.000 Overland Slope  
5.170 Pervious Area  
75.000 Pervious length  
5.000 Pervious slope  
0.330 Impervious Area  
4.787 Impervious length  
5.000 Impervious slope  
0.250 Pervious Manning 'n'  
74.000 Pervious SCS Curve No.  
0.192 Pervious Runoff coefficient  
0.100 Pervious Ia/S coefficient  
8.924 Pervious Initial abstraction  
0.015 Pervious Manning 'n'  
98.000 Impervious SCS Curve No.  
0.765 Impervious Runoff coefficient  
0.100 Impervious Ia/S coefficient  
0.518 Impervious Initial abstraction  
0.081 0.000 0.068 0.068 c.m/sec  
Catchment 20 Pervious Total Area  
Surface Area 5.170 0.330 5.500 hectare  
Time of concentration 38.342 0.694 30.723 minutes  
Rainfall depth 289.564 196.135 254.702 mm  
Rainfall volume 38.800 38.800 38.800 c.m  
Rainfall losses 2005.96 128.04 2134.00 mm  
Runoff depth 31.332 9.111 29.999 mm  
Runoff volume 7.468 29.689 8.801 c.m  
Runoff coefficient 386.10 97.97 484.08 c.m/sec  
Maximum flow 0.192 0.765 0.227  
HYDROGRAPH Add Runoff 0.076 0.046 0.081  
4 Add Runoff 0.081 0.081 0.068 0.068  
HYDROGRAPH COPY TO OutFlow  
8 Copy to OutFlow 0.081 0.081 0.068  
HYDROGRAPH " Combine 2  
6 Combine  
2 Node # Total to Creek  
Maximum flow 0.081 c.m/sec  
Hydrograph volume 484.078 c.m  
0.081 0.081 0.081  
Page 2

40	HYDROGRAPH Start - New Tributary"	0.081	0.000	0.081	0.081"
40	HYDROGRAPH Start - New Tributary"	0.081	0.000	0.081	0.081"
33	CATCHMENT 30"	0.081	0.000	0.081	0.081"
1	Triangular SCS"				
2	Proportional to %"				
30	30 - Infiltration"				
0.000	% Impervious"				
3.060	Total Area"				
100.000	Flow length"				
3.060	Overland Slope"				
4.000	Pervious Area"				
100.000	Pervious length"				
4.000	Impervious Area"				
0.000	Impervious slope"				
0.000	Impervious length"				
0.000	Impervious Manning "n"				
0.250	Pervious Manning "n"				
74.000	Pervious SCS Curve No."				
0.193	Pervious Runoff coefficient"				
0.100	Pervious Ia/S coefficient"				
8.924	Impervious Manning "n"				
0.015	Impervious SCS Curve No."				
98.000	Impervious Runoff coefficient"				
0.000	Impervious Ia/S coefficient"				
0.100	Impervious Initial abstraction"				
0.518	Impervious Initial abstraction"				
0.039	0.039	0.000	0.081	0.081	0.081 c.m/sec"
Catchment 30	Pervious	0.039	0.000	0.081	0.081
Surface Area	3.060	0.000	0.000	0.000	0.000
Time to Concentration	48.721	0.001	194.149	282.315	282.315
Time to Centroid	282.315	0.001	194.149	282.315	282.315
Rainfall depth	38.800	0.000	38.800	38.800	38.800
Rainfall volume	1187.28	0.000	1187.28	1187.28	1187.28
Rainfall losses	31.310	0.000	31.310	31.310	31.310
Runoff depth	7.490	0.000	7.490	7.490	7.490
Runoff volume	229.19	0.000	229.19	229.19	229.19
Runoff coefficient	0.193	0.000	0.193	0.193	0.193
Maximum flow	0.039	0.000	0.039	0.039	0.039
HYDROGRAPH Add Runoff "	0.039	0.000	0.039	0.039	0.039
4	Add Runoff "	0.039	0.000	0.039	0.039
HYDROGRAPH Copy to Outflow"	0.039	0.000	0.039	0.039	0.039
8	Copy to Outflow"	0.039	0.000	0.039	0.039
HYDROGRAPH " Combine	0.039	0.000	0.039	0.039	0.039
6	Combine "	0.039	0.000	0.039	0.039
3	Node #"	0.039	0.000	0.039	0.039
Total to Infiltration"	0.039	0.000	0.039	0.039	0.039
Maximum flow	0.039	0.000	0.039	0.039	0.039
Hydrograph volume	229.189	0.039	229.189	229.189	229.189
HYDROGRAPH Start - New Tributary"	0.039	0.000	0.039	0.039	0.039
2	Start - New Tributary"	0.039	0.000	0.039	0.039
CATCHMENT 40"	0.039	0.000	0.039	0.039	0.039
1	Triangular SCS"				
2	Proportional to %"				
1	SCS method"				

40	40 - South East Flow Off-Site"	0.000	0.000	0.000	0.000
1.070	% Impervious	0.000	0.000	0.000	0.000
60.000	Total Area"	0.000	0.000	0.000	0.000
2.000	Flow length"	0.000	0.000	0.000	0.000
1.070	Overland Slope"	0.000	0.000	0.000	0.000
60.000	Pervious Area"	0.000	0.000	0.000	0.000
2.000	Pervious length"	0.000	0.000	0.000	0.000
0.000	Impervious Area"	0.000	0.000	0.000	0.000
0.000	Impervious slope"	0.000	0.000	0.000	0.000
0.000	Impervious length"	0.000	0.000	0.000	0.000
0.250	Impervious Manning "n"	0.000	0.000	0.000	0.000
74.000	Pervious SCS Curve No."	0.000	0.000	0.000	0.000
0.193	Pervious Runoff coefficient"	0.000	0.000	0.000	0.000
0.100	Pervious Ia/S coefficient"	0.000	0.000	0.000	0.000
8.924	Impervious Manning "n"	0.000	0.000	0.000	0.000
0.015	Impervious SCS Curve No."	0.000	0.000	0.000	0.000
98.000	Impervious Runoff coefficient"	0.000	0.000	0.000	0.000
0.000	Impervious Ia/S coefficient"	0.000	0.000	0.000	0.000
0.100	Impervious Initial abstraction"	0.000	0.000	0.000	0.000
0.518	Impervious Initial abstraction"	0.000	0.000	0.000	0.000
0.014	0.014	0.000	0.039	0.039	0.039 c.m/sec"
Catchment 40	Pervious	0.014	0.000	0.039	0.039
Surface Area	1.070	0.000	0.000	0.000	0.000
Time to Concentration	44.148	0.001	194.149	276.687	276.687
Time to Centroid	276.688	0.001	194.149	276.687	276.687
Rainfall depth	38.800	0.000	38.800	38.800	38.800
Rainfall volume	415.16	0.000	415.16	415.16	415.16
Rainfall losses	31.324	0.000	31.324	31.324	31.324
Runoff depth	7.476	0.000	7.476	7.476	7.476
Runoff volume	79.99	0.000	79.99	79.99	79.99
Runoff coefficient	0.193	0.000	0.193	0.193	0.193
Maximum flow	0.014	0.000	0.014	0.014	0.014
HYDROGRAPH Add Runoff "	0.014	0.000	0.014	0.014	0.014
4	Add Runoff "	0.014	0.000	0.014	0.014
HYDROGRAPH Copy to Outflow"	0.014	0.000	0.014	0.014	0.014
8	Copy to Outflow"	0.014	0.000	0.014	0.014
HYDROGRAPH " Combine	0.014	0.000	0.014	0.014	0.014
6	Combine "	0.014	0.000	0.014	0.014
4	Node #"	0.014	0.000	0.014	0.014
Total to Off-Site"	0.014	0.000	0.014	0.014	0.014
Maximum flow	0.014	0.000	0.014	0.014	0.014
Hydrograph volume	79.994	0.014	79.994	79.994	79.994
HYDROGRAPH Start - New Tributary"	0.014	0.000	0.014	0.014	0.014
2	Start - New Tributary"	0.014	0.000	0.014	0.014
CATCHMENT 41"	0.014	0.000	0.014	0.014	0.014
1	Triangular SCS"				
2	Proportional to %"				
1	SCS method"				
41	41 -South West Flow Off Site"				
0.000	% Impervious				
3.180	Total Area"				
185.000	Flow length"				
4.000	Overland Slope"				
3.180	Pervious Area"				
185.000	Pervious length"				
4.000	Impervious Area"				
0.000	Impervious slope"				
0.000	Impervious length"				

218173-PreDev-Oct01-2yr-SCS.out									
4.000	Impervious Slope"								
0.250	Pervious Manning 'n'"								
74.000	Pervious SCS Curve No."								
0.193	Pervious Runoff coefficient"								
0.100	Pervious Ia/S coefficient"								
8.924	Pervious Initial abstraction"								
0.015	Impervious Manning 'n'"								
98.000	Impervious SCS Curve No."								
0.000	Impervious Runoff coefficient"								
0.100	Impervious Ia/S coefficient"								
0.518	Impervious Initial abstraction"								
0.030	0.000	0.014	0.014						
	Impervious	Total Area "							
	Catchment 41	0.000	3.180	0.000					
	Surface Area	70.472	308.949	194.149					
	Time of concentration	38.800	1233.84	8.464					
	Rainfall depth	31.312	7.488	30.336					
	Rainfall volume	238.13	0.00	0.00					
	Rainfall losses	238.13	0.00	0.00					
	Runoff depth	0.00	0.00	0.00					
	Runoff volume	0.00	0.00	0.00					
	Runoff coefficient	0.00	0.00	0.00					
	Maximum flow	0.030	0.030	0.000					
	HYDROGRAPH Add_Runoff "	0.030	0.014	0.014					
4	Add runoff "	0.030	0.030	0.014					
	HYDROGRAPH copy to outflow"								
8	Copy to outflow"	0.030	0.030	0.030					
	HYDROGRAPH " Combine 4"								
6	Combine "								
4	Node #"								
	Total to off-site"								
	Maximum flow	0.030	0.030	0.014					
	Hydrograph volume	318.126	0.030	0.042					
	HYDROGRAPH Start - New Tributary"								
2	Start - New Tributary"	0.030	0.000	0.030					
	CATCHMENT 42"								
1	Triangular SCS"								
2	Proportional to %"								
42	% Impervious"								
20.000	Total Area"								
0.690	Flow length"								
20.000	Overland Slope"								
0.690	Pervious Area"								
20.000	Pervious length"								
4.000	Pervious slope"								
0.000	Impervious Area"								
0.000	Impervious slope"								
4.000	Impervious length"								
0.000	Impervious Manning 'n'"								
0.250	Pervious SCS Curve No."								
0.192	Pervious Runoff coefficient"								
8.924	Pervious Ia/S coefficient"								
0.015	Impervious Initial abstraction"								
98.000	Impervious SCS Curve No."								
0.000	Impervious Runoff coefficient"								
0.100	Impervious Ia/S coefficient"								

218173-PreDev-Oct01-2yr-SCS.out									
0.518	Impervious Initial abstraction"	0.015	0.000	0.030	0.042	c.m/sec"			
	Catchment 42	0.000	0.000	0.000	0.000	0.000			
	Surface Area	18.550	0.000	0.000	0.000	0.000			
	Time of concentration	245.304	194.150	245.304	194.150	245.304			
	Rainfall depth	38.800	38.800	38.800	38.800	38.800			
	Rainfall volume	267.72	0.00	267.72	0.00	267.72			
	Rainfall losses	31.347	0.00	31.347	0.00	31.347			
	Runoff depth	7.453	30.342	7.453	30.342	7.453			
	Runoff volume	51.43	0.00	51.43	0.00	51.43			
	Runoff coefficient	0.192	0.000	0.192	0.000	0.192			
	Maximum flow	0.015	0.015	0.015	0.015	0.015			
	HYDROGRAPH Add_Runoff "	0.015	0.030	0.015	0.030	0.015			
4	Add runoff "	0.015	0.015	0.015	0.030	0.042"			
	HYDROGRAPH Copy to Outflow"								
8	Copy to Outflow"	0.015	0.015	0.015	0.015	0.042"			
	HYDROGRAPH " Combine 4"								
6	Combine "								
4	Node #"								
	Total to off-site"								
	Maximum flow	0.047	0.047	0.047	0.047	0.047			
	Hydrograph volume	369.554	0.015	0.015	0.015	0.047"			
	HYDROGRAPH Start - New Tributary"								
2	Start - New Tributary"	0.015	0.000	0.015	0.015	0.047"			
	CATCHMENT 50"								
1	Triangular SCS"								
2	Proportional to %"								
1	SCS method"								
50	50 - Flow to Golf Course"								
7.000	% Impervious"								
2.010	Total Area"								
45.000	Flow length"								
3.000	Overland Slope"								
1.869	Pervious Area"								
45.000	Pervious length"								
3.000	Pervious slope"								
0.141	Impervious Area"								
3.387	Impervious length"								
3.000	Impervious slope"								
0.250	Pervious Manning 'n'"								
74.000	Pervious SCS Curve No."								
0.193	Pervious Runoff coefficient"								
0.100	Pervious Ia/S coefficient"								
8.924	Pervious Initial abstraction"								
0.015	Impervious Manning 'n'"								
98.000	Impervious SCS Curve No."								
0.764	Impervious Runoff coefficient"								
0.100	Impervious Ia/S coefficient"								
0.518	Impervious Initial abstraction"								
0.034	0.000	0.015	0.015	0.015	0.047	c.m/sec"			
	Impervious	Total Area "							
	Catchment 50	0.000	0.000	0.000	0.000	0.000			
	Surface Area	1.869	0.000	0.000	0.000	0.000			
	Time of concentration	32.895	263.000	32.895	263.000	32.895			
	Rainfall depth	38.800	38.800	38.800	38.800	38.800			
	Rainfall volume	725.29	54.59	725.29	54.59	725.29			
	Rainfall losses	31.328	9.176	31.328	9.176	31.328			
	Runoff depth	7.472	29.640	7.472	29.640	7.472			

Page 6

	0.518	Impervious Initial abstraction"	0.015	0.000	Pervious	0.042	Impervious
40		Catchment 42					0.000
40		Surface Area			18.550		0.000
40		Time of concentration			245.304		194.150
40		Time to Centroid			38.800		38.800
40		Rainfall depth			267.72		0.00
40		Rainfall volume			31.347		8.458
40		Rainfall losses			7.453		30.342
40		Rainfall depth			51.43		0.00
40		Runoff volume			0.192		0.000
40		Runoff coefficient			0.015		0.000
40		Maximum flow					0.000
40		HYDROGRAPH Add "Runoff "					
40		4 Add "Runoff "	0.015	0.035	0.030	0.042"	
40		HYDROGRAPH Copy to "outflow"					
40		8 Copy to "outflow"	0.015	0.015	0.015	0.042"	
40		HYDROGRAPH " Combine 4"					
40		6 Combine					
40		4 Node #					
40		Total to Off-Site"					
40		Maximum flow			0.047		
40		Hydrograph volume	0.015	0.015	369.534		
40		HYDROGRAPH Start - New Tributary"			0.015		
40		2 Start - New Tributary"	0.015	0.000	0.015	0.047"	
40		CATCHMENT 50"					
40		1 Triangular SCS"					
40		2 Proportional to %"					
40		1 SCS method"					
40		50 50 - Flow to Golf Course"					
40		7.000 % Impervious"					
40		2.010 Total Area"					
40		45.000 Flow length"					
40		3.000 Overland Slope"					
40		1.869 Pervious Area"					
40		45.000 Pervious length"					
40		3.000 Pervious slope"					
40		0.141 Impervious Area"					
40		3.387 Impervious length"					
40		3.000 Impervious slope"					
40		0.250 Pervious Manning "n" "					
40		74.000 Pervious SCS Curve No. "					
40		0.193 Pervious Runoff coefficient"					
40		0.100 Pervious Ia/S coefficient"					
40		8.924 Pervious Initial abstraction"					
40		0.015 Impervious Manning "n" "					
40		98.000 Impervious SCS Curve No. "					
40		0.764 Impervious Runoff coefficient"					
40		0.400 Impervious Ia/S coefficient"					
40		0.518 Impervious Initial abstraction"					
40		Catchment 50	0.034	0.000	0.015	0.047	
40		Surface Area					
40		Time of concentration			32.895		0.141
40		Time to Centroid			263.000		0.657
40		Rainfall depth			38.800		195.956
40		Rainfall volume			725.29		38.800
40		Rainfall losses			31.328		54.59
40		Runoff depth			7.472		9.160
40		Runoff volume					29.640
40		Runoff coefficient					
40		Maximum flow					
40		HYDROGRAPH Add "Runoff "					
40		4 Add "Runoff "	0.015	0.035	0.030	0.042"	
40		HYDROGRAPH Copy to "outflow"					
40		8 Copy to "outflow"	0.015	0.015	0.015	0.042"	
40		HYDROGRAPH " Combine 4"					
40		6 Combine					
40		4 Node #					
40		Total to Off-Site"					
40		Maximum flow			0.047		
40		Hydrograph volume	0.015	0.015	369.534		
40		HYDROGRAPH Start - New Tributary"			0.015		
40		2 Start - New Tributary"	0.015	0.000	0.015	0.047"	
40		CATCHMENT 50"					
40		1 Triangular SCS"					
40		2 Proportional to %"					

218173-PreDev-Oct01-2yr-SCS.out

Runoff volume 139.67 41.70 181.38  
 Runoff coefficient 0.193 0.764 0.233  
 Maximum flow 0.032 0.019 0.034  
 HYDROGRAPH Add Runoff " 0.034 0.019 0.034  
 4 Add Runoff " 0.034 0.034 0.015 0.047"  
 HYDROGRAPH Copy to Outflow" 0.034 0.034 0.047"  
 8 Copy to Outflow" 0.034 0.034 0.047"  
 HYDROGRAPH " Combine 3"  
 6 Combine " 3"  
 3 Node #  
 Total to Infiltration" 0.069  
 Maximum flow 0.069  
 Hydrograph volume 410.566  
 0.034 0.034 0.069  
 HYDROGRAPH Start - New Tributary"  
 2 Start - New Tributary" 0.034 0.034 0.069"  
 CATCHMENT 60"  
 1 Triangular SCS"  
 2 Proportional to %"  
 1 SCS method"  
 60 60-Infiltration on Golf Course"  
 0.000 % Impervious"  
 0.550 Total Area"  
 45.000 Flow Length"  
 3.000 Overland Slope"  
 0.550 Pervious Area"  
 45.000 Pervious Length"  
 3.000 Pervious Slope"  
 0.000 Pervious Area"  
 0.000 Pervious Slope"  
 0.000 Pervious Length"  
 0.250 Pervious Manning 'n'"  
 74.000 Pervious SCS Curve No."  
 0.193 Pervious Runoff coefficient"  
 0.100 Pervious Ia/s coefficient"  
 8.924 Pervious Initial abstraction"  
 0.015 Impervious Manning 'n'"  
 98.000 Impervious SCS Curve No."  
 0.000 Impervious Runoff coefficient"  
 0.100 Impervious Ia/s coefficient"  
 0.518 Impervious Initial abstraction"  
 0.009 0.000 0.034 0.069 c.m/sec"  
 Catchment 60 Pervious Total Area "  
 Surface Area 0.550 0.000 0.550 hectare"  
 Time of concentration 32.895 0.001 32.895 minutes"  
 Rainfall depth 194.150 38.800 262.999 mm"  
 Rainfall volume 213.40 0.00 213.40 c.m"  
 Rainfall losses 31.328 8.459 31.328 mm"  
 Runoff depth 7.472 30.341 7.472 mm"  
 Runoff volume 41.10 0.00 41.10 c.m"  
 Runoff coefficient 0.193 0.000 0.193  
 Maximum flow 0.009 0.000 0.009 c.m/sec"  
 HYDROGRAPH Add Runoff " 0.009 0.000 0.009  
 4 Add Runoff " 0.009 0.034 0.069"  
 HYDROGRAPH Copy to Outflow"  
 8 Copy to Outflow" 0.009 0.009 0.069"  
 HYDROGRAPH Combine 3" 0.009 0.069"  
 Page 7

218173-PreDev-Oct01-2yr-SCS.out

6 Combine " 0.078 c.m/sec"  
 3 Node #  
 Total to Infiltration" 451.663 c.m"  
 Maximum flow 0.009 0.078  
 Hydrograph volume 0.009  
 START/RE-START TOTALS 60"  
 3 Runoff Totals on EXIT"  
 Total Catchment area 21.320 hectare"  
 Total Impervious area 0.629 hectare"  
 Total % Impervious 2.948"  
 EXIT"

```

218173-PreDev-Oct01-Syr-SCS.out
-----
MIDUSS Output
MIDUSS version Version 2.25 rev. 473
MIDUSS created Sunday, February 07, 2010
Units used: C:\Users\jswiger\Desktop\Wilder Lake\ie METRIC
Job folder: C:\Users\jswiger\Desktop\Wilder Lake\MIDUSS
Output filename: 218173-PreDev-Oct01-Syr-SCS.out
Licensee name: Hewlett-Packard Company
Date & Time last used: 10/1/2019 at 11:32:59 AM
TIME PARAMETERS
31 10.000 Time Step
360.000 Max. Storm Length"
2400.000 Max. Hydrograph"
32 STORM Mass Curve
3 3 Mass Curve
49.400 Rainfall depth"
360.000 Duration"
47 C:\Program Files (x86)\MIDUSS\SCS_6hr_Type2.mrd SCS 6 hour Type
Maximum intensity 78.868 mm/hr"
Total depth 49.400 mm"
6 005hyd Hydrograph extension used in this file"
CATCHMENT 10"
1 Triangular SCS"
2 Proportional to %"
10 SCS method"
10 SCS method"
3.260 Total Area"
100.000 Flow length"
5.000 Overland Slope"
5.102 Pervious Area"
100.000 Pervious length"
5.000 Pervious slope"
0.158 Impervious Area"
3.093 Impervious length"
5.000 Impervious slope"
0.250 Pervious Manning "n"
0.255 Pervious SCS Curve No.
0.100 Pervious Runoff coefficient"
8.924 Pervious Ia/S coefficient"
0.015 Impervious Initial abstraction"
98.000 Impervious Manning "n"
0.776 Impervious SCS Curve No.
0.100 Impervious Runoff coefficient"
0.518 Impervious Ia/S coefficient"
0.139 Impervious Initial abstraction"
0.139 Pervious
Catchment 10 Pervious
Surface Area 5.102 Hectare"
Time of concentration 35.829 minutes"
Time to centroid 264.327 minutes"
Rainfall depth 49.400 mm"
Rainfall volume 2520.49 c.m"
Rainfall losses 36.787 mm"
Runoff volume 12.613 mm"
Runoff depth 38.352 mm"
Runoff coefficient 643.54 c.m"
Maximum flow 0.255 c.m/sec"
HYDROGRAPH Add Runoff "
4 Add Runoff" 0.139 0.000 0.000"

```

```

218173-PreDev-Oct01-Syr-SCS.out
-----
HYDROGRAPH Copy to Outflow"
8 Copy to Outflow" 0.139 0.139 0.000"
HYDROGRAPH " Combine " 1"
6 Combine " Combine " 1"
1 Node #
Wilder Lake Final"
Maximum flow 0.139 c.m/sec"
Hydrograph volume 704.058 c.m"
0.139 0.139 0.139"
HYDROGRAPH Start - New Tributary"
2 Start - New Tributary" 0.139 0.139 0.139"
0.139 0.000
CATCHMENT 20"
1 Triangular SCS"
2 Proportional to %"
1 SCS method"
20 SCS method"
20 SCS method"
6.000 % Impervious"
5.500 Total Area"
75.000 Flow length"
5.000 Overland Slope"
5.170 Pervious Area"
75.000 Pervious length"
5.000 Pervious slope"
0.330 Impervious Area"
4.787 Impervious length"
5.000 Impervious slope"
0.250 Pervious Manning "n"
74.000 Pervious SCS Curve No.
0.255 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No.
0.786 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.163 0.000 0.139
Catchment 20 Pervious
Surface Area 5.170 Hectare"
Time of concentration 30.149 minutes"
Time to centroid 257.210 minutes"
Rainfall depth 49.400 mm"
Rainfall volume 2553.98 c.m"
Rainfall losses 36.801 mm"
Runoff volume 12.599 mm"
Runoff depth 38.806 mm"
Runoff coefficient 651.36 c.m"
Maximum flow 0.255 c.m/sec"
HYDROGRAPH Add Runoff "
4 Add Runoff" 0.163 0.139 0.139"
HYDROGRAPH Copy to Outflow"
8 Copy to Outflow" 0.163 0.163 0.139"
HYDROGRAPH " Combine " 2"
6 Combine " Combine " 2"
2 Node #
Total to Creek"
Maximum flow 0.163 c.m/sec"
Hydrograph volume 779.416 c.m"
0.163 0.163 0.163"

```

		218173-Predev-Oct01-5yr-SCS.out	
40	HYDROGRAPH Start - New Tributary		
2	Start - New Tributary	0.163	0.163
40	HYDROGRAPH Start - New Tributary		
2	Start - New Tributary	0.163	0.163
33	CATCHMENT 30		
1	Triangular SCS		
2	Proportional to %		
1	SCS method		
30	30 - Infiltration		
0.000	% Impervious		
3.060	Total Area		
100.000	Flow Length		
3.060	Overland Slope		
3.060	Pervious Area		
100.000	Pervious length		
4.000	Pervious slope		
0.000	Impervious Area		
4.000	Impervious length		
4.000	Impervious slope		
0.250	Pervious Manning 'n'		
74.000	Pervious SCS Curve No.		
0.255	Pervious Runoff coefficient		
0.100	Pervious Ia/S coefficient		
8.924	Pervious Initial abstraction		
0.015	Impervious Manning 'n'		
98.000	Impervious SCS Curve No.		
0.100	Impervious Runoff coefficient		
0.100	Impervious Ia/S coefficient		
0.518	Impervious Initial abstraction		
	Catchment 30	0.077	0.163
	Surface Area	3.060	Pervious
	Time of Concentration	38.310	0.000
	Time to Centroid	267.503	0.001
	Rainfall depth	49.400	192.686
	Rainfall volume	1511.64	49.400
	Rainfall losses	36.796	0.00
	Runoff depth	12.604	9.631
	Runoff volume	385.68	39.769
	Runoff coefficient	0.255	0.00
	Maximum Flow	0.077	0.000
40	HYDROGRAPH Add Runoff		
4	Add Runoff	0.077	0.163
40	HYDROGRAPH Copy to Outflow		
8	Copy to Outflow	0.077	0.163
40	HYDROGRAPH		
6	Combine	Combine	
3	Node #	3	
	Total to Infiltration		
	Maximum Flow		0.077
	Hydrograph volume	0.077	385.677
		0.077	0.077
40	HYDROGRAPH Start - New Tributary		
2	Start - New Tributary	0.077	0.077
33	CATCHMENT 40		
1	Triangular SCS		
2	Proportional to %		
1	SCS method		

[illegible]



[illegible]

218173-PreDev-Oct01-5yr-SCS.out

Runoff volume 234.95 54.48 289.43  
 Runoff coefficient 0.254 0.784 0.291  
 Maximum flow 0.059 0.025 0.063  
 HYDROGRAPH Add Runoff " 0.097"  
 4 Add Runoff " 0.063 0.063 0.030 0.097"  
 HYDROGRAPH Copy to Outflow"  
 8 Copy to Outflow" 0.063 0.063 0.097"  
 HYDROGRAPH " Combine 3"  
 6 Combine " Node #"  
 3 Node #"  
 Total to Infiltration" 0.137  
 Maximum flow 675.108  
 Hydrograph volume 0.063 0.137  
 HYDROGRAPH Start - New Tributary" 0.063  
 2 Start - New Tributary" 0.000 0.063 0.137"  
 CATCHMENT 60"  
 1 Triangular SCS"  
 2 Proportional to %"  
 1 SCS method" 60-Infiltration on Golf Course"  
 60 % Infiltration"  
 0.000 Total Area"  
 0.550 Flow Length"  
 43.000 Overland Slope"  
 3.000 Pervious Area"  
 45.000 Pervious Length"  
 3.000 Pervious Slope"  
 0.000 Pervious Area"  
 0.000 Pervious Length"  
 3.000 Pervious Slope"  
 0.250 Pervious Manning "n"  
 74.000 Pervious SCS Curve No."  
 0.254 Pervious Runoff coefficient"  
 0.100 Pervious Ia/S coefficient"  
 8.924 Pervious Initial abstraction"  
 0.015 Pervious Manning "n"  
 98.000 Pervious SCS Curve No."  
 0.000 Pervious Runoff coefficient"  
 0.100 Pervious Ia/S coefficient"  
 0.518 Pervious Initial abstraction"  
 0.017 0.000 0.063 0.137 c.m/sec"  
 Catchment 60 Pervious Total Area " hectare"  
 Surface Area 0.550 0.000 0.550  
 Time of concentration 23.885 0.000 23.885  
 Rainfall depth 251.800 192.886 251.800  
 Rainfall volume 49.400 49.400 49.400  
 Rainfall losses 271.70 0.00 271.70  
 Runoff depth 36.831 9.629 36.831  
 Runoff volume 12.569 0.00 12.569  
 Runoff coefficient 69.13 0.00 69.13  
 Maximum flow 0.254 0.000 0.254  
 HYDROGRAPH Add Runoff " 0.017 0.000 0.017  
 4 Add Runoff " 0.017 0.063 0.137"  
 HYDROGRAPH Copy to Outflow"  
 8 Copy to Outflow" 0.017 0.017 0.137"  
 HYDROGRAPH Combine 3" Page 7

218173-PreDev-Oct01-5yr-SCS.out

6 Combine " Node #"  
 3 Node #"  
 Total to Infiltration" 0.154  
 Maximum flow 744.237  
 Hydrograph volume 0.017 0.154  
 START/RE-START TOTALS 60"  
 3 Runoff Totals on EXIT"  
 Total Catchment area 21.320  
 Total Impervious area 0.629  
 Total % Impervious 2.948"  
 EXIT" hectare"  
 hectare"

```

218173-PreDev-Oct03-10yr-SCS.out
MIDUSS Output ----->
MIDUSS version Version 2.25 rev. 473
MIDUSS created Sunday, February 07, 2010
Units used: ie METRIC
Job folder: C:\Users\jswiger\Desktop\wilder Lake\
MIDUSS
Output filename: 218173-PreDev-Oct03-10yr-SCS.out
License name:
Company: Hewlett-Packard Company
Date & Time last used: 10/3/2019 at 2:42:23 PM
TIME PARAMETERS
10 10.000 Time Step
360.000 Max. Storm length"
2400.000 Max. Hydrograph
3 STORM Mass Curve
56.400 Rainfall depth"
360.000 Duration"
47 C:\Program Files (x86)\MIDUSS\SCS_6hr_Type2.mrd SCS 6 hour Type
Maximum intensity 90.044 mm/hr"
Total depth 56.400 mm"
6 010hyd Hydrograph extension used in this file"
CATCHMENT 10"
1 Triangular SCS"
2 Proportional to %"
1 SCS method"
10 10- Wilder Lake South"
3.000 % Impervious"
3.260 Total Area"
100.000 Flow length"
5.000 Overland Slope"
5.102 Pervious Area"
100.000 Pervious length"
5.000 Pervious slope"
0.158 Impervious Area"
3.093 Impervious length"
5.000 Impervious slope"
0.250 Pervious Manning "n"
74.000 Pervious SCS Curve No."
0.292 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No."
0.785 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
Catchment 10 Pervious 0.000 0.000 c.m/sec"
Surface Area 5.102 Pervious Total Area "
Time of concentration 31.838 Pervious 5.102 c.m/sec"
Time to Centroid 258.268 Pervious 29.425 minutes"
Rainfall depth 56.400 Pervious 192.500 mm"
Rainfall volume 2877.64 Pervious 56.400 c.m"
Rainfall losses 39.957 Pervious 2966.64 mm"
Runoff depth 16.443 Pervious 39.122 mm"
Runoff volume 838.95 Pervious 17.278 c.m"
Runoff coefficient 0.292 Pervious 69.88 c.m/sec"
Maximum flow 0.192 Pervious 908.83 c.m/sec"
HYDROGRAPH Add Runoff "
4 Add Runoff " 0.196 0.000 0.000 0.000"
Page 1

```

```

218173-PreDev-Oct03-10yr-SCS.out
8 HYDROGRAPH Copy to Outflow" 0.196 0.196 0.000"
HYDROGRAPH 1"
6 Combine " Combine 1"
1 Node #
Wilder Lake Final"
Maximum flow 0.196 c.m/sec"
Hydrograph volume 908.834 c.m"
HYDROGRAPH Start - New Tributary" 0.196 0.196"
2 Start - New Tributary" 0.000 0.196 0.196"
CATCHMENT 20"
1 Triangular SCS"
2 Proportional to %"
1 SCS method"
20 20 - Creek"
6.000 % Impervious"
5.500 Total Area"
75.000 Flow length"
5.000 Overland Slope"
5.170 Pervious Area"
75.000 Pervious length"
5.000 Pervious slope"
0.330 Impervious Area"
4.787 Impervious length"
5.000 Impervious slope"
0.250 Pervious Manning "n"
74.000 Pervious SCS Curve No."
0.292 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No."
0.795 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
Catchment 20 Pervious 0.000 0.196 c.m/sec"
Surface Area 5.170 Pervious Total Area "
Time of concentration 26.791 Pervious 5.500 hectare"
Time to Centroid 251.765 Pervious 22.908 minutes"
Rainfall depth 56.400 Pervious 193.283 mm"
Rainfall volume 2915.88 Pervious 56.400 c.m"
Rainfall losses 39.957 Pervious 186.12 mm"
Runoff depth 16.443 Pervious 3102.00 mm"
Runoff volume 850.10 Pervious 38.255 c.m"
Runoff coefficient 0.292 Pervious 44.812 mm"
Maximum flow 0.210 Pervious 147.88 c.m"
HYDROGRAPH Add Runoff " 0.210 0.069 0.322 c.m/sec"
4 Add Runoff " 0.220 0.220 0.196"
HYDROGRAPH Copy to Outflow"
8 Copy to Outflow" 0.220 0.220 0.196"
HYDROGRAPH 2"
6 Combine " Combine 2"
1 Node #
Wilder Lake Final"
Maximum flow 0.220 c.m/sec"
Hydrograph volume 997.979 c.m"
HYDROGRAPH Add Runoff " 0.220 0.220 0.220"
Page 2

```

Run	Iteration	Time (s)	Memory (MB)	Output	Time (s)	Memory (MB)	Output
40	1	0.000	0.000	HYDROGRAPH Start - New Tributary	0.000	0.000	0.000
40	2	0.000	0.000	2 Start - New Tributary	0.000	0.000	0.000
40	1	0.000	0.000	HYDROGRAPH Start - New Tributary	0.000	0.000	0.000
40	2	0.000	0.000	2 Start - New Tributary	0.000	0.000	0.000
33	1	0.000	0.000	CATCHMENT 30"	0.000	0.000	0.000
33	2	0.000	0.000	2 Triangular SCS	0.000	0.000	0.000
33	3	0.000	0.000	3 Proportional to %	0.000	0.000	0.000
33	4	0.000	0.000	4 SCS method	0.000	0.000	0.000
33	5	0.000	0.000	5 30 - Infiltration	0.000	0.000	0.000
33	6	0.000	0.000	6 % Impervious	0.000	0.000	0.000
33	7	0.000	0.000	7 Total Area	0.000	0.000	0.000
33	8	0.000	0.000	8 Flow length	0.000	0.000	0.000
33	9	0.000	0.000	9 Overland Slope	0.000	0.000	0.000
33	10	0.000	0.000	10 Pervious Area	0.000	0.000	0.000
33	11	0.000	0.000	11 Pervious Slope	0.000	0.000	0.000
33	12	0.000	0.000	12 Pervious Area	0.000	0.000	0.000
33	13	0.000	0.000	13 Impervious Slope	0.000	0.000	0.000
33	14	0.000	0.000	14 Impervious Slope	0.000	0.000	0.000
33	15	0.000	0.000	15 Pervious Manning 'n'	0.000	0.000	0.000
33	16	0.000	0.000	16 Pervious SCS Curve No.	0.000	0.000	0.000
33	17	0.000	0.000	17 Pervious Runoff coefficient	0.000	0.000	0.000
33	18	0.000	0.000	18 Pervious Ia/S coefficient	0.000	0.000	0.000
33	19	0.000	0.000	19 Pervious Initial abstraction	0.000	0.000	0.000
33	20	0.000	0.000	20 Impervious Manning 'n'	0.000	0.000	0.000
33	21	0.000	0.000	21 Impervious SCS Curve No.	0.000	0.000	0.000
33	22	0.000	0.000	22 Impervious Runoff coefficient	0.000	0.000	0.000
33	23	0.000	0.000	23 Impervious Ia/S coefficient	0.000	0.000	0.000
33	24	0.000	0.000	24 Impervious Initial abstraction	0.000	0.000	0.000
33	25	0.000	0.000	25 CATCHMENT 30"	0.000	0.000	0.000
33	26	0.000	0.000	26 Surface Area	0.000	0.000	0.000
33	27	0.000	0.000	27 Time to Concentration	0.000	0.000	0.000
33	28	0.000	0.000	28 Time to Centroid	0.000	0.000	0.000
33	29	0.000	0.000	29 Rainfall depth	0.000	0.000	0.000
33	30	0.000	0.000	30 Rainfall volume	0.000	0.000	0.000
33	31	0.000	0.000	31 Rainfall losses	0.000	0.000	0.000
33	32	0.000	0.000	32 Rainfall depth	0.000	0.000	0.000
33	33	0.000	0.000	33 Runoff volume	0.000	0.000	0.000
33	34	0.000	0.000	34 Runoff coefficient	0.000	0.000	0.000
33	35	0.000	0.000	35 Maximum flow	0.000	0.000	0.000
33	36	0.000	0.000	36 HYDROGRAPH Add Runoff	0.000	0.000	0.000
33	37	0.000	0.000	37 4 Add Runoff	0.000	0.000	0.000
33	38	0.000	0.000	38 HYDROGRAPH Copy to Outflow	0.000	0.000	0.000
33	39	0.000	0.000	39 8 Copy to Outflow	0.000	0.000	0.000
33	40	0.000	0.000	40 HYDROGRAPH Combine	0.000	0.000	0.000
33	41	0.000	0.000				

Line	Code	Description	Value	Unit	Value	Unit
40	40	40 - South East Flow off-site				
41	0.000	% Impervious				
42	1.070	Total Area				
43	60.000	Flow length				
44	2.000	Overland Slope				
45	1.070	Pervious Area				
46	60.000	pervious length				
47	2.000	pervious slope				
48	0.000	Impervious Area				
49	0.000	Impervious length				
50	2.000	Impervious slope				
51	0.250	pervious Manning 'n'				
52	74.000	pervious SCS Curve No.				
53	0.292	pervious runoff coefficient				
54	0.100	pervious Ia/S coefficient				
55	8.924	pervious initial abstraction				
56	0.015	Impervious Manning 'n'				
57	98.000	Impervious SCS Curve No.				
58	0.000	Impervious runoff coefficient				
59	0.100	Impervious Ia/S coefficient				
60	0.518	Impervious initial abstraction				
61	0.042	0.000	0.111			
62	Catchment 40	Pervious	0.111			
63	Surface Area	1.070	Impervious	0.000		
64	Time of concentration	30.848	Impervious	0.001		
65	Time to Centroid	257.034	Impervious	191.938		
66	Rainfall depth	56.400	Impervious	56.400		
67	Rainfall volume	603.48	Impervious	0.00		
68	Rainfall losses	39.948	Impervious	10.382		
69	Rainfall depth	16.452	Impervious	46.018		
70	Runoff volume	176.04	Impervious	0.000		
71	Runoff coefficient	0.292	Impervious	0.000		
72	Maximum Flow	0.042	Impervious	0.000		
73	HYDROGRAPH Add, Runoff	0.042	Impervious	0.000		
74	4 Add Runoff	0.042	0.042	0.111		
75	8 COPY to Outflow	0.042	0.042	0.111		
76	HYDROGRAPH Copy to Outflow	0.042	0.042	0.111		
77	6 Combine	Combine	4	0.042		
78	4 Node #					
79	Total to Off-Site					
80	Maximum flow					
81	Hydrograph volume	0.042	0.042	0.042		
82	2 Start - New Tributary					
83	41 - South West Flow off site					
84	CATCHMENT 41	0.042	0.000	0.042		
85	1 Triangular SCS					
86	2 Proportional to %					
87	1 SCS method					
88	41 - South West Flow off site					
89	% Impervious					
90	0.000	Total Area				
91	3.180	Flow length				
92	185.000	Overland Slope				
93	4.000	Pervious Area				
94	3.180	pervious length				
95	185.000	pervious slope				
96	4.000	Impervious Area				
97	0.000	Impervious length				
98	0.000	Impervious slope				
99	0.000	Impervious Area				
100	0.000	Impervious length				
101	0.000	Impervious slope				
102	0.000	Impervious Area				
103	0.000	Impervious length				
104	0.000	Impervious slope				
105	0.000	Impervious Area				
106	0.000	Impervious length				
107	0.000	Impervious slope				
108	0.000	Impervious Area				

218173-PreDev-Oct03-10yr-SCS.out			
4.000	Impervious slope "		
0.250	Pervious Manning "n"		
74.000	Pervious SCS Curve No.		
0.292	Pervious Runoff coefficient "		
0.100	Pervious Ia/S coefficient "		
8.924	Pervious Initial abstraction "		
0.015	Impervious Manning "n"		
98.000	Impervious SCS Curve No.		
0.000	Impervious Runoff coefficient "		
0.100	Impervious Ia/S coefficient "		
0.518	Impervious Initial abstraction "		
	0.089	0.000	0.042
	Catchment 41	Pervious	Impervious
	Surface Area	3,180	0.000
	Time of Concentration	49,241	0.001
	Time to Centroid	280,586	191,938
	Rainfall depth	56,400	56,400
	Rainfall volume	1793.52	0.00
	Rainfall losses	39.925	10.386
	Runoff depth	16.475	46.014
	Runoff volume	523,90	0.00
	Runoff coefficient	0.252	0.000
	Maximum flow		0.000
40	HYDROGRAPH Add, Runoff "	0.089	
	4 Add Runoff "		0.042
40	HYDROGRAPH Copy to, Outflow "	0.089	0.042
8	Copy to Outflow "	0.089	0.042
40	HYDROGRAPH " Combine 4"	0.089	0.089
	6 Combine " Combine 4"		
4	Note "		
	Total to Off-Site "		
	Maximum flow	0.127	
	Hydrograph volume	699,935	
	0.089	0.089	0.127
40	HYDROGRAPH Start - New Tributary "		
2	Start - New Tributary "	0.089	
	0.089	0.000	0.089
33	CATCHMENT 42 "		
1	Triangular SCS "		
2	Proportional to % "		
1	SCS method "		
42	42 - West Flow off Site "		
	% Impervious "		
0.000	Total Area "		
0.690	Flow length "		
20,000	Overland Slope "		
0.690	Pervious Area "		
20,000	Pervious length "		
0.000	Pervious slope "		
0.000	Impervious Area "		
0.000	Impervious length "		
4.000	Impervious slope "		
0.250	Pervious Manning "n"		
74.000	Pervious SCS Curve No.		
0.289	Pervious Runoff coefficient "		
0.100	Pervious Ia/S coefficient "		
8.924	Pervious Initial abstraction "		
0.015	Impervious Manning "n"		
98.000	Impervious SCS Curve No.		
0.000	Impervious Runoff coefficient "		
0.100	Impervious Ia/S coefficient "		

Page 5

218173-PreDev-Oct03-10yr-SCS.out

0.518	Impervius Initial abstraction	0.039	0.000	0.089	Pervious	0.127	c.m/sec"
	Catchment 42				Impervius	0.000	Total Area
	Surface Area					0.690	hectare"
	Time of concentration					12.961	minutes"
	Time to Centroid					234.306	minutes"
	Rainfall depth					56.400	mm"
	Rainfall volume					389.16	C.m"
	Rainfall losses					40.128	mm"
	Rainfall volume					16.272	mm"
	Rainfall losses					112.28	C.m"
	Runoff volume					0.289	C.m/Sec"
	Runoff coefficient					0.000	
	Maximum flow					0.000	
	HYDROGRAPH Add Runoff "					0.039	
40	4 Add Runoff "					0.039	0.127"
	HYDROGRAPH Copy to Outflow"						
40	8 Copy to Outflow"					0.039	0.127"
	HYDROGRAPH " Combine 4"						
40	6 Combine "						
	4 Node #"						
	Total to off-site"						
	Maximum flow					0.144	
	Hydrograph volume					812.213	c.m/sec"
	0.039					0.039	c.m"
	HYDROGRAPH Start - New Tributary"						0.144"
40	2 Start - New Tributary"						
	0.039					0.000	0.144"
33	CATCHMENT 50"						
	1 Triangular SCS"						
	2 Proportional to %"						
	SCS method"						
	50 50 - Flow to Golf Course"						
	7.000 % Impervious"						
	2.010 Total Area"						
	45.000 Flow length"						
	3.000 Overland Slope"						
	1.869 Pervious Area"						
	45.000 Pervious length"						
	3.000 Pervious slope"						
	0.141 Impervious Area"						
	3.387 Impervious length"						
	3.000 Impervious slope"						
	0.250 Pervious Manning "n"						
	74.000 Pervious SCS Curve No."						
	0.292 Pervious Runoff coefficient"						
	0.100 Pervious Ia/S coefficient"						
	8.924 Pervious Initial abstraction"						
	0.015 Impervious Manning "n"						
	98.000 Impervious SCS Curve No."						
	0.793 Impervious Runoff coefficient"						
	0.100 Impervious Ia/S coefficient"						
	Impervius Initial abstraction"						
0.518	0.089					0.039	0.144
	Catchment 50					Impervius	Total Area
	Surface Area					1.869	hectare"
	Time of concentration					22.984	minutes"
	Time to Centroid					246.791	mm"
	Rainfall depth					56.400	C.m"
	Rainfall volume					389.16	mm"
	Rainfall losses					40.128	C.m"
	Rainfall volume					16.272	mm"
	Rainfall losses					112.28	C.m"
	Runoff depth					0.289	C.m/Sec"
	Runoff coefficient					0.000	
	Maximum flow					0.000	
	HYDROGRAPH Add Runoff "					0.039	
40	4 Add Runoff "					0.039	0.127"
	HYDROGRAPH Copy to Outflow"						
40	8 Copy to Outflow"					0.039	0.127"
	HYDROGRAPH " Combine 4"						
40	6 Combine "						
	4 Node #"						
	Total to off-site"						
	Maximum flow					0.144	
	Hydrograph volume					812.213	c.m/sec"
	0.039					0.039	c.m"
	HYDROGRAPH Start - New Tributary"						0.144"
40	2 Start - New Tributary"						
	0.039					0.000	0.144"
33	CATCHMENT 50"						
	1 Triangular SCS"						
	2 Proportional to %"						
	SCS method"						
	50 50 - Flow to Golf Course"						
	7.000 % Impervious"						
	2.010 Total Area"						
	45.000 Flow length"						
	3.000 Overland Slope"						
	1.869 Pervious Area"						
	45.000 Pervious length"						
	3.000 Pervious slope"						
	0.141 Impervious Area"						
	3.387 Impervious length"						
	3.000 Impervious slope"						
	0.250 Pervious Manning "n"						
	74.000 Pervious SCS Curve No."						
	0.292 Pervious Runoff coefficient"						
	0.100 Pervious Ia/S coefficient"						
	8.924 Pervious Initial abstraction"						
	0.015 Impervious Manning "n"						
	98.000 Impervious SCS Curve No."						
	0.793 Impervious Runoff coefficient"						
	0.100 Impervious Ia/S coefficient"						
	Impervius Initial abstraction"						
0.518	0.089					0.039	0.144
	Catchment 50					Impervius	Total Area
	Surface Area					1.869	hectare"
	Time of concentration					22.984	minutes"
	Time to Centroid					246.791	mm"
	Rainfall depth					56.400	C.m"
	Rainfall volume					389.16	mm"
	Rainfall losses					40.128	C.m"
	Rainfall volume					16.272	mm"
	Rainfall losses					112.28	C.m"
	Runoff depth					0.289	C.m/Sec"
	Runoff coefficient					0.000	
	Maximum flow					0.000	
	HYDROGRAPH Add Runoff "					0.039	
40	4 Add Runoff "					0.039	0.127"
	HYDROGRAPH Copy to Outflow"						
40	8 Copy to Outflow"					0.039	0.127"
	HYDROGRAPH " Combine 4"						
40	6 Combine "						
	4 Node #"						
	Total to off-site"						
	Maximum flow					0.144	
	Hydrograph volume					812.213	c.m/sec"
	0.039					0.039	c.m"
	HYDROGRAPH Start - New Tributary"						0.144"
40	2 Start - New Tributary"						
	0.039					0.000	0.144"
33	CATCHMENT 50"						
	1 Triangular SCS"						
	2 Proportional to %"						
	SCS method"						
	50 50 - Flow to Golf Course"						
	7.000 % Impervious"						
	2.010 Total Area"						
	45.000 Flow length"						
	3.000 Overland Slope"						
	1.869 Pervious Area"						
	45.000 Pervious length"						
	3.000 Pervious slope"						
	0.141 Impervious Area"						
	3.387 Impervious length"						
	3.000 Impervious slope"						
	0.250 Pervious Manning "n"						
	74.000 Pervious SCS Curve No."						
	0.292 Pervious Runoff coefficient"						
	0.100 Pervious Ia/S coefficient"						
	8.924 Pervious Initial abstraction"						
	0.015 Impervious Manning "n"						
	98.000 Impervious SCS Curve No."						
	0.793 Impervious Runoff coefficient"						
	0.100 Impervious Ia/S coefficient"						
	Impervius Initial abstraction"						
0.518	0.089					0.039	0.144
	Catchment 50					Impervius	Total Area
	Surface Area					1.869	hectare"
	Time of concentration					22.984	minutes"
	Time to Centroid					246.791	mm"
	Rainfall depth					56.400	C.m"
	Rainfall volume					389.16	mm"
	Rainfall losses					40.128	C.m"
	Rainfall volume					16.272	mm"
	Rainfall losses					112.28	C.m"
	Runoff depth					0.289	C.m/Sec"
	Runoff coefficient					0.000	
	Maximum flow					0.000	
	HYDROGRAPH Add Runoff "					0.039	
40	4 Add Runoff "					0.039	0.127"
	HYDROGRAPH Copy to Outflow"						
40	8 Copy to Outflow"					0.039	0.127"
	HYDROGRAPH " Combine 4"						
40	6 Combine "						
	4 Node #"						
	Total to off-site"						
	Maximum flow					0.144	
	Hydrograph volume					812.213	c.m/sec"
	0.039					0.039	c.m"
	HYDROGRAPH Start - New Tributary"						0.144"
40	2 Start - New Tributary"						
	0.039					0.000	0.144"
33	CATCHMENT 50"						
	1 Triangular SCS"						
	2 Proportional to %"						
	SCS method"						
	50 50 - Flow to Golf Course"						
	7.000 % Impervious"						
	2.010 Total Area"						
	45.000 Flow length"						
	3.000 Overland Slope"						
	1.869 Pervious Area"						
	45.000 Pervious length"						
	3.000 Pervious slope"						
	0.141 Impervious Area"						
	3.387 Impervious length"						
	3.000 Impervious slope"						
	0.250 Pervious Manning "n"						
	74.000 Pervious SCS Curve No."						
	0.292 Pervious Runoff coefficient"						
	0.100 Pervious Ia/S coefficient"						
	8.924 Pervious Initial abstraction"						
	0.015 Impervious Manning "n"						
	98.000 Impervious SCS Curve No."						
	0.793 Impervious Runoff coefficient"						
	0.100 Impervious Ia/S coefficient"						
	Impervius Initial abstraction"						
0.518	0.089					0.039	0.144
	Catchment 50					Impervius	Total Area
	Surface Area					1.869	hectare"
	Time of concentration					22.984	minutes"
	Time to Centroid					246.791	mm"
	Rainfall depth					56.400	C.m"
	Rainfall volume					389.16	mm"
	Rainfall losses					40.128	C.m"
	Rainfall volume					16.272	mm"
	Rainfall losses					112.28	C.m"
	Runoff depth					0.289	C.m/Sec"
	Runoff coefficient					0.000	
	Maximum flow					0.000	
	HYDROGRAPH Add Runoff "					0.039	
40	4 Add Runoff "					0.039	0.127"
	HYDROGRAPH Copy to Outflow"						
40	8 Copy to Outflow"					0.039	0.127"
	HYDROGRAPH " Combine 4"						
40	6 Combine "						
	4 Node #"						
	Total to off-site"						
	Maximum flow					0.144	
	Hydrograph volume					812.213	c.m/sec"
	0.039					0.039	c.m"
	HYDROGRAPH Start - New Tributary"						0.144"
40	2 Start - New Tributary"						
	0.039					0.000	0.144"
33	CATCHMENT 50"						
	1 Triangular SCS"						
	2 Proportional to %"						
	SCS method"						
	50 50 - Flow to Golf Course"						
	7.000 % Impervious"						
	2.010 Total Area"						
	45.000 Flow length"						
	3.000 Overland Slope"						
	1.869 Pervious Area"						
	45.000 Pervious length"						
	3.000 Pervious slope"						
	0.141 Impervious Area"						
	3.387 Impervious length"						
	3.000 Impervious slope"						
	0.250 Pervious Manning "n"						
	74.000 Pervious SCS Curve No."						
	0.292 Pervious Runoff coefficient"						
	0.100 Pervious Ia/S coefficient"						
	8.924 Pervious Initial abstraction"						
	0.015 Impervious Manning "n"						
	98.000 Impervious SCS Curve No."						
	0.793 Impervious Runoff coefficient"						
	0.100 Impervious Ia/S coefficient"						
	Impervius Initial abstraction"						

```

218173-PreDev-Oct03-10yr-SCS.out
Runoff volume 307.60 62.90 370.50
Runoff coefficient 0.292 0.793 0.327
Maximum flow 0.085 0.029 0.089
HYDROGRAPH Add Runoff "
4 Add Runoff " 0.089 0.089 0.039 0.144"
HYDROGRAPH Copy to Outflow"
8 Copy to Outflow" 0.089 0.089 0.144"
HYDROGRAPH " Combine 3"
6 Combine "
3 Node #
Total to Infiltration"
Maximum flow 0.189 c.m/sec"
Hydrograph volume 874.511 c.m"
0.089 0.089 0.189"
HYDROGRAPH start - New Tributary"
2 Start - New Tributary" 0.089 0.189"
CATCHMENT 60"
1 Triangular SCS"
2 Proportional to %"
SCS method"
60 60-Infiltration on Golf Course"
0.000 % Impervious"
0.550 Total Area"
45.000 Flow length"
3.000 Overland Slope"
0.550 Pervious Area"
45.000 Pervious length"
3.000 Pervious slope"
0.000 Impervious Area"
0.000 Impervious length"
3.000 Impervious slope"
0.250 Pervious Manning 'n'"
74.000 Pervious SCS Curve No."
0.292 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Pervious Manning 'n'"
98.000 Pervious SCS Curve No."
0.000 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
0.518 Pervious Initial abstraction"
0.025 0.000 0.089 0.189 c.m/sec"
Catchment 60 Pervious Total Area "
Surface Area 0.550 Impervious 0.000 hectare"
Time of concentration 22.984 0.001 22.984 minutes"
Rainfall depth 246.791 191.938 246.791 mm
Rainfall volume 56.400 56.400 56.400 c.m"
Rainfall losses 310.20 0.00 310.20 mm"
Runoff depth 39.945 10.381 16.455 mm"
Runoff volume 16.455 46.019 16.455 c.m"
Runoff coefficient 90.50 0.00 90.50 "
Maximum flow 0.292 0.000 0.292 c.m/sec"
HYDROGRAPH Add Runoff "
4 Add Runoff " 0.025 0.025 0.089 0.189"
HYDROGRAPH Copy to Outflow"
8 Copy to Outflow" 0.025 0.025 0.189"
HYDROGRAPH Combine 3"

```

```

218173-PreDev-Oct03-10yr-SCS.out
6 Combine "
3 Node #
Total to Infiltration"
Maximum flow 0.212 c.m/sec"
Hydrograph volume 965.016 c.m"
0.025 0.212"
START/RE-START TOTALS 60"
3 Runoff Totals on EXIT"
Total Catchment area 21.320 hectare"
Total % Impervious area 0.629 hectare"
Total % Impervious 2.948"
EXIT"

```

```

218173-PreDev-Oct03-25yr-SCS.out
-----
MIDUSS Output Version 2.25 rev. 473
MIDUSS version Sunday, February 07, 2010
Units used: C:\Users\jswiger\Desktop\Wildier Lake\
Job folder: MIDUSS
Output filename: 218173-PreDev-Oct03-25yr-SCS.out
Licensee name: Hewlett-Packard Company
Date & Time last used: 10/3/2019 at 2:46:50 PM

TIME PARAMETERS
31 10.000 Time Step
360.000 Max. Storm length"
2400.000 Max. Hydrograph"
32 STORM Mass Curve
3 360.000 Rainfall depth"
360.000 Duration"
47 C:\Program Files (x86)\MIDUSS\SCS_6hr_Type2.mrd SCS 6 hour Type

Maximum intensity 104.253 mm/hr"
Total depth 65.300 mm"
6 025hyd Hydrograph extension used in this file"
CATCHMENT 10"
1 Triangular SCS"
2 Proportional to %"
10 10- Wildier Lake South"
SCS method"
% Impervious"
3.000 Total Area"
100.000 Flow length"
5.000 Overland Slope"
5.102 Pervious Area"
100.000 Pervious length"
5.000 Pervious slope"
0.158 Impervious Area"
3.093 Impervious length"
5.000 Impervious slope"
0.250 Pervious Manning "n"
0.334 Pervious SCS Curve No."
0.100 Pervious Runoff coefficient"
8.924 Pervious Ia/S coefficient"
0.015 Pervious Initial abstraction"
98.000 Impervious Manning "n"
0.794 Impervious SCS Curve No."
0.100 Impervious Runoff coefficient"
0.518 Impervious Ia/S coefficient"
0.000 Impervious Initial abstraction"
0.278 Pervious
0.000 Pervious
Imperious Total Area"
Surface Area 5.102 hectare"
Time of concentration 28.178 minutes"
Time to Centroid 252.330 minutes"
Rainfall depth 65.300 mm"
Rainfall volume 3331.74 C.m"
Rainfall losses 43.511 mm"
Runoff depth 111.72 mm"
Runoff volume 111.72 C.m"
Runoff coefficient 0.334 C.m/sec"
Maximum flow 0.273
HYDROGRAPH Add Runoff "
4 Add Runoff" 0.278 0.278 0.000 0.000"
Page 1

```

```

218173-PreDev-Oct03-25yr-SCS.out
-----
8 HYDROGRAPH Copy to Outflow" 0.278 0.278 0.000"
Copy to Outflow" 0.278 0.278 0.000"
HYDROGRAPH " Combine 1" 0.278 0.278 0.000"
6 Combine " Combine 1" 0.278 0.278 0.000"
1 Node #
Wildier Lake Final"
Maximum Flow 0.278 c.m./sec"
Hydrograph Volume 1193.537 c.m"
0.278 0.278 0.278
HYDROGRAPH Start - New Tributary"
2 Start - New Tributary" 0.278 0.278 0.278"
0.000 0.278 0.278"
CATCHMENT 20"
1 Triangular SCS"
2 Proportional to %"
1 SCS method"
20 20 - Creek"
6.000 % Impervious"
5.500 Total Area"
75.000 Flow length"
5.000 Overland Slope"
5.170 Pervious Area"
75.000 Pervious length"
5.000 Pervious slope"
0.330 Impervious Area"
4.787 Impervious length"
5.000 Impervious slope"
0.250 Pervious Manning "n"
74.000 Pervious SCS Curve No."
0.333 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No."
0.803 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.321 0.000 0.278 0.278 c.m./sec"
Catchment 20 Imperious Total Area"
Surface Area 5.170 hectare"
Time of concentration 23.711 minutes"
Time to Centroid 226.463 minutes"
Rainfall depth 65.300 mm"
Rainfall volume 3376.01 C.m"
Rainfall losses 43.352 mm"
Runoff depth 21.748 mm"
Runoff volume 1124.37 C.m"
Runoff coefficient 0.333 c.m./sec"
Maximum flow 0.309
HYDROGRAPH Add Runoff "
4 Add Runoff" 0.321 0.321 0.278 0.278"
HYDROGRAPH Copy to Outflow" 0.321 0.321 0.278"
8 Copy to Outflow" 0.321 0.321 0.278"
HYDROGRAPH " Combine 2" 0.321 0.321 0.278"
6 Combine " Combine 2" 0.321 0.321 0.278"
2 Node #
Total to Creek"
Maximum flow 0.321 c.m./sec"
Hydrograph Volume 1297.388 c.m"
0.321 0.321 0.321"
Page 2

```



218173-PreDev-Oct03-25yr-SCS.out

HYDROGRAPH Start - New Tributary" 0.321 0.321"

2 Start - New Tributary" 0.321 0.321"

HYDROGRAPH Start - New Tributary" 0.321 0.321"

2 Start - New Tributary" 0.321 0.321"

CATCHMENT 30"

1 Triangular SCS"

2 Proportional to %"

3 SCS method"

30 - Infiltration"

3.060 Total Area"

100.000 Flow length"

4.000 Overland Slope"

3.060 Pervious Area"

100.000 Pervious length"

4.000 Pervious slope"

0.000 Pervious Area"

0.000 Pervious length"

4.000 Pervious slope"

0.250 Pervious Manning 'n'"

74.000 Pervious SCS Curve No."

0.334 Pervious Runoff coefficient"

0.100 Pervious Ia/S coefficient"

8.924 Pervious Initial abstraction"

0.015 Pervious Manning 'n'"

98.000 Pervious SCS Curve No."

0.000 Pervious Runoff coefficient"

0.100 Pervious Ia/S coefficient"

0.518 Pervious Initial abstraction"

0.163 0.321 c.m/sec"

Catchment 30 Pervious

Surface Area 3.060 Total Area"

Time of concentration 30.129 30.129

Time to Centroid 254.933 254.933

Rainfall depth 65.300 65.300

Rainfall volume 1998.18 1998.18

Rainfall losses 43.513 43.513

Runoff depth 21.787 21.787

Runoff volume 666.67 666.67

Runoff coefficient 0.334 0.334

Maximum flow 0.163 0.163

HYDROGRAPH Add Runoff"

4 Add Runoff" 0.163 0.163 0.321 0.321"

HYDROGRAPH Copy to Outflow" 0.163 0.321 0.321"

8 Copy to Outflow" 0.163 0.163 0.321 0.321"

HYDROGRAPH " Combine 3" 0.163 0.163 0.321 0.321"

6 Combine " Combine 3"

3 Node #"

Total to Infiltration"

Maximum flow 0.163 0.163 c.m/sec"

Hydrograph volume 666.675 666.675

0.163 0.163 c.m"

HYDROGRAPH Start - New Tributary"

2 Start - New Tributary" 0.163 0.163

CATCHMENT 40"

1 Triangular SCS"

2 Proportional to %"

1 SCS method"

Page 3

218173-PreDev-Oct03-25yr-SCS.out

40 - South East Flow Off-Site"

40 % Impervious"

1.070 Total Area"

60.000 Flow length"

2.000 Overland Slope"

1.070 Pervious Area"

60.000 Pervious length"

2.000 Pervious slope"

0.000 Pervious Area"

0.000 Pervious length"

2.000 Pervious slope"

0.250 Pervious Manning 'n'"

74.000 Pervious SCS Curve No."

0.334 Pervious Runoff coefficient"

0.100 Pervious Ia/S coefficient"

8.924 Pervious Initial abstraction"

0.015 Pervious Manning 'n'"

98.000 Pervious SCS Curve No."

0.000 Pervious Runoff coefficient"

0.100 Pervious Ia/S coefficient"

0.518 Pervious Initial abstraction"

0.057 0.163 c.m/sec"

Catchment 40 Pervious

Surface Area 1.070 Total Area"

Time of concentration 27.501 27.501

Time to Centroid 251.214 251.214

Rainfall depth 65.300 65.300

Rainfall volume 698.71 698.71

Rainfall losses 43.512 43.512

Runoff depth 21.788 21.788

Runoff volume 233.13 233.13

Runoff coefficient 0.334 0.334

Maximum flow 0.057 0.057 c.m/sec"

HYDROGRAPH Add Runoff"

4 Add Runoff" 0.057 0.057 0.163 0.163"

HYDROGRAPH Copy to Outflow" 0.057 0.163 0.163"

8 Copy to Outflow" 0.057 0.057 0.163 0.163"

HYDROGRAPH " Combine 4"

6 Combine " Combine 4"

4 Node #"

Total to off-Site"

Maximum flow 0.057 0.057 c.m/sec"

Hydrograph volume 233.131 233.131

0.057 0.057 c.m"

HYDROGRAPH Start - New Tributary"

2 Start - New Tributary" 0.057 0.057 0.057"

CATCHMENT 41"

1 Triangular SCS"

2 Proportional to %"

1 SCS method"

41 - South west Flow off Site"

41 % Impervious"

185.000 Total Area"

185.000 Flow length"

4.000 Overland Slope"

3.180 Pervious Area"

185.000 Pervious length"

4.000 Pervious slope"

0.000 Pervious Area"

0.000 Pervious length"

Page 4

218173-PreDev-Oct03-25yr-SCS.out									
4.000	Impervious slope "n"								
0.150	Pervious Manning "n"								
74.000	Pervious SCS Curve No.								
0.334	Pervious Runoff coefficient								
0.100	Pervious Ia/S coefficient								
8.924	Pervious Initial abstraction"								
0.015	Impervious Manning "n"								
98.000	Impervious SCS Curve No.								
0.000	Impervious Runoff coefficient"								
0.100	Impervious Ia/S coefficient"								
0.518	Impervious Initial abstraction"								
	0.130	0.000	0.057						
	Catchment 41								
	Surface Area	3.180	Pervious						
	Time of Concentration	43.580							
	Time to Centroid	272.435							
	Rainfall depth	65.300							
	Rainfall volume	2076.54							
	Rainfall losses	43.490							
	Runoff depth	21.810							
	Runoff volume	693.54							
	Runoff coefficient	0.334							
	Maximum flow	0.130							
	HYDROGRAPH Add Runoff	0.130							
40	4 Add Runoff"	0.130	0.130	0.057					
	HYDROGRAPH Copy to Outflow"								
40	8 Copy to Outflow"	0.130	0.130	0.130					
	HYDROGRAPH " Combine	4"							
40	6 Combine								
	4 Node #"								
	Maximum flow								
	Total to Off-site"								
	Hydrograph volume	0.130	0.130	926.676					
	HYDROGRAPH Start - New Tributary"								
40	2 Start - New Tributary"	0.130	0.000	0.130					
	CATCHMENT 42"								
33	1 Triangular SCS"								
	2 Proportional to %"								
	1 SCS method"								
	42 % Impervious"								
	0.000								
	0.690								
	20.000								
	4.000								
	0.690								
	20.000								
	4.000								
	0.000								
	4.000								
	0.000								
	0.230								
	74.000								
	0.332								
	0.100								
	8.924								
	0.015								
	98.000								
	0.000								
	0.100								

218173-PreDev-Oct03-25Yr-SCS.out									
0.518	Impervious Initial abstraction"	0.054	0.000	0.130	0.182	c.m/sec"			
Catchment 42	Pervious				Impervious	Total Area			
Surface Area	0.690				0.000	0.690			hectare "
Time of Concentration	11.471				0.000	11.471			minutes"
Time to Centroid	230.606				191.157	230.606			minutes"
Rainfall depth	65.300				65.300	65.300			mm"
Rainfall volume	450.57				0.00	450.57			c.m"
Rainfall losses	43.628				11.319	43.628			mm"
Rainfall depth	21.672				53.981	21.672			mm"
Runoff volume	149.54				0.00	149.54			c.m"
Runoff coefficient	0.332				0.000	0.332			"
Maximum flow	0.054				0.000	0.054			c.m/sec"
HYDROGRAPH Add	Runoff "	0.054							
4	Add Runoff "	0.054	0.054	0.130	0.182"				
	HYDROGRAPH Copy to outflow"								
8	Copy to outflow"	0.054	0.054	0.054	0.182"				
	HYDROGRAPH " Combine	4"							
6	Combine "								
4	Node #"								
	Total " to off-site"								
	Maximum Flow			0.203	c.m/sec"				
	Hydrograph Volume	0.054	1076.216	0.203"					
	HYDROGRAPH Start - New Tributary"		0.054						
2	Start - New Tributary"								
	0.054			0.054	0.203"				
	CATCHMENT 50"								
1	Triangular SCS"								
2	Proportional to %"								
1	SCS method"								
50	50 - Flow to Golf Course"								
7.000	% Impervious"								
2.010	Total Area"								
45.000	Flow length"								
3.000	Overland Slope"								
1.869	Pervious Area"								
45.000	Pervious length"								
3.000	Pervious slope"								
0.141	Impervious Area"								
3.387	Impervious length"								
3.000	Impervious slope"								
0.250	Pervious Manning "n"								
4.000	Pervious SCS Curve No."								
0.333	Pervious Runoff coefficient"								
8.924	Pervious Ia/S coefficient"								
0.100	Pervious Initial abstraction"								
0.015	Impervious Manning "n"								
38.000	Impervious SCS Curve No."								
0.801	Impervious Runoff coefficient"								
0.100	Impervious Ia/S coefficient"								
0.518	Impervious Initial abstraction"			0.054	0.203	c.m/sec"			
	0.054	0.000			Impervious	Total Area			
Catchment 50	Pervious				0.141	2.010			hectare "
Surface Area	1.869				0.577	17.303			minutes"
Time of Concentration	20.342				192.125	234.450			minutes"
Time to Centroid	242.117				65.300	65.300			mm"
Rainfall depth	65.300				11.319	43.628			c.m"
Rainfall volume	450.57				13.009	41.433			mm"
Rainfall losses	43.628				52.291	23.867			
Rainfall depth	21.672								

218173-PreDev-Oct03-25yr-SCS.out

Runoff volume 406.16 73.57 479.73  
 Runoff coefficient 0.333 0.801 0.365  
 Maximum flow 0.122 0.034 0.127  
 HYDROGRAPH Add Runoff " 0.122 0.034 0.127  
 4 Add Runoff " 0.127 0.127 0.054 0.203"  
 HYDROGRAPH Copy to Outflow"  
 8 Copy to Outflow" 0.127 0.127 0.203"  
 HYDROGRAPH " Combine 3"  
 6 Combine " 3"  
 3 Node #  
 Total to Infiltration" 0.276  
 Maximum flow 1146.404  
 Hydrograph volume 0.127 0.127 0.276"  
 HYDROGRAPH Start - New Tributary"  
 2 Start - New Tributary" 0.127 0.127 0.276"  
 CATCHMENT 60"  
 1 Triangular SCS"  
 2 Proportional to %"  
 1 SCS method"  
 60 60-Infiltration on Golf Course"  
 0.000 % Impervious"  
 0.550 Total Area"  
 45.000 Flow Length"  
 3.000 Overland Slope"  
 0.550 Pervious Area"  
 45.000 Pervious Length"  
 3.000 Pervious Slope"  
 0.000 Pervious Area"  
 0.000 Impervious Length"  
 3.000 Impervious Slope"  
 0.250 Pervious Manning 'n'"  
 74.000 Pervious SCS Curve No."  
 0.333 Pervious Runoff coefficient"  
 0.100 Pervious Ia/S coefficient"  
 8.924 Pervious Initial abstraction"  
 0.015 Impervious Manning 'n'"  
 98.000 Impervious SCS Curve No."  
 0.000 Impervious Runoff coefficient"  
 0.100 Impervious Ia/S coefficient"  
 0.518 Impervious Initial abstraction"  
 0.036 0.000 0.127 0.276 c.m/sec"  
 Catchment 60 Pervious Total Area "  
 Surface Area 0.550 0.000 0.550 hectare"  
 Time of concentration 20.342 20.342 minutes"  
 Rainfall depth 242.117 242.117 mm"  
 Rainfall volume 65.300 65.300 c.m"  
 Rainfall losses 359.15 359.15 mm"  
 Runoff depth 43.572 43.572 mm"  
 Runoff volume 21.728 21.728 c.m"  
 Runoff coefficient 0.333 0.000 0.333  
 Maximum flow 0.036 0.000 0.036 c.m/sec"  
 HYDROGRAPH Add Runoff "  
 4 Add Runoff " 0.036 0.127 0.276"  
 HYDROGRAPH Copy to Outflow"  
 8 Copy to Outflow" 0.036 0.036 0.276"  
 HYDROGRAPH Combine 3" Page 7

218173-PreDev-Oct03-25yr-SCS.out

6 Combine " 21.320 hectare"  
 3 Node # 0.629 hectare"  
 Total to Infiltration" 2.948"  
 Maximum flow 0.312 c.m/sec"  
 Hydrograph volume 1265.906 c.m"  
 0.036 0.312  
 START/RE-START TOTALS 60"  
 3 Runoff Totals on EXIT"  
 Total Catchment area  
 Total Impervious area  
 Total % Impervious  
 EXIT"

```

218173-PreDev-Oct01-50yr-SCS.out
MIDUSS Output ----->
MIDUSS version Version 2.25 rev. 473"
MIDUSS created Sunday, February 07, 2010"
Units used: ie METRIC"
Job folder: C:\Users\jswiger\Desktop\wilder Lake"
Output filename: 218173-PreDev-Oct01-50yr-SCS.out"
Licensee name: GMBP"
Date & Time last used: Hewlett-Packard Company"
10/1/2019 at 11:38:59 AM"

TIME PARAMETERS
31 10.000 Time Step"
360.000 Max. Storm length"
2400.000 Max. Hydrograph"
32 3 Mass Curve"
71.900 Rainfall depth"
360.000 Duration"
47 C:\Program Files (x86)\MIDUSS\SCS_6hr_Type2.mrd SCS 6 hour Type

Maximum intensity 114.790 mm/hr"
Total depth 71.900 mm"
6 050hyd Hydrograph extension used in this file"
CATCHMENT 10"
1 Triangular SCS"
2 Proportional to %"
10- Wilder Lake South"
3.000 % Impervious"
5.260 Total Area"
100.000 Flow length"
5.000 Overland Slope"
5.102 Pervious Area"
100.000 Pervious length"
5.000 Pervious slope"
0.158 Impervious Area"
3.093 Impervious length"
5.000 Impervious slope"
0.250 Pervious Manning "n"
74.000 Pervious SCS Curve No."
0.361 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No."
0.799 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.349 0.000 0.000 0.000 c.m/sec"
Catchment 10 Pervious Total Area "
Surface Area 5.102 5.260 hectare"
Time of concentration 26.112 24.465 minutes"
Time to centroid 248.923 245.207 minutes"
Rainfall depth 71.900 71.900 mm"
Rainfall volume 3668.48 3781.94 c.m"
Rainfall losses 45.948 45.003 mm"
Runoff depth 25.952 26.897 mm"
Runoff volume 1324.13 1414.80 c.m"
Runoff coefficient 0.361 0.799 c.m/sec"
Maximum flow 0.343 0.374 c.m/sec"
4 HYDROGRAPH Add Runoff "
Add Runoff " 0.349 0.349 0.000 0.000"
Page 1

```

```

218173-PreDev-Oct01-50yr-SCS.out
HYDROGRAPH Copy to Outflow"
8 Copy to Outflow" 0.349 0.349 0.349 0.000"
HYDROGRAPH " Combine 1"
6 Combine "
1 Node #
Wilder Lake Final"
Maximum flow 0.349 c.m/sec"
Hydrograph volume 1414.804 c.m"
0.349 0.349 0.349"
HYDROGRAPH Start - New Tributary"
2 Start - New Tributary" 0.349 0.000 0.349 0.349"
CATCHMENT 20"
1 Triangular SCS"
2 Proportional to %"
1 SCS method"
20 20 - Creek"
6.000 % Impervious"
5.500 Total Area"
75.000 Flow length"
5.170 Overland Slope"
75.000 Pervious Area"
5.000 Pervious length"
5.000 Pervious slope"
0.230 Impervious Area"
4.787 Impervious length"
5.000 Impervious slope"
0.250 Pervious Manning "n"
74.000 Pervious SCS Curve No."
0.362 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No."
0.808 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.402 0.000 0.349 0.349 c.m/sec"
Catchment 20 Pervious Total Area "
Surface Area 5.170 5.500 hectare"
Time of concentration 21.972 19.298 minutes"
Time to centroid 243.363 236.915 minutes"
Rainfall depth 71.900 71.900 mm"
Rainfall volume 3717.23 3954.30 c.m"
Rainfall losses 45.895 45.836 mm"
Runoff depth 26.007 27.931 mm"
Runoff volume 1344.58 1536.19 c.m"
Runoff coefficient 0.362 0.808 c.m/sec"
Maximum flow 0.389 0.402 c.m/sec"
4 HYDROGRAPH Add Runoff "
Add Runoff " 0.402 0.402 0.349 0.349"
HYDROGRAPH Copy to Outflow"
8 Copy to Outflow" 0.402 0.402 0.402 0.349"
HYDROGRAPH " Combine 2"
6 Combine "
2 Node #
Total to Creek"
Maximum flow 0.402 c.m/sec"
Hydrograph volume 1536.187 c.m"
0.402 0.402 0.402"
Page 2

```

40	218173-PreDev-Oct01-50yr-SCS.out	HYDROGRAPH Start - New Tributary"	0.402	0.402"
40		2 Start - New Tributary"	0.402	0.402"
40		HYDROGRAPH Start - New Tributary"	0.402	0.402"
33		2 Start - New Tributary"	0.402	0.402"
		CATCHMENT 30"		
		1 Triangular SCS"		
		2 Proportional to %"		
		3 SCS method"		
		30 - Infiltration"		
		0.000 % Impervious"		
		3.060 Total Area"		
		100.000 Flow length"		
		4.000 Overland Slope"		
		3.060 Pervious Area"		
		100.000 Pervious length"		
		4.000 Pervious slope"		
		0.000 Pervious Manning "n"		
		0.000 Pervious SCS Curve No."		
		0.362 Pervious Runoff coefficient"		
		0.100 Pervious Ia/S coefficient"		
		8.924 Pervious Initial abstraction"		
		0.015 Pervious Manning "n"		
		98.000 Pervious SCS Curve No."		
		0.000 Pervious Runoff coefficient"		
		0.100 Pervious Ia/S coefficient"		
		0.518 Pervious Initial abstraction"		
		Catchment 30	0.197	0.402
		Surface Area	3.060	0.000
		Time to concentration	27.920	0.001
		Rainfall depth	251.247	190.669
		Rainfall volume	71.900	71.900
		Rainfall losses	2200.14	0.00
		Runoff depth	45.896	12.015
		Runoff volume	26.004	59.885
		Runoff coefficient	795.73	0.00
		Maximum flow	0.362	0.000
		HYDROGRAPH Add Runoff "	0.197	0.000
40		4 Add Runoff "	0.197	0.402
40		HYDROGRAPH Copy to Outflow"	0.197	0.402"
40		8 Copy to Outflow"	0.197	0.402"
40		6 Combine " Combine 3"	0.197	0.402"
40		3 Node #"		
		Total to Infiltration"		
		Maximum flow	0.197	0.197
		Hydrograph volume	795.730	0.197
		HYDROGRAPH Start - New Tributary"	0.197	0.197"
40		2 Start - New Tributary"	0.197	0.197"
33		CATCHMENT 40"		
		1 Triangular SCS"		
		2 Proportional to %"		
		1 SCS method"		

40	218173-PreDev-Oct01-50yr-SCS.out	40 - South East Flow Off-Site"		
40		% Impervious"		
40		1.070 Total Area"		
40		60.000 Flow length"		
40		2.000 Overland Slope"		
40		1.070 Pervious Area"		
40		60.000 Pervious length"		
40		2.000 Pervious slope"		
40		0.000 Pervious Manning "n"		
40		0.250 Pervious SCS Curve No."		
40		0.361 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		8.924 Pervious Initial abstraction"		
40		0.015 Pervious Manning "n"		
40		98.000 Pervious SCS Curve No."		
40		0.000 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		0.518 Pervious Initial abstraction"		
40		Catchment 40	0.074	0.197
40		Surface Area	1.070	0.000
40		Time to concentration	25.300	0.001
40		Rainfall depth	247.798	190.669
40		Rainfall volume	71.900	71.900
40		Rainfall losses	45.937	0.00
40		Runoff depth	25.963	12.014
40		Runoff volume	277.81	59.886
40		Runoff coefficient	0.361	0.000
40		Maximum flow	0.074	0.000
40		HYDROGRAPH Add Runoff "	0.074	0.197
40		4 Add Runoff "	0.074	0.197
40		HYDROGRAPH Copy to Outflow"	0.074	0.197
40		8 Copy to Outflow"	0.074	0.197
40		6 Combine " Combine 4"	0.074	0.197
40		4 Node #"		
40		Total to Off-Site"		
40		Maximum flow	0.074	0.074
40		Hydrograph volume	277.807	0.074
40		HYDROGRAPH Start - New Tributary"	0.074	0.074"
40		2 Start - New Tributary"	0.074	0.074"
40		CATCHMENT 41"		
40		1 Triangular SCS"		
40		2 Proportional to %"		
40		1 SCS method"		
40		41 - South West Flow Off Site"		
40		0.000 % Impervious"		
40		3.180 Total Area"		
40		185.000 Flow length"		
40		4.000 Overland Slope"		
40		3.180 Pervious Area"		
40		185.000 Pervious length"		
40		4.000 Pervious slope"		
40		0.000 Pervious Manning "n"		
40		0.000 Pervious SCS Curve No."		
40		0.361 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		8.924 Pervious Initial abstraction"		
40		0.015 Pervious Manning "n"		
40		98.000 Pervious SCS Curve No."		
40		0.000 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		0.518 Pervious Initial abstraction"		
40		Catchment 40	0.074	0.197
40		Surface Area	1.070	0.000
40		Time to concentration	25.300	0.001
40		Rainfall depth	247.798	190.669
40		Rainfall volume	71.900	71.900
40		Rainfall losses	45.937	0.00
40		Runoff depth	25.963	12.014
40		Runoff volume	277.81	59.886
40		Runoff coefficient	0.361	0.000
40		Maximum flow	0.074	0.000
40		HYDROGRAPH Add Runoff "	0.074	0.197
40		4 Add Runoff "	0.074	0.197
40		HYDROGRAPH Copy to Outflow"	0.074	0.197
40		8 Copy to Outflow"	0.074	0.197
40		6 Combine " Combine 4"	0.074	0.197
40		4 Node #"		
40		Total to Off-Site"		
40		Maximum flow	0.074	0.074
40		Hydrograph volume	277.807	0.074
40		HYDROGRAPH Start - New Tributary"	0.074	0.074"
40		2 Start - New Tributary"	0.074	0.074"
40		CATCHMENT 41"		
40		1 Triangular SCS"		
40		2 Proportional to %"		
40		1 SCS method"		
40		41 - South West Flow Off Site"		
40		0.000 % Impervious"		
40		3.180 Total Area"		
40		185.000 Flow length"		
40		4.000 Overland Slope"		
40		3.180 Pervious Area"		
40		185.000 Pervious length"		
40		4.000 Pervious slope"		
40		0.000 Pervious Manning "n"		
40		0.000 Pervious SCS Curve No."		
40		0.361 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		8.924 Pervious Initial abstraction"		
40		0.015 Pervious Manning "n"		
40		98.000 Pervious SCS Curve No."		
40		0.000 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		0.518 Pervious Initial abstraction"		
40		Catchment 40	0.074	0.197
40		Surface Area	1.070	0.000
40		Time to concentration	25.300	0.001
40		Rainfall depth	247.798	190.669
40		Rainfall volume	71.900	71.900
40		Rainfall losses	45.937	0.00
40		Runoff depth	25.963	12.014
40		Runoff volume	277.81	59.886
40		Runoff coefficient	0.361	0.000
40		Maximum flow	0.074	0.000
40		HYDROGRAPH Add Runoff "	0.074	0.197
40		4 Add Runoff "	0.074	0.197
40		HYDROGRAPH Copy to Outflow"	0.074	0.197
40		8 Copy to Outflow"	0.074	0.197
40		6 Combine " Combine 4"	0.074	0.197
40		4 Node #"		
40		Total to Off-Site"		
40		Maximum flow	0.074	0.074
40		Hydrograph volume	277.807	0.074
40		HYDROGRAPH Start - New Tributary"	0.074	0.074"
40		2 Start - New Tributary"	0.074	0.074"
40		CATCHMENT 41"		
40		1 Triangular SCS"		
40		2 Proportional to %"		
40		1 SCS method"		
40		41 - South West Flow Off Site"		
40		0.000 % Impervious"		
40		3.180 Total Area"		
40		185.000 Flow length"		
40		4.000 Overland Slope"		
40		3.180 Pervious Area"		
40		185.000 Pervious length"		
40		4.000 Pervious slope"		
40		0.000 Pervious Manning "n"		
40		0.000 Pervious SCS Curve No."		
40		0.361 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		8.924 Pervious Initial abstraction"		
40		0.015 Pervious Manning "n"		
40		98.000 Pervious SCS Curve No."		
40		0.000 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		0.518 Pervious Initial abstraction"		
40		Catchment 40	0.074	0.197
40		Surface Area	1.070	0.000
40		Time to concentration	25.300	0.001
40		Rainfall depth	247.798	190.669
40		Rainfall volume	71.900	71.900
40		Rainfall losses	45.937	0.00
40		Runoff depth	25.963	12.014
40		Runoff volume	277.81	59.886
40		Runoff coefficient	0.361	0.000
40		Maximum flow	0.074	0.000
40		HYDROGRAPH Add Runoff "	0.074	0.197
40		4 Add Runoff "	0.074	0.197
40		HYDROGRAPH Copy to Outflow"	0.074	0.197
40		8 Copy to Outflow"	0.074	0.197
40		6 Combine " Combine 4"	0.074	0.197
40		4 Node #"		
40		Total to Off-Site"		
40		Maximum flow	0.074	0.074
40		Hydrograph volume	277.807	0.074
40		HYDROGRAPH Start - New Tributary"	0.074	0.074"
40		2 Start - New Tributary"	0.074	0.074"
40		CATCHMENT 41"		
40		1 Triangular SCS"		
40		2 Proportional to %"		
40		1 SCS method"		
40		41 - South West Flow Off Site"		
40		0.000 % Impervious"		
40		3.180 Total Area"		
40		185.000 Flow length"		
40		4.000 Overland Slope"		
40		3.180 Pervious Area"		
40		185.000 Pervious length"		
40		4.000 Pervious slope"		
40		0.000 Pervious Manning "n"		
40		0.000 Pervious SCS Curve No."		
40		0.361 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		8.924 Pervious Initial abstraction"		
40		0.015 Pervious Manning "n"		
40		98.000 Pervious SCS Curve No."		
40		0.000 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		0.518 Pervious Initial abstraction"		
40		Catchment 40	0.074	0.197
40		Surface Area	1.070	0.000
40		Time to concentration	25.300	0.001
40		Rainfall depth	247.798	190.669
40		Rainfall volume	71.900	71.900
40		Rainfall losses	45.937	0.00
40		Runoff depth	25.963	12.014
40		Runoff volume	277.81	59.886
40		Runoff coefficient	0.361	0.000
40		Maximum flow	0.074	0.000
40		HYDROGRAPH Add Runoff "	0.074	0.197
40		4 Add Runoff "	0.074	0.197
40		HYDROGRAPH Copy to Outflow"	0.074	0.197
40		8 Copy to Outflow"	0.074	0.197
40		6 Combine " Combine 4"	0.074	0.197
40		4 Node #"		
40		Total to Off-Site"		
40		Maximum flow	0.074	0.074
40		Hydrograph volume	277.807	0.074
40		HYDROGRAPH Start - New Tributary"	0.074	0.074"
40		2 Start - New Tributary"	0.074	0.074"
40		CATCHMENT 41"		
40		1 Triangular SCS"		
40		2 Proportional to %"		
40		1 SCS method"		
40		41 - South West Flow Off Site"		
40		0.000 % Impervious"		
40		3.180 Total Area"		
40		185.000 Flow length"		
40		4.000 Overland Slope"		
40		3.180 Pervious Area"		
40		185.000 Pervious length"		
40		4.000 Pervious slope"		
40		0.000 Pervious Manning "n"		
40		0.000 Pervious SCS Curve No."		
40		0.361 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		8.924 Pervious Initial abstraction"		
40		0.015 Pervious Manning "n"		
40		98.000 Pervious SCS Curve No."		
40		0.000 Pervious Runoff coefficient"		
40		0.100 Pervious Ia/S coefficient"		
40		0.518 Pervious Initial abstraction"		
40		Catchment 40	0.074	0.197
40		Surface Area	1.070	0.000
40		Time to concentration	25.300	0.001
40		Rainfall depth	247.798	190.669
40		Rainfall volume	71.900	71.900
40		Rainfall losses	45.937	0.00
40		Runoff depth	25.963	12.014
40		Runoff volume	277.81	59.886

218173-prdev-oct01-50yr-SCS.out			
4.000	Impervious slope "n"		0.074
0.250	Pervious Manning "n"		0.000
74.000	Pervious SCS Curve No.		0.001
0.362	Pervious Runoff coefficient		190.669
0.100	Pervious Ia/S coefficient		71.900
8.924	Pervious Initial abstraction		0.00
0.015	Impervious Manning "n"		12.019
98.000	Impervious SCS Curve No.		59.881
0.000	Impervious Runoff coefficient		0.00
0.100	Impervious Ia/S coefficient		0.00
0.518	Impervious Initial abstraction		0.000
	Catchment 41	Pervious	0.074
	Surface Area	3.180	0.000
	Time of Concentration	40.385	0.001
	Time to Centroid	267.660	190.669
	rainfall depth	71.900	71.900
	rainfall volume	2286.42	0.00
	rainfall losses	45.885	12.019
	runoff depth	26.015	59.881
	runoff volume	827.27	0.00
	Runoff coefficient	0.362	0.00
	Maximum flow	0.162	0.000
	HYDROGRAPH Add Runoff	0.162	0.000
4	Add Runoff		
	HYDROGRAPH Copy to Outflow	0.162	0.074
40	8 Copy to outflow	0.162	0.074
	HYDROGRAPH " Combine	4	
40	6 Combine		
4	Node #		
	Total to Off-Site"		
	Maximum flow		
	Hydrograph volume	0.231	c.m/s
	0.162	0.162	c.m"
40	HYDROGRAPH Start - New Tributary"	1105.077	0.231"
2	Start - New Tributary"		
	0.162	0.000	0.231"
33	CATCHMENT 42"		
1	Triangular SCS"		
2	Proportional to %"		
1	SCS method		
42	42 - West Flow Off Site"		
	% Impervious"		
0.000	Total Area"		
0.690	Flow Length"		
20.000	Overland Slope"		
0.690	Pervious Area"		
20.000	Pervious length"		
4.000	Pervious slope"		
0.000	Impervious Area"		
0.000	Impervious length"		
4.000	Impervious slope"		
0.250	Pervious Manning "n"		
74.000	Pervious SCS Curve No.		
0.362	Pervious Runoff coefficient		
0.100	Pervious Ia/S coefficient		
8.924	Pervious Initial abstraction		
0.015	Impervious Manning "n"		
98.000	Impervious SCS Curve No.		
0.000	Impervious Runoff coefficient		
0.100	Impervious Ia/S coefficient		

Page 5

[illegible]



218173-PreDev-Oct01-50yr-SCS-out									
Runoff volume	484.32	81.47	565.79						
Runoff coefficient	0.360	0.805	0.391						
Maximum Flow									
HYDROGRAPH Add Runoff	0.151	0.038	0.157						
4 Add Runoff	0.157	0.157	0.068	0.261"					
HYDROGRAPH Copy to Outflow									
8 Copy to Outflow	0.157	0.157	0.157	0.261"					
HYDROGRAPH Combine	3"								
6 Combine									
3 Node #									
Total to Infiltration		0.351							
Maximum flow		1361.521							
Hydrograph volume	0.157	0.157	0.157	0.351"					
HYDROGRAPH Start - New Tributary									
2 Start - New Tributary	0.157	0.000	0.157	0.351"					
CATCHMENT 60"									
1 Triangular SCS									
2 Proportional to %									
1 SCS method									
60 60-Infiltration on Golf Course"									
% Impervious									
0.000									
0.550									
45.000									
3.000									
0.550									
45.000									
3.000									
0.000									
0.000									
0.000									
0.000									
0.250									
74.000									
0.360									
0.100									
8.924									
0.015									
98.000									
0.000									
0.100									
0.000									
0.518									
0.045		0.000	0.157						
Catchment 60									
Surface Area		0.550							
Time of concentration		18.850	0.001						
Time to Centroid		239.289	190.670						
Rainfall depth		71.900	71.900						
Rainfall volume		395.45	0.00						
Rainfall losses		45.991	12.012						
Rainfall depth		25.909	59.888						
Runoff volume		142.50	0.000						
Runoff coefficient		0.360	0.000						
Maximum Flow		0.045	0.000						
HYDROGRAPH Add Runoff		3"							
4 Add Runoff	0.045	0.045	0.157	0.351"					
HYDROGRAPH Copy to Outflow									
8 Copy to Outflow	0.045	0.045	0.045	0.351"					

```

38 6  Combine " 218173-Pradev-Oct01-50yr-SCS.out
39 3  Node #"
40      Total to Infiltration" 0.395 c.m./sec"
41      Maximum flow 1504.023 c.m."
42      Hydrograph volume 0.045 0.395"
43 38 START/RE-START TOTALS 60"
44 3  Runoff Totals on EXIT"
45      Total Catchment area 21.320 hectare"
46      Total Impervious area 0.629 hectare"
47      Total % impervious 2.948"
48 EXIT"
49 19

```

218173-FredEv-Oct03-100yr-SCS.out

HYDROGRAPH Copy to Outflow"

8 Copy to Outflow" 0.429 0.429 0.429 0.000"

HYDROGRAPH " Combine 1" 0.429 0.000"

1 Node #"

Wilder Lake Final" 0.429 c.m/sec"

Maximum flow 1646.875 c.m"

Hydrograph volume 0.429 0.429 0.429"

2 START - New Tributary"

HYDROGRAPH Start - New Tributary" 0.429 0.429"

2 START - New Tributary" 0.000 0.429 0.429"

CATCHMENT 20"

1 Triangular SCS"

2 Proportional to %"

1 SCS method" 20

20 Creek" 20

% Impervious" 6.000

Total Area" 5.000

Flow length" 75.000

Overland Slope" 3.000

Pervious Area" 3.170

Pervious length" 75.000

Pervious slope" 5.000

Impervious Area" 0.330

Impervious length" 4.787

Impervious slope" 5.000

Pervious Manning "n" 0.250

Pervious SCS Curve No. 74.000

Pervious Runoff coefficient" 0.386

Pervious Ia/S coefficient" 0.100

Pervious Initial abstraction" 8.924

Impervious Manning "n" 0.015

Impervious SCS Curve No. 98.000

Impervious Runoff coefficient" 0.811

Impervious Ia/S coefficient" 0.100

Impervious Initial abstraction" 0.518

Surface Area" 0.487 0.000 0.429 0.429 c.m/sec"

Catchment 20" Impervious Total Area "

5.170 5.500

Time of concentration 20.569 0.516

Time to Centroid 240.810 191.126

Rainfall depth 78.400 234.939

Rainfall volume 78.400 78.400

Rainfall losses 40.632 28

Rainfall losses 48.102 4312.00

Runoff depth 30.298 11.802

Runoff volume 1566.43 46.104

Runoff coefficient 0.386 32.298

Maximum flow 1776.30 c.m"

Hydrograph Add Runoff " 0.473 0.097 0.487 c.m/sec"

4 Add Runoff" 0.487 0.429 0.429"

HYDROGRAPH Copy to Outflow"

8 Copy to Outflow" 0.487 0.487 0.487 0.429"

HYDROGRAPH " Combine 2" 0.487 0.429"

6 Combine " 0.487 0.429"

2 Node #"

Total to Creek" 0.487 c.m/sec"

Maximum flow 1776.303 c.m"

Hydrograph volume 0.487 0.487 0.487 0.487"

Page 2

40	HYDROGRAPH Start - New Tributary"	218173-PreDev-Oct03-100yr-SCS.out	40	40 - South East Flow Off-Site	218173-PreDev-Oct03-100yr-SCS.out
2	Start - New Tributary"	0.487	2	Start - New Tributary"	0.487
40	HYDROGRAPH Start - New Tributary"	0.487	40	HYDROGRAPH Start - New Tributary"	0.487
2	Start - New Tributary"	0.487	2	Start - New Tributary"	0.487
33	CATCHMENT 30"		33	CATCHMENT 40"	
1	Triangular SCS"		1	Triangular SCS"	
2	Proportional to %"		2	Proportional to %"	
30	SCS method"		30	SCS method"	
30	% Impervious"		30	% Impervious"	
0.000	Total Area"		0.000	Total Area"	
100.000	Flow length"		100.000	Flow length"	
4.000	Overland Slope"		4.000	Overland Slope"	
3.060	Pervious Area"		3.060	Pervious Area"	
100.000	Pervious length"		100.000	Pervious length"	
4.000	Pervious slope"		4.000	Pervious slope"	
0.000	Impervious Area"		0.000	Impervious Area"	
0.000	Impervious length"		0.000	Impervious length"	
4.000	Impervious slope"		4.000	Impervious slope"	
0.250	Pervious Manning 'n'"		0.250	Pervious Manning 'n'"	
74.000	Pervious SCS Curve No."		74.000	Pervious SCS Curve No."	
0.386	Pervious Runoff coefficient"		0.386	Pervious Runoff coefficient"	
0.100	Pervious Ia/S coefficient"		0.100	Pervious Ia/S coefficient"	
8.924	Pervious Initial abstraction"		8.924	Pervious Initial abstraction"	
0.015	Impervious Manning 'n'"		0.015	Impervious Manning 'n'"	
98.000	Impervious SCS Curve No."		98.000	Impervious SCS Curve No."	
0.000	Impervious Runoff coefficient"		0.000	Impervious Runoff coefficient"	
0.100	Impervious Ia/S coefficient"		0.100	Impervious Ia/S coefficient"	
0.518	Impervious Initial abstraction"		0.518	Impervious Initial abstraction"	
Catchment 30			Catchment 40		
Surface Area	3.060	0.000	Surface Area	1.070	0.000
Time to concentration	26.137	0.001	Time of concentration	23.684	0.001
Time to centroid	248.275	190.249	Time to centroid	248.275	190.249
Rainfall depth	78.400	78.400	Rainfall depth	78.400	78.400
Rainfall volume	2399.04	0.00	Rainfall volume	838.88	0.00
Rainfall losses	48.108	12.690	Rainfall losses	48.079	12.689
Runoff depth	30.292	65.710	Runoff depth	30.321	65.711
Runoff volume	926.92	0.00	Runoff volume	324.43	0.00
Runoff coefficient	0.386	0.000	Runoff coefficient	0.387	0.000
Maximum flow	0.241	0.000	Maximum flow	0.090	0.000
HYDROGRAPH Add Runoff "			HYDROGRAPH Add Runoff "		
4	Add Runoff "	0.487	4	Add Runoff "	0.090
HYDROGRAPH Copy to Outflow"			HYDROGRAPH Copy to Outflow"		
8	Copy to Outflow"	0.241	8	Copy to Outflow"	0.241
HYDROGRAPH Combine 3"			HYDROGRAPH Combine 4"		
6	Combine " Combine 3"	0.241	6	Combine " Combine 4"	0.090
Node #"			Node #"		
Total to Infiltration"			Total to Off-site"		
Maximum flow	0.241	0.487	Maximum flow	0.090	0.090
Hydrograph volume	926.923	0.241	Hydrograph volume	324.434	0.090
HYDROGRAPH Start - New Tributary"			HYDROGRAPH Start - New Tributary"		
2	Start - New Tributary"	0.241	2	Start - New Tributary"	0.090
HYDROGRAPH Combine 3"			HYDROGRAPH Combine 4"		
6	Combine " Combine 3"	0.241	6	Combine " Combine 4"	0.090
Node #"			Node #"		
Total to Infiltration"			Total to Off-site"		
Maximum flow	0.241	0.487	Maximum flow	0.090	0.090
Hydrograph volume	926.923	0.241	Hydrograph volume	324.434	0.090
HYDROGRAPH Start - New Tributary"			HYDROGRAPH Start - New Tributary"		
2	Start - New Tributary"	0.241	2	Start - New Tributary"	0.090
HYDROGRAPH Combine 3"			HYDROGRAPH Combine 4"		
6	Combine " Combine 3"	0.241	6	Combine " Combine 4"	0.090
Node #"			Node #"		
Total to Infiltration"			Total to Off-site"		
Maximum flow	0.241	0.487	Maximum flow	0.090	0.090
Hydrograph volume	926.923	0.241	Hydrograph volume	324.434	0.090
HYDROGRAPH Start - New Tributary"			HYDROGRAPH Start - New Tributary"		
2	Start - New Tributary"	0.241	2	Start - New Tributary"	0.090
HYDROGRAPH Combine 3"			HYDROGRAPH Combine 4"		
6	Combine " Combine 3"	0.241	6	Combine " Combine 4"	0.090
Node #"			Node #"		
Total to Infiltration"			Total to Off-site"		
Maximum flow	0.241	0.487	Maximum flow	0.090	0.090
Hydrograph volume	926.923	0.241	Hydrograph volume	324.434	0.090
HYDROGRAPH Start - New Tributary"			HYDROGRAPH Start - New Tributary"		
2	Start - New Tributary"	0.241	2	Start - New Tributary"	0.090</

218173-Predev-Oct03-100yr-SCS.out		
40	40 - South East Flow Off-Site	
0.000	% Impervious	0.241
1.070	Total Area	Impervious
60.000	Flow length	0.000
2.000	Overland Slope	0.001
1.070	Pervious Area	190.249
60.000	Pervious length	78.400
2.000	Pervious Slope	0.000
0.000	Impervious Area	12.689
0.000	Impervious length	65.711
2.000	Impervious slope	0.000
0.250	Pervious Manning "n"	0.000
74.000	Pervious SCS Curve No. "	0.241
0.387	Pervious Runoff coefficient"	Impervious
0.100	Pervious Ia/S coefficient"	0.000
8.924	Pervious Initial abstraction"	0.001
0.015	Impervious Manning "n"	190.249
98.000	Impervious SCS Curve No. "	78.400
0.000	Impervious Runoff coefficient"	0.000
0.100	Impervious Ia/S coefficient"	12.689
0.518	Impervious Initial abstraction"	65.711
	Catchment 40	0.000
	Surface Area	0.241
	Time of Concentration	Impervious
	Time to Centroid	1.070
	Rainfall depth	23.684
	Rainfall volume	244.932
	Rainfall losses	28.400
	Rainfall depth	858.88
	Runoff volume	48.979
	Runoff coefficient	30.321
	Maximum Flow	324.43
	Maximum Flow	0.387
	Hydrograph Add Runoff "	0.090
40	4 Add Runoff "	0.090
	HYDROGRAPH Copy to Outflow"	0.090
40	8 Copy to Outflow"	0.241
	HYDROGRAPH Combine 4"	0.090
40	6 Combine " Combine 4"	0.241
	4 Node #"	
	Total to Off-Site"	
	Maximum Flow	0.090
	Hydrograph volume	324.434
	HYDROGRAPH Start - New Tributary"	0.090
40	2 Start - New Tributary"	0.090
	0.090	0.000
33	CATCHMENT 41"	0.090
	1 Triangular SCS"	
	2 proportional to %"	
	1 SCS method	
	41 -South West Flow Off Site"	
	% Impervious	
	Total Area	
	Flow length	
	Overland Slope	
	Pervious Area	
	Pervious length	
	Pervious Slope	
	Pervious Area	
	Pervious length	
	Pervious Slope	
	Impervious Area	
	Impervious length"	

Page 4



```

218173-PreDev-Oct03-100yr-SCS.out
Runoff volume 565.24 89.23 654.47
Runoff coefficient 0.386 0.809 0.415
Maximum flow 0.187 0.042 0.187
HYDROGRAPH Add Runoff " 0.187
4 Add Runoff " 0.187 0.187 0.082 0.323"
HYDROGRAPH Copy to Outflow"
8 Copy to Outflow" 0.187 0.187 0.323"
HYDROGRAPH " Combine 3"
6 Combine " Node #
3 Node #
Total to Infiltration"
Maximum flow 0.429
Hydrograph volume 1581.392
Hydrograph volume 0.187 0.187 0.429"
HYDROGRAPH Start - New Tributary"
2 Start - New Tributary" 0.187 0.187 0.429"
CATCHMENT 60"
1 Triangular SCS"
2 Proportional to %"
SCS method"
60 60-Infiltration on Golf Course"
0.000 % Impervious"
0.550 Total Area"
45.000 Flow length"
3.000 Overland Slope"
0.550 Pervious Area"
45.000 Pervious length"
3.000 Pervious slope"
0.000 Impervious Area"
0.000 Impervious length"
3.000 Impervious slope"
0.250 Pervious Manning "n"
74.000 Pervious SCS Curve No. "
0.386 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning "n"
98.000 Impervious SCS Curve No. "
0.000 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
Catchment 60" Pervious 0.053 0.000 0.187 0.429 c.m/sec"
Surface Area 0.550 Total Area"
Time of concentration 17.646 0.001 17.646
Rainfall depth 236.922 190.249 236.922
Rainfall volume 78.400 78.400 78.400
Rainfall losses 431.20 0.00 431.20
Rainfall depth 48.162 12.687 48.162
Runoff volume 30.238 65.713 30.238
Runoff coefficient 166.31 0.00 166.31
Maximum flow 0.386 0.000 0.386
HYDROGRAPH Add Runoff "
4 Add Runoff " 0.053 0.053 0.187 0.429"
HYDROGRAPH Copy to Outflow"
8 Copy to Outflow" 0.053 0.053 0.429"
HYDROGRAPH " Combine 3"

```

```

218173-PreDev-Oct03-100yr-SCS.out
6 Combine "
3 Node #
Total to Infiltration"
Maximum flow 0.482
Hydrograph volume 1747.702
Hydrograph volume 0.053 0.053
START/RE-START TOTALS 60"
3 Runoff Totals on EXIT"
Total Catchment area 21.320
Total Impervious area 0.629
Total % Impervious 2.948"
EXIT"

```





218173-PreDev-Oct01-Hazel-SCS-2.out									
40	Maximum flow	0.737	0.737	0.737	0.737	0.737	0.737	0.737	c.m./sec"
	Hydrograph volume	10998.661							c.m./sec"
	HYDROGRAPH Start - New Tributary	0.737	0.737	0.737	0.737	0.737	0.737	0.737	c.m./sec"
33	2 Start - New Tributary	0.000	0.737	0.737	0.737	0.737	0.737	0.737	c.m./sec"
	CATCHMENT 20								
	1 Triangular SCS								
	2 Proportional to %								
	1 SCS method								
	20 - Creek								
	% Impervious								
	6.000 Total Area								
	75.000 Flow length								
	5.000 Overland Slope								
	5.170 Pervious Area								
	75.000 Pervious length								
	5.000 Pervious slope								
	0.330 Impervious Area								
	4.787 Impervious length								
	5.000 Impervious slope								
	0.250 Pervious Manning "n"								
	74.000 Pervious SCS Curve No.								
	0.730 Pervious Runoff coefficient								
	0.100 Pervious Ia/S coefficient								
	8.924 Pervious Initial abstraction								
	0.015 Pervious Manning "n"								
	98.000 Impervious SCS Curve No.								
	0.730 Pervious Runoff coefficient								
	0.100 Pervious Ia/S coefficient								
	0.518 Impervious Initial abstraction								
	0.754 Pervious								
	Catchment 20								
	Surface Area	5.170							0.737 c.m./sec"
	Time to Centroid	21.079							0.737 c.m./sec"
	Rainfall depth	2518.236							0.737 c.m./sec"
	Rainfall volume	284.997							0.737 c.m./sec"
	Rainfall losses	1.4734							0.737 c.m./sec"
	Runoff depth	76.930							0.737 c.m./sec"
	Runoff volume	208.067							0.737 c.m./sec"
	Runoff coefficient	1.0757							0.737 c.m./sec"
	Maximum flow	0.730							0.737 c.m./sec"
	HYDROGRAPH Add Runoff	0.713							0.737 c.m./sec"
40	4 Add Runoff	0.754	0.754	0.737	0.737	0.737	0.737	0.737	c.m./sec"
	HYDROGRAPH Copy to Outflow								
40	8 Copy to Outflow	0.754	0.754	0.754	0.754	0.754	0.754	0.754	c.m./sec"
40	6 Combine	2							
	2 Node #								
	Total to Creek								
	Maximum flow	0.754							0.754 c.m./sec"
	Hydrograph volume	11567.516							0.754 c.m./sec"
40	HYDROGRAPH Start - New Tributary	0.754	0.754	0.754	0.754	0.754	0.754	0.754	c.m./sec"
40	2 Start - New Tributary	0.000	0.754	0.754	0.754	0.754	0.754	0.754	c.m./sec"
	HYDROGRAPH Start - New Tributary								
33	2 Start - New Tributary	0.000	0.754	0.754	0.754	0.754	0.754	0.754	c.m./sec"
	CATCHMENT 30								

218173-PreDev-Oct01-Hazel-SCS-2.out									
1	Triangular SCS								
2	Proportional to %								
1	SCS method								
30	30 - Infiltration								
30	% Impervious								
3.060	Total Area								
100.000	Flow length								
4.000	Overland Slope								
3.060	Pervious Area								
100.000	Pervious length								
4.000	Pervious slope								
0.000	Impervious Area								
0.000	Impervious length								
4.000	Impervious slope								
0.250	Pervious Manning "n"								
74.000	Pervious SCS Curve No.								
0.730	Pervious Runoff coefficient								
0.100	Pervious Ia/S coefficient								
8.924	Pervious Initial abstraction								
0.015	Impervious Manning "n"								
98.000	Impervious SCS Curve No.								
0.730	Impervious Runoff coefficient								
0.100	Impervious Ia/S coefficient								
0.518	Impervious Initial abstraction								
0.429	Pervious								0.754 c.m./sec"
Catchment 30									
Surface Area	3.060								0.000
Time to Centroid	26.785								0.001
Rainfall depth	2526.282								2268.083
Rainfall volume	284.997								284.997
Rainfall losses	8720.91								0.01
Runoff depth	76.985								33.597
Runoff volume	208.012								251.400
Runoff coefficient	6365.16								0.01
Maximum flow	0.730								0.000
HYDROGRAPH Add Runoff	0.429								0.000
4 Add Runoff	0.429	0.429	0.754	0.754	0.754	0.754	0.754	0.754	c.m./sec"
HYDROGRAPH Copy to Outflow									
8 Copy to Outflow	0.429	0.429	0.429	0.429	0.429	0.429	0.429	0.429	c.m./sec"
HYDROGRAPH	Combine	3							
3 Node #									
Total to Infiltration									
Maximum flow	0.429								0.429 c.m./sec"
Hydrograph volume	6365.167								6365.167 c.m./sec"
HYDROGRAPH Start - New Tributary	0.429	0.429	0.429	0.429	0.429	0.429	0.429	0.429	c.m./sec"
2 Start - New Tributary	0.000	0.429	0.429	0.429	0.429	0.429	0.429	0.429	c.m./sec"
CATCHMENT 40									
1 Triangular SCS									
2 Proportional to %									
1 SCS method									
40	40 - South East Flow off-Site								
0.000	% Impervious								
1.070	Total Area								
60.000	Flow length								
2.000	Overland Slope								
1.070	Pervious Area								
60.000	Pervious length								

[illegible]

218173-PreDev-Oct01-Hazel-SCS-2.0ut									
98.000	Impervious SCS Curve No.								
0.000	Impervious Runoff coefficient"								
0.100	Impervious Ia/S coefficient"								
0.518	Impervious Initial abstraction"	0.445	0.000	0.150	Impervious	0.150	Impervious		
	Catchment 41								
	Surface Area		38.743						
	Time of concentration		2543.007						
	Time to Centroid								
	Rainfall depth								
	Rainfall volume		9062.91						
	Rainfall losses		76.829						
	Runoff depth		208.168						
	Runoff volume		6619.73						
	Runoff coefficient		0.730						
	Maximum flow		0.445						
	HYDROGRAPH Add Runoff "								
40	4 Add Runoff "	0.445	0.445	0.150		0.150			
	HYDROGRAPH Copy to Outflow"								
40	8 Copy to Outflow"	0.445	0.445	0.150		0.150			
	HYDROGRAPH " Combine 4"								
40	6 Combine								
	4 Node #								
	Total to off-site"								
	Maximum flow								
	Hydrograph volume	0.445	0.445	0.584	c.m./sec	0.584	c.m./sec		
	HYDROGRAPH Start - New Tributary"								
40	2 Start - New Tributary"	0.445	0.000	0.445		0.584			
	CATCHMENT 42"								
33	1 Triangular SCS"								
	2 Proportional to %"								
	42 SCS method"								
	42 - West Flow Off Site"								
	% Impervious"								
	Total Area"								
	Flow length"								
	Overland Slope"								
	Pervious Area"								
	Pervious length"								
	Pervious slope"								
	Impervious Area"								
	Impervious length"								
	Impervious slope"								
	Pervious Manning "n"								
	Pervious SCS Curve No."								
	Pervious Runoff coefficient"								
	Pervious Ia/S coefficient"								
	Pervious initial abstraction"								
	Impervious Manning "n"								
	Impervious SCS Curve No."								
	Impervious Runoff coefficient"								
	Impervious Ia/S coefficient"								
	Impervious Initial abstraction"	0.094	0.000	0.445		0.584	c.m./sec		
	Surface Area								
	Time of concentration								
	Time to Centroid								
	Rainfall depth								

Page 6

[illegible]

218173-PreDev-Oct01-Haze1-SCS-2.out	8	Copy to Outflow"	0.278	0.278	3"	0.278	0.677"
40	6	HYDROGRAPH	Combine	3"			
50	3	Node #					
60		Total to Infiltration"					
70		Maximum flow				0.699	c.m./sec"
80		Hydrograph volume			10386.460		
90		0.278	0.278		0.278		c.m./sec"
100	40	HYDROGRAPH Start - New Tributary"				0.699"	
110	2	Start - New Tributary"					
120		0.278	0.000		0.278		0.699"
130	33	CATCHMENT 60"					
140	1	Triangular SCS"					
150	2	Proportional to %"					
160	1	SCS method"					
170	60	60-Infiltration on Golf Course"					
180		% Impervious"					
190	0.000	Total Area"					
200	0.550	Flow length"					
210	43.000	Overland Slope"					
220	3.000	Pervious Area"					
230	0.350	Pervious length"					
240	4.000	Pervious slope"					
250	3.000	Impervious Area"					
260	0.000	Impervious length"					
270	3.000	Impervious slope"					
280	0.250	Pervious Manning "n"					
290	74.000	Pervious SCS Curve No."					
300	0.728	Pervious Runoff coefficient"					
310	0.000	Pervious Ia/S coefficient"					
320	8.924	Pervious Initial abstraction"					
330	0.015	Impervious Manning "n"					
340	98.000	Impervious SCS Curve No."					
350	0.000	Impervious Runoff coefficient"					
360	0.000	Impervious Ia/S coefficient"					
370	0.518	Impervious Initial abstraction"					
380		0.076	0.000	0.278		0.699	c.m./sec"
390		Catchment 60	Pervious	Impervious	Total Area		
400		Surface Area	0.550	0.000	0.550		hectare"
410		Time of Concentration	18.084	0.001	18.084		minutes"
420		Time to Centroid	2513.316	2268.062	2513.315		minutes"
430		Rainfall depth	284.997	284.997	284.997		mm"
440		Rainfall volume	1567.48	0.00	1567.49		c.m."
450		Rainfall losses	77.589	33.581	77.589		mm"
460		Runoff depth	207.408	251.416	207.408		mm"
470		Runoff volume	1140.74	0.00	1140.74		c.m."
480		Runoff coefficient	0.728	0.000	0.728		c.m./sec"
490		Maximum flow	0.076	0.000	0.076		c.m./sec"
500		HYDROGRAPH Add Runoff "	0.076				
510	4	Add Runoff "	0.076	0.278	0.699"		
520	40	HYDROGRAPH Copy to Outflow"					
530	8	Copy to Outflow"	0.076	0.076	3"	0.076	0.699"
540	6	HYDROGRAPH	Combine	3"			
550	3	Node #					
560		Total to Infiltration"					
570		Maximum flow				0.774	c.m./sec"
580		Hydrograph volume			11727.206		
590		0.076	0.076		0.076		c.m./sec"
600		START/RE-START TOTALS 60"				0.774"	
610	38						

Page 8



**APPENDIX B:**  
**MIDUSS MODELLING – POST-DEVELOPMENT CONDITIONS**

218173- PostDev-Nov2019-2Yr.out  
-----  
MIDUSS Output Version 2.25 rev. 473  
MIDUSS version Sunday, February 07, 2010  
Units used: C:\Users\jswiger\Desktop\Wilder Lake  
Job folder: MIDUSS  
Output filename: 218173- PostDev-Nov7-Yr.out  
Licensee name: Hewlett-Packard Company  
Date & Time last used: 11/7/2019 at 3:51:56 PM  
TIME PARAMETERS  
31 10.000 Time Step  
360.000 Max. Storm length  
2400.000 Max. Hydrograph  
32 STORM Mass Curve  
3 Mass Curve  
38.800 Rainfall depth  
360.000 Duration  
47 C:\Program Files (x86)\MIDUSS\SCS\_6hr\_Type2.mrd SCS 6 hour Type  
Maximum intensity 61.945 mm/hr  
Total depth Hydrograph extension used in this file  
6 002HYD 100  
CATCHMENT 100  
1 Triangular SCS  
2 Proportional to %  
100 100- Wilder Lake South  
4.250 Total Area  
70.000 Flow length  
4.000 Overland Slope  
3.825 Pervious Area  
70.000 Pervious length  
4.000 Pervious slope  
0.425 Pervious Area  
7.778 Pervious length  
4.000 Pervious slope  
0.250 Pervious Manning 'n'  
74.000 Pervious SCS Curve No.  
0.193 Pervious Runoff coefficient  
8.924 Pervious Initial abstraction  
0.015 Pervious Manning 'n'  
98.000 Pervious SCS Curve No.  
0.781 Pervious Runoff coefficient  
0.100 Pervious Ia/S coefficient  
0.518 Pervious Initial abstraction  
0.065 0.000 0.000  
Catchment 100 Pervious  
Surface Area 3.825  
Time of concentration 27.427  
Rainfall depth 197.107  
Rainfall volume 38.800  
Rainfall losses 1484.10  
Runoff depth 31.321  
Runoff volume 7.479  
Runoff coefficient 0.193  
Maximum flow 0.055  
HYDROGRAPH Add Runoff 0.065 0.000 0.000  
4 Add Runoff 0.065 0.065 0.000  
Page 1

218173- PostDev-Nov2019-2Yr.out  
-----  
MIDUSS Output Version 2.25 rev. 473  
MIDUSS version Sunday, February 07, 2010  
Units used: C:\Users\jswiger\Desktop\Wilder Lake  
Job folder: MIDUSS  
Output filename: 218173- PostDev-Nov7-Yr.out  
Licensee name: Hewlett-Packard Company  
Date & Time last used: 11/7/2019 at 3:51:56 PM  
TIME PARAMETERS  
31 10.000 Time Step  
360.000 Max. Storm length  
2400.000 Max. Hydrograph  
32 STORM Mass Curve  
3 Mass Curve  
38.800 Rainfall depth  
360.000 Duration  
47 C:\Program Files (x86)\MIDUSS\SCS\_6hr\_Type2.mrd SCS 6 hour Type  
Maximum intensity 61.945 mm/hr  
Total depth Hydrograph extension used in this file  
6 002HYD 100  
CATCHMENT 100  
1 Triangular SCS  
2 Proportional to %  
100 100- Wilder Lake South  
4.250 Total Area  
70.000 Flow length  
4.000 Overland Slope  
3.825 Pervious Area  
70.000 Pervious length  
4.000 Pervious slope  
0.425 Pervious Area  
7.778 Pervious length  
4.000 Pervious slope  
0.250 Pervious Manning 'n'  
74.000 Pervious SCS Curve No.  
0.193 Pervious Runoff coefficient  
8.924 Pervious Initial abstraction  
0.015 Pervious Manning 'n'  
98.000 Pervious SCS Curve No.  
0.781 Pervious Runoff coefficient  
0.100 Pervious Ia/S coefficient  
0.518 Pervious Initial abstraction  
0.065 0.000 0.000  
Catchment 100 Pervious  
Surface Area 3.825  
Time of concentration 27.427  
Rainfall depth 197.107  
Rainfall volume 38.800  
Rainfall losses 1484.10  
Runoff depth 31.321  
Runoff volume 7.479  
Runoff coefficient 0.193  
Maximum flow 0.055  
HYDROGRAPH Add Runoff 0.065 0.000 0.000  
4 Add Runoff 0.065 0.065 0.000  
Page 1

218173- PostDev-Nov2019-2Yr.out  
-----  
MIDUSS Output Version 2.25 rev. 473  
MIDUSS version Sunday, February 07, 2010  
Units used: C:\Users\jswiger\Desktop\Wilder Lake  
Job folder: MIDUSS  
Output filename: 218173- PostDev-Nov7-Yr.out  
Licensee name: Hewlett-Packard Company  
Date & Time last used: 11/7/2019 at 3:51:56 PM  
TIME PARAMETERS  
31 10.000 Time Step  
360.000 Max. Storm length  
2400.000 Max. Hydrograph  
32 STORM Mass Curve  
3 Mass Curve  
38.800 Rainfall depth  
360.000 Duration  
47 C:\Program Files (x86)\MIDUSS\SCS\_6hr\_Type2.mrd SCS 6 hour Type  
Maximum intensity 61.945 mm/hr  
Total depth Hydrograph extension used in this file  
6 002HYD 100  
CATCHMENT 100  
1 Triangular SCS  
2 Proportional to %  
100 100- Wilder Lake South  
4.250 Total Area  
70.000 Flow length  
4.000 Overland Slope  
3.825 Pervious Area  
70.000 Pervious length  
4.000 Pervious slope  
0.425 Pervious Area  
7.778 Pervious length  
4.000 Pervious slope  
0.250 Pervious Manning 'n'  
74.000 Pervious SCS Curve No.  
0.193 Pervious Runoff coefficient  
8.924 Pervious Initial abstraction  
0.015 Pervious Manning 'n'  
98.000 Pervious SCS Curve No.  
0.781 Pervious Runoff coefficient  
0.100 Pervious Ia/S coefficient  
0.518 Pervious Initial abstraction  
0.065 0.000 0.000  
Catchment 100 Pervious  
Surface Area 3.825  
Time of concentration 27.427  
Rainfall depth 197.107  
Rainfall volume 38.800  
Rainfall losses 1484.10  
Runoff depth 31.321  
Runoff volume 7.479  
Runoff coefficient 0.193  
Maximum flow 0.055  
HYDROGRAPH Add Runoff 0.065 0.000 0.000  
4 Add Runoff 0.065 0.065 0.000  
Page 1

218173- PostDev-Nov2019-2Yr.out  
-----  
MIDUSS Output Version 2.25 rev. 473  
MIDUSS version Sunday, February 07, 2010  
Units used: C:\Users\jswiger\Desktop\Wilder Lake  
Job folder: MIDUSS  
Output filename: 218173- PostDev-Nov7-Yr.out  
Licensee name: Hewlett-Packard Company  
Date & Time last used: 11/7/2019 at 3:51:56 PM  
TIME PARAMETERS  
31 10.000 Time Step  
360.000 Max. Storm length  
2400.000 Max. Hydrograph  
32 STORM Mass Curve  
3 Mass Curve  
38.800 Rainfall depth  
360.000 Duration  
47 C:\Program Files (x86)\MIDUSS\SCS\_6hr\_Type2.mrd SCS 6 hour Type  
Maximum intensity 61.945 mm/hr  
Total depth Hydrograph extension used in this file  
6 002HYD 100  
CATCHMENT 100  
1 Triangular SCS  
2 Proportional to %  
100 100- Wilder Lake South  
4.250 Total Area  
70.000 Flow length  
4.000 Overland Slope  
3.825 Pervious Area  
70.000 Pervious length  
4.000 Pervious slope  
0.425 Pervious Area  
7.778 Pervious length  
4.000 Pervious slope  
0.250 Pervious Manning 'n'  
74.000 Pervious SCS Curve No.  
0.193 Pervious Runoff coefficient  
8.924 Pervious Initial abstraction  
0.015 Pervious Manning 'n'  
98.000 Pervious SCS Curve No.  
0.781 Pervious Runoff coefficient  
0.100 Pervious Ia/S coefficient  
0.518 Pervious Initial abstraction  
0.065 0.000 0.000  
Catchment 100 Pervious  
Surface Area 3.825  
Time of concentration 27.427  
Rainfall depth 197.107  
Rainfall volume 38.800  
Rainfall losses 1484.10  
Runoff depth 31.321  
Runoff volume 7.479  
Runoff coefficient 0.193  
Maximum flow 0.055  
HYDROGRAPH Add Runoff 0.065 0.000 0.000  
4 Add Runoff 0.065 0.065 0.000  
Page 1



40	423.000	0.000	0.000	
41	423.250	1.00E-04	15.300"	
42	423.500	0.00020	68.600"	
43	423.750	0.00030	162.000"	
44	424.000	0.00040	284.900"	
45	424.250	0.00050	438.500"	
46	424.500	0.00060	624.800"	
47	424.750	0.00350	845.500"	
48	425.000	0.03910	1102.500"	
49	425.250	0.04240	1397.400"	
50	425.500	0.04540	1731.900"	
51	425.750	1.114	2108.600"	
52	Peak outflow	0.008	c.m/sec"	
53	Maximum level	424.556	metre"	
54	Maximum storage	674.222	c.m"	
55	Centroidal lag	174.511	hours"	
56	0.206	0.206	0.065 c.m/sec"	
57	HYDROGRAPH " Combine	6"		
58	6 " Node #"			
59	Maximum flow	0.065	c.m/sec"	
60	Hydrograph volume	591.041	c.m"	
61	0.206	0.206	0.065"	
62	HYDROGRAPH Start - New Tributary	0.008	0.065"	
63	2 Start - New Tributary	0.206	0.000	
64	0.206	0.000	0.008	
65	CATCHMENT 102			
66	1 Triangular SCS			
67	2 Proportional to %			
68	1 SCS method			
69	102 102 - Developed Area North			
70	30.000	% Impervious		
71	45.000	Total Area		
72	2.000	Flow length		
73	0.630	Overland Slope		
74	45.000	Pervious Area		
75	2.000	Pervious length		
76	0.270	Pervious slope		
77	19.286	Impervious Area		
78	2.000	Impervious length		
79	0.250	Impervious slope		
80	74.600	Pervious Manning "n"		
81	0.193	Pervious SCS Curve No.		
82	0.100	Pervious Runoff coefficient		
83	8.924	Pervious Ia/S coefficient		
84	0.015	Pervious Initial abstraction		
85	98.000	Impervious Manning "n"		
86	0.838	Impervious SCS Curve No.		
87	0.100	Impervious Runoff coefficient		
88	0.518	Impervious Ia/S coefficient		
89	0.039	Impervious Initial abstraction		
90	0.000	Impervious Initial abstraction		
91	Catchment 102			
92	Surface Area	Pervious	0.065 c.m/sec"	
93	Time of concentration	0.630	Impervious	
94	Time to centroid	37.149	Total Area	
95	Rainfall depth	268.079	0.270	
96	Rainfall volume	38.800	2.108	
97	Rainfall losses	244.44	14.351	
98	Rainfall depth	31.319	197.933	
99	Runoff volume	7.481	222.441	
100	Runoff depth	47.13	38.800	
101	Runoff coefficient	0.133	104.76	
102		0.838	349.20	
103			6.296	
104			23.812	
105			14.988	
106			134.89	
107			0.838	

40	Maximum flow	0.009	0.038	0.039	c.m/sec"
41	HYDROGRAPH Add Runoff	0.009	0.038	0.039	
42	4 Add Runoff	0.039	0.039	0.065"	
43	POND DESIGN				
44	0.039	Current peak flow	c.m/sec"		
45	0.030	Target outflow	c.m/sec"		
46	134.9	Hydrograph volume	c.m"		
47	11.	Number of stages			
48	423.200	Minimum water level	metre"		
49	424.850	Maximum water level	metre"		
50	423.200	Starting water level	metre"		
51	0	Keep Design Data: 1 = True; 0 = False			
52	Level Discharge	Volume			
53	423.350	0.000	0.000"		
54	423.500	1.00E-05	9.800"		
55	423.650	2.00E-05	24.900"		
56	423.800	3.00E-05	45.600"		
57	423.950	4.00E-05	72.300"		
58	424.100	5.00E-05	105.600"		
59	424.250	6.00E-05	145.700"		
60	424.400	0.01320	193.300"		
61	424.550	0.01548	248.600"		
62	424.700	0.01746	312.100"		
63	424.850	0.5145	384.400"		
64	Peak outflow	0.000	c.m/sec"		
65	Maximum level	424.205	metre"		
66	Maximum storage	133.770	c.m"		
67	Centroidal lag	441.029	hours"		
68	0.039	0.039	0.000	0.065 c.m/sec"	
69	HYDROGRAPH " Combine	6"			
70	6 " Node #"				
71	Maximum flow	0.065	c.m/sec"		
72	Hydrograph volume	598.502	c.m"		
73	0.039	0.039	0.000	0.065"	
74	HYDROGRAPH Start - New Tributary	0.000	0.065"		
75	2 Start - New Tributary	0.039	0.000	0.065"	
76	0.039	0.000	0.000	0.065"	
77	CATCHMENT 200				
78	1 Triangular SCS				
79	2 Proportional to %				
80	1 SCS method				
81	200 200 - Creek				
82	% Impervious				
83	11.000	Total Area			
84	4.210	Flow length			
85	75.000	Overland Slope			
86	3.747	Pervious Area			
87	75.000	Pervious length			
88	5.000	Pervious slope			
89	0.463	Impervious Area			
90	9.270	Impervious length			
91	5.000	Impervious slope			
92	0.250	Pervious Manning "n"			
93	74.000	Pervious SCS Curve No.			
94	0.192	Pervious Runoff coefficient			
95	0.100	Pervious Ia/S coefficient			
96	8.924	Pervious Initial abstraction			
97	0.015	Impervious Manning "n"			
98	98.000	Impervious SCS Curve No.			
99	0.838	Impervious Runoff coefficient			
100	0.518	Impervious Initial abstraction			
101	0.039	Impervious Initial abstraction			
102	0.000	Impervious Initial abstraction			
103	Catchment 200				
104	Surface Area	Pervious	0.065 c.m/sec"		
105	Time of concentration	0.630	Impervious		
106	Time to centroid	37.149	Total Area		
107	Rainfall depth	268.079	0.270		
108	Rainfall volume	38.800	2.108		
109	Rainfall losses	244.44	14.351		
110	Rainfall depth	31.319	197.933		
111	Runoff volume	7.481	222.441		
112	Runoff depth	47.13	38.800		
113	Runoff coefficient	0.133	104.76		
114		0.838	349.20		
115			6.296		
116			23.812		
117			14.988		
118			134.89		
119			0.838		

218173- PostDev-Nov2019-2-yr.out									
0.100	Impervious	Ia/S coefficient	0.000	0.000	0.065	c.m/sec	"	Runoff depth	mm"
0.518	Impervious	Initial abstraction"	0.071	0.000	0.065	c.m/sec	"	Runoff volume	c.m"
	Catchment 200	Pervious	Imperious	Total Area "	0.065	c.m/sec	"	Runoff coefficient	c.m/sec"
	Surface Area	3,747	0.463	4,210	0.065	c.m/sec	"	Maximum flow	c.m/sec"
	Time of concentration	38.342	1.032	25.855	0.065	c.m/sec	"	4 Add Runoff "	c.m/sec"
	Rainfall depth	269.563	197.197	245.344	0.065	c.m/sec	"	HYDROGRAPH Copy to outflow"	c.m/sec"
	Rainfall volume	38.800	38.800	38.800	0.065	c.m/sec	"	8 Copy to outflow"	c.m/sec"
	Rainfall losses	1453.80	179.68	1633.48	0.065	c.m/sec	"	HYDROGRAPH Combine 10"	c.m/sec"
	Rainfall losses	31.332	8.404	28.810	0.065	c.m/sec	"	6 Combine "	c.m/sec"
	Runoff depth	7.468	30.396	9.990	0.065	c.m/sec	"	10 Node #"	c.m/sec"
	Runoff volume	279.82	140.76	420.59	0.065	c.m/sec	"	To Pond 201"	c.m/sec"
	Runoff coefficient	0.192	0.783	0.257	0.065	c.m/sec	"	Maximum flow	c.m/sec"
	Maximum flow	0.055	0.065	0.071	0.065	c.m/sec	"	Hydrograph volume	c.m/sec"
	HYDROGRAPH Add Runoff "	0.071	0.071	0.071	0.065	c.m/sec	"	0.064	c.m/sec"
40	4 Add Runoff "	0.071	0.071	0.071	0.065	c.m/sec	"	HYDROGRAPH Start - New Tributary"	c.m/sec"
	HYDROGRAPH Copy to outflow"	0.000	0.065	0.065	0.065	c.m/sec	"	2 Start - New Tributary"	c.m/sec"
40	8 Copy to outflow"	0.071	0.071	0.071	0.065	c.m/sec	"	0.064	c.m/sec"
	HYDROGRAPH Start - New Tributary"	0.071	0.071	0.071	0.065	c.m/sec	"	CATCHMENT 400"	c.m/sec"
40	6 Combine "	0.071	0.071	0.071	0.065	c.m/sec	"	1 Triangular SCS"	c.m/sec"
	7 Node #"	0.071	0.071	0.071	0.065	c.m/sec	"	2 Proportional to %"	c.m/sec"
	Total to Creek"	0.071	0.071	0.071	0.065	c.m/sec	"	400 South to off site"	c.m/sec"
	Maximum flow	0.071	0.071	0.071	0.065	c.m/sec	"	% Impervious	c.m/sec"
	Hydrograph volume	0.071	0.071	0.071	0.065	c.m/sec	"	0.990 Total Area "	c.m/sec"
	HYDROGRAPH Start - New Tributary"	0.071	0.071	0.071	0.065	c.m/sec	"	75,000 Flow length "	c.m/sec"
	2 Start - New Tributary"	0.000	0.071	0.071	0.065	c.m/sec	"	2,000 Overland Slope "	c.m/sec"
	CATCHMENT 201"	0.000	0.071	0.071	0.065	c.m/sec	"	0.911 Pervious Area "	c.m/sec"
	1 Triangular SCS"	0.000	0.071	0.071	0.065	c.m/sec	"	75,000 Pervious length "	c.m/sec"
	2 Proportional to %"	0.000	0.071	0.071	0.065	c.m/sec	"	2,000 Pervious slope "	c.m/sec"
	1 SCS method"	0.000	0.071	0.071	0.065	c.m/sec	"	0.079 Imperious Area "	c.m/sec"
	201	0.000	0.071	0.071	0.065	c.m/sec	"	6,522 Imperious length "	c.m/sec"
	% Impervious"	0.000	0.071	0.071	0.065	c.m/sec	"	2,000 Imperious slope "	c.m/sec"
	2,150 Total Area "	0.000	0.071	0.071	0.065	c.m/sec	"	0.250 Pervious Manning "n"	c.m/sec"
	45,000 Flow length "	0.000	0.071	0.071	0.065	c.m/sec	"	74,000 Pervious SCS Curve No. "	c.m/sec"
	2,000 Overland Slope "	0.000	0.071	0.071	0.065	c.m/sec	"	0.193 Pervious Runoff coefficient "	c.m/sec"
	1,720 Pervious Area "	0.000	0.071	0.071	0.065	c.m/sec	"	0.100 Pervious Ia/S coefficient "	c.m/sec"
	45,000 Pervious length "	0.000	0.071	0.071	0.065	c.m/sec	"	8,924 Pervious Initial abstraction "	c.m/sec"
	2,000 Pervious slope "	0.000	0.071	0.071	0.065	c.m/sec	"	0.015 Imperious Manning "n"	c.m/sec"
	0.430 Imperious Area "	0.000	0.071	0.071	0.065	c.m/sec	"	98,000 Imperious SCS Curve No. "	c.m/sec"
	11,250 Imperious slope "	0.000	0.071	0.071	0.065	c.m/sec	"	0.790 Imperious Runoff coefficient "	c.m/sec"
	2,000 Imperious length "	0.000	0.071	0.071	0.065	c.m/sec	"	0.100 Imperious Ia/S coefficient "	c.m/sec"
	0.250 Pervious Manning "n"	0.000	0.071	0.071	0.065	c.m/sec	"	0.518 Imperious Initial abstraction "	c.m/sec"
	74,000 Pervious SCS Curve No. "	0.000	0.071	0.071	0.065	c.m/sec	"	Catchment 400	c.m/sec"
	0.193 Pervious Runoff coefficient "	0.000	0.071	0.071	0.065	c.m/sec	"	Surface Area	c.m/sec"
	0.100 Pervious Ia/S coefficient "	0.000	0.071	0.071	0.065	c.m/sec	"	Time of Concentration	c.m/sec"
	8,924 Pervious Initial abstraction "	0.000	0.071	0.071	0.065	c.m/sec	"	284,422	c.m/sec"
	0.015 Imperious Manning "n"	0.000	0.071	0.071	0.065	c.m/sec	"	Rainfall depth	c.m/sec"
	98,000 Imperious SCS Curve No. "	0.000	0.071	0.071	0.065	c.m/sec	"	38,800	c.m/sec"
	0.818 Imperious Runoff coefficient "	0.000	0.071	0.071	0.065	c.m/sec	"	Rainfall volume	c.m/sec"
	0.100 Imperious Ia/S coefficient "	0.000	0.071	0.071	0.065	c.m/sec	"	353,39	c.m/sec"
	0.518 Imperious Initial abstraction "	0.000	0.071	0.071	0.065	c.m/sec	"	Rainfall losses	c.m/sec"
	Catchment 201	0.000	0.071	0.071	0.065	c.m/sec	"	71,322	c.m/sec"
	Surface Area	1,720	0.430	2,150	0.071	c.m/sec	"	Runoff depth	c.m/sec"
	Time of concentration	37.149	1.525	18.809	0.071	c.m/sec	"	Runoff volume	c.m/sec"
	Rainfall depth	268.079	197.342	231.764	0.071	c.m/sec	"	Runoff coefficient	c.m/sec"
	Rainfall volume	38.800	38.800	38.800	0.071	c.m/sec	"	Maximum flow	c.m/sec"
	Rainfall losses	166.84	16.84	166.84	0.071	c.m/sec	"	HYDROGRAPH Add Runoff "	c.m/sec"
	Rainfall losses	31.319	7.047	26.465	0.071	c.m/sec	"	4 Add Runoff "	c.m/sec"
								8 Copy to outflow"	c.m/sec"

[illegible][illegible]

30.000	Pervious length	0.126 c.m/sec	
4.000	Pervious slope	0.000	
0.000	Impervious Area	0.001	
0.000	Impervious length	194.150	
4.000	Impervious slope	38.800	
0.250	Pervious Manning "n"	0.00	
74.000	Pervious SCS Curve No.	31.317	
0.193	Pervious Runoff coefficient	7.483	
0.100	Pervious Ia/S coefficient	41.35	
8.924	Pervious Initial abstraction	0.000	
0.015	Impervious Manning "n"	0.000	
98.000	Impervious SCS Curve No.	0.000	
0.000	Impervious Runoff coefficient	0.000	
0.100	Impervious Ia/S coefficient	0.000	
0.518	Impervious Initial abstraction	0.000	
	Catchment 600	0.063	
	Surface Area	0.550	
	Time to Concentration	23.659	
	Time to Centroid	251.504	
	Rainfall depth	38.800	
	Rainfall volume	213.40	
	Rainfall losses	31.317	
	Runoff depth	7.483	
	Runoff volume	30.342	
	Runoff coefficient	41.35	
	Maximum flow	0.193	
	HYDROGRAPH Add Runoff	0.011	
40	4 Add Runoff	0.011	0.126"
40	8 HYDROGRAPH Copy to Outflow	0.011	0.126"
40	HYDROGRAPH Copy to Outflow	0.011	0.126"
40	6 HYDROGRAPH Combine 10"	0.011	0.126"
40	10 Node #	10"	0.126"
	To Pond 201"	0.128	c.m/sec
	Maximum flow	534.969	c.m"
	Hydrograph volume	0.011	0.128"
40	7 HYDROGRAPH Confluence 10"	0.011	0.128"
40	10 Node #	10"	0.128"
	To Pond 201"	0.128	c.m/sec
	Maximum flow	534.969	c.m"
	Hydrograph volume	0.011	0.000"
54	POND DESIGN	0.011	0.128
0.128	Current peak flow	c.m/sec	
0.100	Target outflow	c.m/sec	
535.0	Hydrograph volume	c.m"	
10.	Number of stages	10	
423.300	Minimum water level	metre	
424.500	Maximum water level	metre	
423.300	Starting water level	metre	
0	Keep Design Data: 1 = True; 0 = False		
	Level Discharge	Volume	
	423.300	0.000	
	423.400	172.400	
	423.500	353.500	
	423.600	543.700	
	423.700	743.000	
	423.800	951.800	

218173- PostDev-Nov2019-2yr.out				
423.900	0.05904	1170.100"		
424.000	0.3343	1398.300"		
424.100	0.8324	1836.400"		
424.200	1.475	1884.800"		
Peak outflow	0.023	c.m/sec		
Maximum level	423.475	metre"		
Maximum storage	307.387	c.m"		
Centroidal lag	7.840	hours"		
0.011	0.128	0.023	0.000 c.m/sec"	
HYDROGRAPH Combine	7"			
6 Combine "				
7 Node #				
Total to Creek"				
Maximum flow	0.081	c.m/sec"		
Hydrograph volume	955.447	c.m"		
0.011	0.023	0.081"		
3 START/RE-START TOTALS 10"				
3 Runoff Totals on EXIT"				
Total Catchment area			21.320	
Total Impervious area			3.692	
Total % Impervious			17.317"	
EXIT"				

```

218173- PostDev-Oct2019-5yr.out
MIDUSS Output -----
MIDUSS version Version 2.25 rev. 473"
MIDUSS created Sunday, February 07, 2010"
Units used: C:\Users\jswiger\Desktop\Wilder Lake"
Job folder: C:\Users\jswiger\Desktop\Wilder Lake"
MIDUSS"
Output filename: 218173- PostDev-Oct22-5yr.gdb"
Licensee name: Hewlett-Packard Company"
Company Date & time last used: 10/22/2019 at 2:59:54 PM"
TIME PARAMETERS"
31 10.000 Time Step"
360.000 Max. Storm length"
2400.000 Max. Hydrograph"
32 STORM Mass Curve"
3 Mass Curve"
49.400 Rainfall depth"
360.000 Duration"
47 C:\Program Files (x86)\MIDUSS\SCS_6hr_Type2.mrd SCS 6 hour Type
Maximum intensity 78.868 mm/hr"
Total depth 49.400 mm"
6 005hvd Hydrograph extension used in this file"
CATCHMENT 101"
1 Triangular SCS"
1 Proportional to %"
1 SCS method"
100 100- Wilder Lake South"
1 % Impervious"
10.000 Total Area"
4.250 Flow length"
70.000 Overland Slope"
3.825 Pervious Area"
70.000 Pervious length"
4.000 Pervious Slope"
0.425 Impervious Area"
7.778 Impervious length"
4.000 Pervious Manning "n"
0.250 Pervious SCS Curve No."
0.255 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Pervious Manning "n"
98.000 Impervious SCS Curve No."
0.799 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
Catchment 100 Pervious 0.000 0.000
Surface Area 3.825 0.425 0.000 c.m/sec"
Time of concentration 30.929 23.171 0.895 hectare"
Time to Centroid 258.231 195.261 0.895 minutes"
Rainfall depth 49.400 49.400 241.964 mm"
Rainfall volume 1889.55 209.95 2099.50 c.m"
Rainfall losses 36.808 9.928 34.120 mm"
Runoff depth 12.592 39.472 15.280 mm"
Runoff volume 481.64 167.76 649.39 c.m"
Runoff coefficient 0.255 0.799 0.309 c.m/sec"
Maximum flow 0.115 0.077 0.125 c.m/sec"
HYDROGRAPH Add Runoff "
4 Add Runoff" 0.125 0.125 0.000"

```

```

218173- PostDev-Oct2019-5yr.out
HYDROGRAPH Copy to outflow"
8 Copy to outflow" 0.125 0.125 0.000"
HYDROGRAPH " Combine 6"
6 Combining "
6 Node "
to Wilder Lake"
Maximum flow 0.125 c.m/sec"
Hydrograph volume 649.394 c.m"
0.125 0.125 0.125"
HYDROGRAPH start - New Tributary"
2 start - New Tributary" 0.125 0.125 0.125"
0.000 0.000
CATCHMENT 101"
1 Triangular SCS"
2 Proportional to %"
1 SCS method"
101 101 - Developed Area South"
1 % Impervious"
30.000 Total Area"
4.890 Flow length"
75.000 Overland Slope"
3.423 Pervious Area"
75.000 Pervious length"
2.000 Pervious Slope"
1.467 Impervious Area"
32.143 Impervious length"
2.000 Impervious Manning "n"
0.250 Pervious SCS Curve No."
0.255 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Pervious Manning "n"
98.000 Impervious SCS Curve No."
0.875 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.275 0.000 0.125 c.m/sec"
Catchment 101 Pervious 0.000 0.125
Surface Area 3.423 1.467 4.890 hectare"
Time of concentration 39.688 2.581 17.613 minutes"
Time to Centroid 269.723 197.459 226.530 mm"
Rainfall depth 49.400 49.400 49.400 c.m"
Rainfall volume 1890.96 6.167 2413.66 mm"
Rainfall losses 36.783 6.167 27.398 mm"
Runoff depth 12.617 43.233 21.802 c.m"
Runoff volume 634.23 0.875 1066.10 c.m"
Runoff coefficient 0.255 0.875 0.441 c.m/sec"
Maximum flow 0.084 0.266 0.275 c.m/sec"
HYDROGRAPH Add Runoff "
4 Add Runoff" 0.275 0.125 0.125"
POND DESIGN"
0.275 0.275 0.125 c.m/sec"
0.275 Current peak flow c.m/sec"
0.080 Target outflow c.m/sec"
1066.1 Hydrograph volume c.m"
12. Number of stages"
423.000 Minimum water level metre"
423.750 Maximum water level metre"
423.000 Starting water level metre"
0 Keep Design Data: 1 = True, 0 = False"
Level Discharge Volume"
Page 2

```



218173- PostDev-Oct2019-5yr.out

423.000 0.000 0.000"  
 423.250 1.00E-04 15.300"  
 423.500 0.00020 68.600"  
 423.750 0.00030 162.000"  
 424.000 0.00040 284.900"  
 424.250 0.00050 438.500"  
 424.500 0.00060 624.800"  
 424.750 0.00070 845.500"  
 425.000 0.00080 1102.500"  
 425.250 0.00090 1397.400"  
 425.500 0.00100 1731.900"  
 425.750 1.114 2108.600"  
 Peak outflow 0.028 c.m/sec"  
 Maximum storage 424.694 metre"  
 Centroidal lag 796.257 c.m"  
 0.275 0.275 0.028 0.125 c.m/sec"  
 6 Combine " 6"  
 6 Node #"  
 To Wilder Lake"  
 Maximum flow 0.125 c.m/sec"  
 Hydrograph volume 1146.508 c.m"  
 0.275 0.275 0.028 0.125"  
 HYDROGRAPH Start - New Tributary"  
 2 Start - New Tributary" 0.000 0.028 0.125"  
 CATCHMENT 102" 0.000 0.028 0.125"  
 1 Triangular SCS"  
 2 Proportional to %"  
 1 SCS method"  
 102 102 - Developed Area North"  
 30.000 % Impervious"  
 0.900 Total Area"  
 45.000 Flow length"  
 2.000 Overland Slope"  
 0.630 Pervious Area"  
 45.000 Pervious length"  
 2.000 Pervious slope"  
 0.270 Impervious Area"  
 19.286 Impervious length"  
 2.000 Impervious slope"  
 0.250 Pervious Manning "n"  
 74.200 Pervious SCS Curve No. "  
 0.255 Pervious Runoff coefficient"  
 0.100 Pervious Ia/S coefficient"  
 8.924 Pervious Initial abstraction"  
 0.015 Pervious Manning "n"  
 98.000 Impervious SCS Curve No. "  
 0.856 Impervious Runoff coefficient"  
 0.100 Impervious Ia/S coefficient"  
 0.518 Impervious Initial abstraction"  
 CATCHMENT 102" 0.000 0.028 0.125 c.m/sec"  
 Surface Area 0.630 hectare"  
 Time of concentration 29.211 minutes"  
 Time to Centroid 256.003 minutes"  
 Rainfall depth 49.400 mm"  
 Rainfall volume 311.22 c.m"  
 Rainfall losses 36.785 mm"  
 Runoff depth 7.116 mm"  
 Runoff volume 114.17 c.m"  
 Runoff coefficient 0.255 0.856 0.436

218173- PostDev-Oct2019-5yr.out

Maximum flow 0.053 c.m/sec"  
 HYDROGRAPH Add Runoff "0.019 0.050 0.053  
 4 Add Runoff" 0.053 0.053 0.125"  
 POND DESIGN"  
 0.053 Current peak flow c.m/sec"  
 0.053 Target outflow c.m/sec"  
 193.6 Hydrograph volume c.m"  
 11. Number of stages"  
 423.200 Minimum water level metre"  
 424.850 Minimum water level metre"  
 423.200 Starting water level metre"  
 0 Keep Design Data: 1 = True; 0 = False"  
 Level Discharge Volume"  
 423.350 0.000 0.000"  
 423.500 1.00E-05 9.800"  
 423.650 2.00E-05 24.900"  
 423.800 3.00E-05 45.600"  
 423.950 4.00E-05 72.300"  
 424.100 5.00E-05 105.600"  
 424.250 6.00E-05 145.700"  
 424.400 0.01320 193.300"  
 424.550 0.01348 248.600"  
 424.700 0.01746 312.100"  
 424.850 0.5145 384.400"  
 Peak outflow 0.004 c.m/sec"  
 Maximum storage 424.297 metre"  
 Centroidal lag 160.645 c.m"  
 0.053 0.053 0.004 0.125 c.m/sec"  
 HYDROGRAPH Combine 6"  
 6 Combine " 6"  
 6 Node #"  
 To Wilder Lake"  
 Maximum flow 0.125 c.m/sec"  
 Hydrograph volume 1200.646 c.m"  
 0.053 0.053 0.004 0.125"  
 HYDROGRAPH Start - New Tributary"  
 2 Start - New Tributary" 0.000 0.004 0.125"  
 CATCHMENT 200" 0.053 0.000 0.125"  
 1 Triangular SCS"  
 2 Proportional to %"  
 1 SCS method"  
 200 200 - Creek"  
 11.000 % Impervious"  
 4.210 Total Area"  
 75.000 Flow length"  
 5.000 Overland Slope"  
 3.747 Pervious Area"  
 75.000 Pervious length"  
 5.000 Pervious slope"  
 0.463 Impervious Area"  
 9.270 Impervious length"  
 5.000 Impervious slope"  
 0.250 Pervious Manning "n"  
 74.000 Pervious SCS Curve No. "  
 0.255 Pervious Runoff coefficient"  
 0.100 Pervious Ia/S coefficient"  
 8.924 Pervious Initial abstraction"  
 0.015 Impervious Manning "n"  
 98.000 Impervious SCS Curve No. "  
 0.801 Impervious Runoff coefficient"

218173- PostDev-Oct2019-5yr.out									
0.100	Impervious Ia/S coefficient"								
0.518	Impervious Initial abstraction"								
0.124	0.000	0.004	0.125 c.m/sec"						
Catchment 200	Pervious	Imperious	Total Area "						
Surface Area	3,747	0.463	4,210	hectare"					
Time of concentration	30.149	0.930	21.975	minutes"					
Rainfall depth	257.210	135.358	239.907	mm"					
Rainfall volume	49,400	49,400	49,400	c.m"					
Rainfall losses	1850.97	228.77	2079.74	mm"					
Rainfall losses	36.801	9.810	33.832	mm"					
Runoff depth	12,599	39,590	15,588	mm"					
Runoff volume	472.06	183.34	655.40	c.m"					
Runoff coefficient	0.255	0.801	0.315	c.m/sec"					
Maximum flow	0.113	0.084	0.124	c.m/sec"					
4	Add Runoff"	0.124	0.124	0.125"					
HYDROGRAPH Copy to Outflow"									
8	Copy to Outflow"	0.124	0.124	0.125"					
HYDROGRAPH 7"									
6	Combine "	0.124	0.124	0.125"					
7	Node #"								
Total to Creek"									
Maximum flow	0.124	0.124	0.124	c.m/sec"					
Hydrograph volume	655.404	0.124	0.124	c.m"					
HYDROGRAPH Start - New Tributary"	0.124	0.124	0.124	0.124"					
2	Start - New Tributary"								
0.124	0.000	0.124	0.124	0.124"					
CATCHMENT 201"									
1	Triangular SCS"								
2	Proportional to %"								
SCS method"									
201	% Impervious"								
20,000	Total Area"								
2,150	Flow length"								
45,000	Overland Slope"								
2,000	Pervious Area"								
1,720	Pervious length"								
45,000	Pervious slope"								
2,000	Impervious Area"								
0.430	Impervious length"								
11,250	Impervious slope"								
2,000	Impervious Manning "n"								
0.350	Pervious SCS Curve No."								
74,000	Pervious Runoff coefficient"								
0.255	Pervious Ia/S coefficient"								
0.100	Pervious Initial abstraction"								
8,924	Impervious Manning "n"								
0.015	Impervious SCS Curve No."								
98,000	Impervious Runoff coefficient"								
0.835	Impervious Ia/S coefficient"								
0.100	Impervious Initial abstraction"								
0.518	Impervious Initial abstraction"								
Catchment 201	Pervious	Imperious	Total Area "						
Surface Area	1,720	0.430	2,150	hectare"					
Time of concentration	29.211	1.375	16.692	minutes"					
Rainfall depth	256.003	195.982	229.008	mm"					
Rainfall volume	49,400	49,400	49,400	c.m"					
Rainfall losses	849.68	212.42	1062.10	mm"					
Rainfall losses	36.785	8.155	31.059	mm"					
4	Add Runoff"	0.124	0.124	0.124 c.m/sec"					
HYDROGRAPH Copy to Outflow"									
8	Copy to Outflow"	0.124	0.124	0.124 c.m/sec"					
HYDROGRAPH 7"									
6	Combine "	0.124	0.124	0.124 c.m/sec"					
7	Node #"								
Total to Creek"									
Maximum flow	0.124	0.124	0.124	c.m/sec"					
Hydrograph volume	655.404	0.124	0.124	c.m"					
HYDROGRAPH Start - New Tributary"	0.124	0.124	0.124	0.124"					
2	Start - New Tributary"								
0.124	0.000	0.124	0.124	0.124"					
CATCHMENT 201"									
1	Triangular SCS"								
2	Proportional to %"								
SCS method"									
201	% Impervious"								
20,000	Total Area"								
2,150	Flow length"								
45,000	Overland Slope"								
2,000	Pervious Area"								
1,720	Pervious length"								
45,000	Pervious slope"								
2,000	Impervious Area"								
0.430	Impervious length"								
11,250	Impervious slope"								
2,000	Impervious Manning "n"								
0.350	Pervious SCS Curve No."								
74,000	Pervious Runoff coefficient"								
0.255	Pervious Ia/S coefficient"								
0.100	Pervious Initial abstraction"								
8,924	Impervious Manning "n"								
0.015	Impervious SCS Curve No."								
98,000	Impervious Runoff coefficient"								
0.835	Impervious Ia/S coefficient"								
0.100	Impervious Initial abstraction"								
0.518	Impervious Initial abstraction"								
Catchment 201	Pervious	Imperious	Total Area "						
Surface Area	1,720	0.430	2,150	hectare"					
Time of concentration	29.211	1.375	16.692	minutes"					
Rainfall depth	256.003	195.982	229.008	mm"					
Rainfall volume	49,400	49,400	49,400	c.m"					
Rainfall losses	849.68	212.42	1062.10	mm"					
Rainfall losses	36.785	8.155	31.059	mm"					
4	Add Runoff"	0.124	0.124	0.124 c.m/sec"					
HYDROGRAPH Copy to Outflow"									
8	Copy to Outflow"	0.124	0.124	0.124 c.m/sec"					
HYDROGRAPH 7"									
6	Combine "	0.124	0.124	0.124 c.m/sec"					
7	Node #"								
Total to Creek"									
Maximum flow	0.124	0.124	0.124	c.m/sec"					
Hydrograph volume	655.404	0.124	0.124	c.m"					
HYDROGRAPH Start - New Tributary"	0.124	0.124	0.124	0.124"					
2	Start - New Tributary"								
0.124	0.000	0.124	0.124	0.124"					
CATCHMENT 201"									
1	Triangular SCS"								
2	Proportional to %"								
SCS method"									
201	% Impervious"								
20,000	Total Area"								
2,150	Flow length"								
45,000	Overland Slope"								
2,000	Pervious Area"								
1,720	Pervious length"								
45,000	Pervious slope"								
2,000	Impervious Area"								
0.430	Impervious length"								
11,250	Impervious slope"								
2,000	Impervious Manning "n"								
0.350	Pervious SCS Curve No."								
74,000	Pervious Runoff coefficient"								
0.255	Pervious Ia/S coefficient"								
0.100	Pervious Initial abstraction"								
8,924	Impervious Manning "n"								
0.015	Impervious SCS Curve No."								
98,000	Impervious Runoff coefficient"								
0.835	Impervious Ia/S coefficient"								
0.100	Impervious Initial abstraction"								
0.518	Impervious Initial abstraction"								
Catchment 201	Pervious	Imperious	Total Area "						
Surface Area	1,720	0.430	2,150	hectare"					
Time of concentration	29.211	1.375	16.692	minutes"					
Rainfall depth	256.003	195.982	229.008	mm"					
Rainfall volume	49,400	49,400	49,400	c.m"					
Rainfall losses	849.68	212.42	1062.10	mm"					
Rainfall losses	36.785	8.155	31.059	mm"					
4	Add Runoff"	0.124	0.124	0.124 c.m/sec"					
HYDROGRAPH Copy to Outflow"									
8	Copy to Outflow"	0.124	0.124	0.124 c.m/sec"					
HYDROGRAPH 7"									
6	Combine "	0.124	0.124	0.124 c.m/sec"					
7	Node #"								
Total to Creek"									
Maximum flow	0.124	0.124	0.124	c.m/sec"					
Hydrograph volume	655.404	0.124	0.124	c.m"					
HYDROGRAPH Start - New Tributary"	0.124	0.124	0.124	0.124"					
2	Start - New Tributary"								
0.124	0.000	0.124	0.124	0.124"					
CATCHMENT 201"									
1	Triangular SCS"								
2	Proportional to %"								
SCS method"									
201	% Impervious"								
20,000	Total Area"								
2,150	Flow length"								
45,000	Overland Slope"								
2,000	Pervious Area"								
1,720	Pervious length"								
45,000	Pervious slope"								
2,000	Impervious Area"								
0.430	Impervious length"								
11,250	Impervious slope"								
2,000	Impervious Manning "n"								
0.350	Pervious SCS Curve No."								
74,000	Pervious Runoff coefficient"								
0.255	Pervious Ia/S coefficient"								
0.100	Pervious Initial abstraction"								
8,924	Impervious Manning "n"								
0.015	Impervious SCS Curve No."								
98,000	Impervious Runoff coefficient"								
0.835	Impervious Ia/S coefficient"								
0.100	Impervious Initial abstraction"								
0.518	Impervious Initial abstraction"								
Catchment 201	Pervious	Imperious	Total Area "						
Surface Area	1,720	0.430	2,150	hectare"					
Time of concentration	29.211	1.375	16.692	minutes"					
Rainfall depth	256.003	195.982	229.008	mm"					
Rainfall volume	49,400	49,400	49,400	c.m"					
Rainfall losses	849.68	212.42	1062.10	mm"					
Rainfall losses	36.785	8.155	31.059	mm"					
4	Add Runoff"	0.124	0.124	0.124 c.m/sec"					
HYDROGRAPH Copy to Outflow"									
8	Copy to Outflow"	0.124	0.124	0.124 c.m/sec"					
HYDROGRAPH 7"									
6	Combine "	0.124	0.124	0.124 c.m/sec"					
7	Node #"								
Total to Creek"									
Maximum flow	0.124	0.124	0.124	c.m/sec"				</	

40	HYDROGRAPH " Combine	8			
6	Node #				
1	Total to Off-Site"		0.024		C.m/sec"
1	Maximum Flow		146.413		C.m"
1	Hydrograph volume		0.024		0.024"
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"		0.000		0.024"
33	CATCHMENT 401"				
1	Triangular SCS"				
1	Proportional to %"				
2	SCS method"				
401	401 - To Off Site - West"				
8.000	% Impervious"				
1.690	Total Area"				
75.000	Flow length"				
3.000	Overland Slope"				
1.555	Pervious Area"				
75.000	Pervious length"				
3.000	Pervious Slope"				
0.135	Impervious Area"				
6.522	Impervious length"				
3.000	Impervious slope"				
0.250	Pervious Manning "n"				
74.000	Pervious SCS Curve No."				
0.355	Pervious Runoff coefficient"				
0.100	Pervious Ia/S coefficient"				
8.924	Pervious Initial abstraction"				
0.015	Impervious Manning "n"				
98.000	Impervious SCS Curve No."				
0.798	Impervious Runoff coefficient"				
0.100	Impervious Ia/S coefficient"				
0.518	Impervious Initial abstraction"				
0.045	Pervious		0.000	0.024	0.024 C.m"
0.135	Impervious				0.024 C.m"
1.6	Surface Area		1.555		1.6
27	Time of concentration		35.142		27
263.142	Time to Centroid		263.458		263.142
195.210	Rainfall depth		49.400		195.210
49.400	Rainfall volume		768.07		49.400
768.07	Rainfall losses		36.780		768.07
36.780	Rainfall depth		12.620		36.780
12.620	Rainfall volume		196.22		12.620
196.22	Runoff coefficient		0.255		196.22
0.255	Runoff volume		0.042		0.255
0.042	Maximum flow				0.042
4	Add runoff"		0.045	0.024	0.024
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"		0.045	0.045	0.024
0.045	Hydrograph " Combine		8"		
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				
6	Node #				
1	Total to off-site"				
1	Maximum Flow				
1	Hydrograph volume				
40	HYDROGRAPH Start - New Tributary"				
2	Start - New Tributary"				
40	HYDROGRAPH Add runoff"				
0.045	Hydrograph Copy to Outflow"				
8	Copy to Outflow"				
0.045	Hydrograph " Combine				

```

218173- PostDev-oct2019-5yr.out
CATCHMENT 500"
1 Triangular SCS"
2 Proportional to %"
3 SCS method"
4 SCS - to golf course"
5 500
6 % Impervious"
7 1.690
8 Total Area"
9 45.000
10 Flow length"
11 4.000
12 Overland Slope"
13 1.268
14 Pervious Area"
15 45.000
16 Pervious length"
17 4.000
18 Pervious slope"
19 4.423
20 Impervious Area"
21 15.000
22 Impervious length"
23 4.000
24 Impervious slope"
25 4.000
26 Pervious Manning 'n'"
27 0.250
28 Pervious SCS Curve No."
29 74.000
30 Pervious SCS"
31 0.255
32 Pervious Runoff coefficient"
33 0.100
34 Pervious Ia/S Coefficient"
35 8.924
36 Pervious Initial abstraction"
37 0.015
38 Pervious Manning 'n'"
39 0.015
40 Impervious SCS Curve No."
41 98.000
42 Impervious SCS"
43 0.832
44 Impervious Runoff coefficient"
45 0.100
46 Impervious Ia/S Coefficient"
47 0.518
48 Impervious Initial abstraction"
49 0.086
50 Pervious Initial abstraction"
51 0.000
52 Pervious Runoff coefficient"
53 0.045
54 Pervious Manning 'n'"
55 1.268
56 Pervious SCS Curve No."
57 74.000
58 Pervious SCS"
59 0.255
60 Pervious Runoff coefficient"
61 0.100
62 Pervious Initial abstraction"
63 0.015
63 Impervious SCS Curve No."
64 98.000
65 Impervious SCS"
66 0.832
67 Impervious Runoff coefficient"
68 0.100
69 Impervious Ia/S Coefficient"
70 0.518
71 Impervious Initial abstraction"
72 0.086
73 Pervious Initial abstraction"
74 0.000
75 Pervious Runoff coefficient"
76 0.045
77 Pervious Manning 'n'"
78 1.268
79 Pervious SCS Curve No."
80 74.000
81 Pervious SCS"
82 0.255
83 Pervious Runoff coefficient"
84 0.100
85 Pervious Initial abstraction"
86 0.015
86 Impervious SCS Curve No."
87 98.000
88 Impervious SCS"
89 0.832
90 Impervious Runoff coefficient"
91 0.100
92 Impervious Ia/S Coefficient"
93 0.518
94 Impervious Initial abstraction"
95 0.086
96 Pervious Initial abstraction"
97 0.000
98 Pervious Runoff coefficient"
99 0.045
100 Pervious Manning 'n'"
101 1.268
102 Pervious SCS Curve No."
103 74.000
104 Pervious SCS"
105 0.255
106 Pervious Runoff coefficient"
107 0.100
107 Impervious SCS Curve No."
108 98.000
109 Impervious SCS"
110 0.832
111 Impervious Runoff coefficient"
112 0.100
112 Impervious Ia/S Coefficient"
113 0.518
113 Impervious Initial abstraction"
114 0.086
115 Pervious Initial abstraction"
116 0.000
117 Pervious Runoff coefficient"
118 0.045
119 Pervious Manning 'n'"
120 1.268
121 Pervious SCS Curve No."
122 74.000
123 Pervious SCS"
124 0.255
125 Pervious Runoff coefficient"
126 0.100
126 Impervious SCS Curve No."
127 98.000
128 Impervious SCS"
129 0.832
130 Impervious Runoff coefficient"
131 0.100
131 Impervious Ia/S Coefficient"
132 0.518
132 Impervious Initial abstraction"
133 0.086
134 Pervious Initial abstraction"
135 0.000
136 Pervious Runoff coefficient"
137 0.045
137 Impervious SCS Curve No."
138 98.000
139 Impervious SCS"
140 0.832
141 Impervious Runoff coefficient"
142 0.100
142 Impervious Ia/S Coefficient"
143 0.518
143 Impervious Initial abstraction"
144 0.086
145 Pervious Initial abstraction"
146 0.000
147 Pervious Runoff coefficient"
148 0.045
149 Pervious Manning 'n'"
150 1.268
151 Pervious SCS Curve No."
152 74.000
153 Pervious SCS"
154 0.255
155 Pervious Runoff coefficient"
156 0.100
156 Impervious SCS Curve No."
157 98.000
158 Impervious SCS"
159 0.832
160 Impervious Runoff coefficient"
161 0.100
161 Impervious Ia/S Coefficient"
162 0.518
162 Impervious Initial abstraction"
163 0.086
164 Pervious Initial abstraction"
165 0.000
166 Pervious Runoff coefficient"
167 0.045
167 Impervious SCS Curve No."
168 98.000
169 Impervious SCS"
170 0.832
171 Impervious Runoff coefficient"
172 0.100
172 Impervious Ia/S Coefficient"
173 0.518
173 Impervious Initial abstraction"
174 0.086
175 Pervious Initial abstraction"
176 0.000
177 Pervious Runoff coefficient"
178 0.045
179 Pervious Manning 'n'"
180 1.268
181 Pervious SCS Curve No."
182 74.000
183 Pervious SCS"
184 0.255
185 Pervious Runoff coefficient"
186 0.100
186 Impervious SCS Curve No."
187 98.000
188 Impervious SCS"
189 0.832
190 Impervious Runoff coefficient"
191 0.100
191 Impervious Ia/S Coefficient"
192 0.518
192 Impervious Initial abstraction"
193 0.086
194 Pervious Initial abstraction"
195 0.000
196 Pervious Runoff coefficient"
197 0.045
197 Impervious SCS Curve No."
198 98.000
199 Impervious SCS"
200 0.832
201 Impervious Runoff coefficient"
202 0.100
202 Impervious Ia/S Coefficient"
203 0.518
203 Impervious Initial abstraction"
204 0.086
205 Pervious Initial abstraction"
206 0.000
207 Pervious Runoff coefficient"
208 0.045
209 Pervious Manning 'n'"
210 1.268
211 Pervious SCS Curve No."
212 74.000
213 Pervious SCS"
214 0.255
215 Pervious Runoff coefficient"
216 0.100
216 Impervious SCS Curve No."
217 98.000
218 Impervious SCS"
219 0.832
220 Impervious Runoff coefficient"
221 0.100
221 Impervious Ia/S Coefficient"
222 0.518
222 Impervious Initial abstraction"
223 0.086
224 Pervious Initial abstraction"
225 0.000
226 Pervious Runoff coefficient"
227 0.045
228 Pervious Manning 'n'"
229 1.268
230 Pervious SCS Curve No."
231 74.000
232 Pervious SCS"
233 0.255
234 Pervious Runoff coefficient"
235 0.100
235 Impervious SCS Curve No."
236 98.000
237 Impervious SCS"
238 0.832
239 Impervious Runoff coefficient"
240 0.100
239 Impervious Ia/S Coefficient"
241 0.518
241 Impervious Initial abstraction"
242 0.086
243 Pervious Initial abstraction"
244 0.000
245 Pervious Runoff coefficient"
246 0.045
247 Pervious Manning 'n'"
248 1.268
249 Pervious SCS Curve No."
250 74.000
251 Pervious SCS"
252 0.255
253 Pervious Runoff coefficient"
254 0.100
254 Impervious SCS Curve No."
255 98.000
256 Impervious SCS"
257 0.832
258 Impervious Runoff coefficient"
259 0.100
258 Impervious Ia/S Coefficient"
260 0.518
260 Impervious Initial abstraction"
261 0.086
262 Pervious Initial abstraction"
263 0.000
264 Pervious Runoff coefficient"
265 0.045
265 Impervious SCS Curve No."
266 98.000
267 Impervious SCS"
268 0.832
269 Impervious Runoff coefficient"
270 0.100
269 Impervious Ia/S Coefficient"
271 0.518
271 Impervious Initial abstraction"
272 0.086
273 Pervious Initial abstraction"
274 0.000
275 Pervious Runoff coefficient"
276 0.045
277 Pervious Manning 'n'"
278 1.268
279 Pervious SCS Curve No."
280 74.000
281 Pervious SCS"
282 0.255
283 Pervious Runoff coefficient"
284 0.100
284 Impervious SCS Curve No."
285 98.000
286 Impervious SCS"
287 0.832
288 Impervious Runoff coefficient"
289 0.100
288 Impervious Ia/S Coefficient"
290 0.518
290 Impervious Initial abstraction"
291 0.086
292 Pervious Initial abstraction"
293 0.000
294 Pervious Runoff coefficient"
295 0.045
295 Impervious SCS Curve No."
296 98.000
297 Impervious SCS"
298 0.832
299 Impervious Runoff coefficient"
300 0.100
299 Impervious Ia/S Coefficient"
301 0.518
301 Impervious Initial abstraction"
302 0.086
303 Pervious Initial abstraction"
304 0.000
305 Pervious Runoff coefficient"
306 0.045
307 Pervious Manning 'n'"
308 1.268
309 Pervious SCS Curve No."
310 74.000
311 Pervious SCS"
312 0.255
313 Pervious Runoff coefficient"
314 0.100
314 Impervious SCS Curve No."
315 98.000
316 Impervious SCS"
317 0.832
318 Impervious Runoff coefficient"
319 0.100
318 Impervious Ia/S Coefficient"
320 0.518
320 Impervious Initial abstraction"
321 0.086
322 Pervious Initial abstraction"
323 0.000
324 Pervious Runoff coefficient"
325 0.045
325 Impervious SCS Curve No."
326 98.000
327 Impervious SCS"
328 0.832
329 Impervious Runoff coefficient"
330 0.100
329 Impervious Ia/S Coefficient"
331 0.518
331 Impervious Initial abstraction"
332 0.086
333 Pervious Initial abstraction"
334 0.000
335 Pervious Runoff coefficient"
336 0.045
337 Pervious Manning 'n'"
338 1.268
339 Pervious SCS Curve No."
340 74.000
341 Pervious SCS"
342 0.255
343 Pervious Runoff coefficient"
344 0.100
344 Impervious SCS Curve No."
345 98.000
346 Impervious SCS"
347 0.832
348 Impervious Runoff coefficient"
349 0
```

218173- PostDev-Oct2019-5yr.out

[illegible]

Page 9

218173- PostDev-Oct2019-5yr.out

	218173-	PostDev-Oct2019-Syr.out			
423..900	0.05904	1170.100"			
424..000	0.3343	1398.300"			
424..100	0.8324	1636.400"			
424..200	1.475	1884.800"			
Peak outflow	0.033	c.m/sec"			
Maximum level	423..559	metre"			
Maximum storage	465.065	c.m."			
Centroidal lag	7..632	hours"			
0.021	0.033	0.000 c.m/sec"			
HYDROGRAPH "	combine	7"			
5 " combine					
Node #					
Total to Creek"					
Maximum flow	0.152	c.m/sec"			
Hydrograph volume	1451.910	c.m."			
0.021	0.033	0.152"			
START/RE-START Totals On"					
3 Runoff Totals on Exit"					
Total Catchment area					
Total Impervious area					
Total % impervious					
Exit"					
hectare"	21..320				
hectare"	3..692				
	17..317"				

Page 10

```

218173- PostDev-Oct2019-10yr.out
MIDUSS Output Version 2.25 rev. 473
MIDUSS version Sunday, February 07, 2010
Units created C:\Users\jswiger\Desktop\Wildcat Lake\MIDUSS
Job folder: C:\Users\jswiger\Desktop\Wildcat Lake\MIDUSS
Output filename: 218173- PostDev-Oct22-10yr.out
License name: Hewlett-Packard Company
Company: GNR
Date & Time last used: 10/22/2019 at 2:58:31 PM
TIME PARAMETERS
10 10.000 Time step
360.000 Max. Storm length
2400.000 Max. Hydrograph
3 3 STORM Mass Curve
56.400 Rainfall depth
360.000 Duration
47 C:\Program Files (x86)\MIDUSS\SCS_6hr_Type2.mrd SCS 6 hour Type
Maximum intensity 90.044 mm/hr
Total depth 56.400 mm
6 010hyd Hydrograph extension used in this file
CATCHMENT 101
1 Triangular SCS
2 Proportional to %
100- Wildcat Lake South
100 SCS method
100 % Impervious
4.250 Total Area
4.000 Flow length
4.000 Overland Slope
3.825 Pervious Area
70.000 Pervious length
4.000 Pervious slope
0.425 Impervious Area
7.778 Impervious length
4.000 Impervious slope
0.250 Pervious Manning 'n'
0.292 Pervious SCS Curve No.
0.100 Pervious Runoff coefficient
8.924 Pervious Initial abstraction
0.015 Pervious Manning 'n'
98.000 Impervious SCS Curve No.
0.806 Impervious Runoff coefficient
0.100 Impervious Initial abstraction
0.518 Impervious Initial abstraction
0.165 0.000 0.000
Catchment 100 Pervious
Surface Area 3.825 Hectare
Time of concentration 27.484 minutes
Time to centroid 252.653 minutes
Rainfall depth 56.400 mm
Rainfall volume 2157.30 c.m
Rainfall losses 39.936 mm
Runoff depth 16.464 mm
Runoff volume 629.74 c.m
Runoff coefficient 0.292 c.m/sec
Maximum flow 193.29 c.m/sec
HYDROGRAPH Add Runoff 0.133
4 Add Runoff 0.165 0.000 0.000

```

```

218173- PostDev-Oct2019-10yr.out
HYDROGRAPH Copy to Outflow
8 Copy to Outflow 0.165 0.165 0.000"
HYDROGRAPH 6 Combine " Combine 6"
6 Node #
6 To Wildcat Lake"
Maximum flow 0.165 c.m/sec
Hydrograph volume 823.033 c.m
HYDROGRAPH Start - New Tributary"
2 Start - New Tributary" 0.165 0.165 0.165"
0.165 0.000 0.165 0.165"
CATCHMENT 101
1 Triangular SCS
2 Proportional to %
1 SCS method
101- Developed Area South"
30.000 % Impervious
4.890 Total Area
75.000 Flow length
2.000 Overland Slope
3.423 Pervious Area
75.000 Pervious length
2.000 Pervious slope
1.467 Impervious Area
32.143 Impervious length
2.000 Impervious slope
0.250 Pervious Manning 'n'
74.000 Pervious SCS Curve No.
0.292 Pervious Runoff coefficient
8.924 Pervious Initial abstraction
0.015 Pervious Manning 'n'
98.000 Impervious SCS Curve No.
0.884 Impervious Runoff coefficient
0.100 Impervious Initial abstraction
0.518 Impervious Initial abstraction
0.324 0.000 0.165
Catchment 101 Pervious
Surface Area 3.423 Hectare
Time of concentration 35.267 minutes
Time to centroid 262.591 minutes
Rainfall depth 56.400 mm
Rainfall volume 1930.57 c.m
Rainfall losses 39.937 mm
Runoff depth 16.463 mm
Runoff volume 563.53 c.m
Runoff coefficient 0.292 c.m/sec
Maximum flow 731.01 c.m/sec
HYDROGRAPH Add Runoff 0.121
4 Add Runoff 0.324 0.165 0.165"
POND DESIGN"
0.324 Current peak flow c.m/sec
0.080 Target outflow c.m/sec
1294.5 Hydrograph volume c.m
12 Number of stages metre
423.000 Minimum water level metre
425.750 Maximum water level metre
423.000 Starting water level metre
0 Keep Design Data: 1 = True; 0 = False
Level Discharge Volume
Page 2

```

218173- PostDev-Oct2019-10yr.out		218173- PstDev-Oct2019-10yr.out	
423.000	0.000	Maximum flow	0.062
423.250	1.00E-04	HYDROGRAPH Add Runoff "	0.057
423.500	15.300"	4 Add runoff "	0.062
423.750	68.600"		
423.750	0.00030		
424.000	162.000"		
424.000	0.00040		
424.250	284.900"		
424.250	0.00050		
424.500	438.500"		
424.500	0.00060		
424.750	624.800"		
424.750	0.03550		
425.000	845.500"		
425.000	0.03910		
425.250	1102.500"		
425.250	0.04240		
425.500	1397.400"		
425.500	0.04540		
425.750	1731.900"		
425.750	1.114		
425.750	2108.600"		
426.000	0.036		
426.000	424.818		
426.000	metre"		
426.000	915.326		
426.000	c.m."		
426.000	103.601		
426.000	hours"		
426.000	0.165 c.m/sec"		
426.000	0.036		
426.000	6"		
426.000	HYDROGRAPH Combine		
426.000	6		
426.000	Node #"		
426.000	To Wilder Lake"		
426.000	Maximum flow		
426.000	Hydrograph volume		
426.000	0.324		
426.000	1547.189		
426.000	0.036		
426.000	0.178		
426.000	HYDROGRAPH Start - New Tributary"		
426.000	2 Start - New Tributary"		
426.000	0.324		
426.000	0.000		
426.000	0.036		
426.000	0.178"		
426.000	CATCHMENT 102"		
426.000	1 Triangular SCS"		
426.000	2 Proportional to %"		
426.000	102 - developed Area North"		
426.000	SCS method"		
426.000	102 - developed Area North"		
426.000	% Impervious"		
426.000	Total Area"		
426.000	Flow length"		
426.000	Overland Slope"		
426.000	Pervious Area"		
426.000	Pervious length"		
426.000	Pervious slope"		
426.000	Impervious Area"		
426.000	Impervious length"		
426.000	Impervious slope"		
426.000	Pervious Manning 'n'"		
426.000	Pervious SCS Curve No."		
426.000	Pervious Runoff coefficient"		
426.000	Pervious Ia/S coefficient"		
426.000	Pervious Initial abstraction"		
426.000	Impervious Manning 'n'"		
426.000	Impervious SCS Curve No."		
426.000	Impervious Runoff coefficient"		
426.000	Impervious Ia/S coefficient"		
426.000	Impervious Initial abstraction"		
426.000	0.062		
426.000	0.000		
426.000	0.036		
426.000	0.178 c.m/sec"		
426.000	Impervious Total Area "		
426.000	Surface Area		
426.000	Time of concentration		
426.000	25.957		
426.000	12.427		
426.000	minutes"		
426.000	219.622		
426.000	195.198		
426.000	56.400		
426.000	mm		
426.000	56.400		
426.000	35.328		
426.000	mm		
426.000	39.988		
426.000	mm		
426.000	16.412		
426.000	mm		
426.000	103.39		
426.000	mm		



218173- PostDev-Oct2019-10Yr.out									
0.100	Impervious Ia/S coefficient								
0.518	Impervious Initial abstraction"	0.166	0.008	0.178	c.m/sec"				
	Catchment 200	3.747	0.008	0.178	c.m/sec"				
	Surface Area	4.210	0.483	0.178	c.m/sec"				
	Time of Concentration	26.791	0.879	20.174	minutes"				
	Rainfall depth	237.117	194.407	237.117	mm"				
	Rainfall volume	56.400	56.400	56.400	c.m"				
	Rainfall losses	213.25	213.25	213.25	mm"				
	Runoff depth	39.937	10.774	36.747	mm"				
	Runoff volume	16.443	45.636	19.653	c.m"				
	Runoff coefficient	0.292	0.809	0.348	c.m/sec"				
	Maximum flow	0.152	0.096	0.166	c.m/sec"				
40	4 Add Runoff "	0.166	0.166	0.178					
	HYDROGRAPH Copy to Outflow"	0.166	0.008	0.178					
40	8 Copy to Outflow"	0.166	0.166	0.178					
	HYDROGRAPH Start - New Tributary"	0.166	0.166	0.178					
40	6 Combine "	0.166	0.166	0.178					
	7 Node #"	0.166	0.166	0.178					
	Total to Creek"	0.166	0.166	0.178					
	Maximum flow	0.166	0.166	0.178					
	HYDROGRAPH volume	827.392	827.392	827.392	c.m/sec"				
40	HYDROGRAPH Start - New Tributary"	0.166	0.166	0.178					
	2 Start - New Tributary"	0.166	0.166	0.178					
33	CATCHMENT 201	0.166	0.000	0.166					
	1 Triangular SCS"	0.166	0.000	0.166					
	2 Proportional to %"	0.166	0.000	0.166					
	1 SCS method"	0.166	0.000	0.166					
	201	0.166	0.000	0.166					
	20.000	0.166	0.000	0.166					
	45.000	0.166	0.000	0.166					
	2.000	0.166	0.000	0.166					
	45.000	0.166	0.000	0.166					
	2.000	0.166	0.000	0.166					
	0.430	0.166	0.000	0.166					
	11.250	0.166	0.000	0.166					
	2.000	0.166	0.000	0.166					
	0.250	0.166	0.000	0.166					
	74.000	0.166	0.000	0.166					
	Pervious SCS Curve No. "	0.291	0.291	0.291					
	Pervious Ia/S coefficient"	0.100	0.100	0.100					
	Pervious Initial abstraction"	8.924	8.924	8.924					
	Pervious Manning 'n' "	0.015	0.015	0.015					
	Impervious SCS Curve No. "	98.000	98.000	98.000					
	Impervious Runoff coefficient"	0.813	0.813	0.813					
	Impervious Ia/S coefficient"	0.100	0.100	0.100					
	Impervious Initial abstraction"	0.518	0.518	0.518					
	Catchment 400	0.034	0.000	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					
	Runoff volume	16.463	45.857	18.815					
	Runoff coefficient	0.292	0.813	0.813					
	Maximum flow	0.032	0.032	0.032					
	HYDROGRAPH Add Runoff "	0.104	0.104	0.104					
40	4 Add Runoff "	0.034	0.034	0.034					
	HYDROGRAPH Copy to Outflow"	0.034	0.034	0.034					
40	8 Copy to Outflow"	0.034	0.034	0.034					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					
	Runoff volume	16.463	45.857	18.815					
	Runoff coefficient	0.292	0.813	0.813					
	Maximum flow	0.032	0.032	0.032					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
40	4 Add Runoff "	0.034	0.034	0.034					
	HYDROGRAPH Copy to Outflow"	0.034	0.034	0.034					
40	8 Copy to Outflow"	0.034	0.034	0.034					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					
	Runoff volume	16.463	45.857	18.815					
	Runoff coefficient	0.292	0.813	0.813					
	Maximum flow	0.032	0.032	0.032					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
40	4 Add Runoff "	0.034	0.034	0.034					
	HYDROGRAPH Copy to Outflow"	0.034	0.034	0.034					
40	8 Copy to Outflow"	0.034	0.034	0.034					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					
	Runoff volume	16.463	45.857	18.815					
	Runoff coefficient	0.292	0.813	0.813					
	Maximum flow	0.032	0.032	0.032					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
40	4 Add Runoff "	0.034	0.034	0.034					
	HYDROGRAPH Copy to Outflow"	0.034	0.034	0.034					
40	8 Copy to Outflow"	0.034	0.034	0.034					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					
	Runoff volume	16.463	45.857	18.815					
	Runoff coefficient	0.292	0.813	0.813					
	Maximum flow	0.032	0.032	0.032					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
40	4 Add Runoff "	0.034	0.034	0.034					
	HYDROGRAPH Copy to Outflow"	0.034	0.034	0.034					
40	8 Copy to Outflow"	0.034	0.034	0.034					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					
	Runoff volume	16.463	45.857	18.815					
	Runoff coefficient	0.292	0.813	0.813					
	Maximum flow	0.032	0.032	0.032					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
40	4 Add Runoff "	0.034	0.034	0.034					
	HYDROGRAPH Copy to Outflow"	0.034	0.034	0.034					
40	8 Copy to Outflow"	0.034	0.034	0.034					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					
	Runoff volume	16.463	45.857	18.815					
	Runoff coefficient	0.292	0.813	0.813					
	Maximum flow	0.032	0.032	0.032					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
40	4 Add Runoff "	0.034	0.034	0.034					
	HYDROGRAPH Copy to Outflow"	0.034	0.034	0.034					
40	8 Copy to Outflow"	0.034	0.034	0.034					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					
	Runoff volume	16.463	45.857	18.815					
	Runoff coefficient	0.292	0.813	0.813					
	Maximum flow	0.032	0.032	0.032					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
40	4 Add Runoff "	0.034	0.034	0.034					
	HYDROGRAPH Copy to Outflow"	0.034	0.034	0.034					
40	8 Copy to Outflow"	0.034	0.034	0.034					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					
	Runoff volume	16.463	45.857	18.815					
	Runoff coefficient	0.292	0.813	0.813					
	Maximum flow	0.032	0.032	0.032					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
40	4 Add Runoff "	0.034	0.034	0.034					
	HYDROGRAPH Copy to Outflow"	0.034	0.034	0.034					
40	8 Copy to Outflow"	0.034	0.034	0.034					
	HYDROGRAPH Add Runoff "	0.034	0.034	0.034					
	Surface Area	0.911	0.911	0.911					
	Time of Concentration	35.267	35.267	35.267					
	Rainfall depth	262.591	262.591	262.591					
	Rainfall volume	56.400	56.400	56.400					
	Rainfall losses	213.25	213.25	213.25					
	Runoff depth	39.937	10.774	36.747					

	218173 - PostDay-Oct-2019 -10yr.out	
0.100 0.518	Impervius Ia/S coefficient Impervius Initial abstraction"	"
	0.166 0.000 Pervious	Impervius Total Area "
Catchment 200	Surface Area	3.747 0.463 4.210
	Time of concentration	26.791 0.879 20.174
	Time to Centroid	251.765 194.407 237.117
	Rainfall depth	56.400 56.400 56.400
	Rainfall volume	2113.25 261.19 2374.44
	Rainfall losses	39.957 10.774 36.747
	Runoff depth	16.443 4.656 19.653
	Runoff volume	616.10 211.29 827.39
	Runoff coefficient	0.292 0.809 0.348
	Maximum flow	0.152 0.096 C.m/sec"
40	HYDROGRAPH Add Runoff "	
	4 Add Runoff "0.166 0.166 0.008 0.178"	
40	HYDROGRAPH Copy to Outflow"	
	8 Copy to Outflow"0.166 0.166 0.166 0.178"	
40	HYDROGRAPH " Combine 7"	
	6 Combine " Node.#"	
	7 Total to Creek"	
	Maximum flow	0.166 C.m/sec"
	Hydrograph volume	827.392 C.m."
	0.166 0.166 0.163 0.166"	
40	HYDROGRAPH Start - New Tributary"	
	2 Start - New Tributary"0.166 0.166 0.166 0.166"	
33	CATCHMENT 201	
	1 Triangular SCS"	
	2 Proportional to %"	
	1 SCS method"	
	201	
	% Impervious"	
	2.150 Total Area"	
	45.000 Flow length"	
	2.000 Overland Slope"	
	1.720 Pervious Area"	
	45.000 Pervious length"	
	2.000 Pervious slope"	
	0.430 Imperivous Area"	
	11.250 Imperivous length"	
	2.000 Imperivous slope"	
	0.250 Pervious Manning "n"	
	74.000 Pervious SCS Curve No."	
	0.291 Pervious Runoff coefficient"	
	0.100 Pervious Ia/S coefficient"	
	8.924 Pervious Initial abstraction"	
	0.015 Imperivous Manning "n"	
	98.000 Imperivous SCS Curve No."	
	0.842 Imperivous Runoff coefficient"	
	0.100 Imperivous Ia/S coefficient"	
0.518	Impervius initial abstraction"	
	0.104 0.000 Pervious	Impervius Total Area "
Catchment 201	Surface Area	1.720 0.430 7.150
	Time of concentration	25.937 1.300 15.610
	Time to Centroid	250.709 195.141 227.391
	Rainfall depth	56.400 56.400 56.400
	Rainfall volume	970.08 242.52 1212.60
	Rainfall losses	39.988 8.936 33.778
		Page 5



[illegible]

```

218173- PostDev-Oct2019-25yr.out
-----
MIDUSS Output
MIDUSS version Version 2.25 rev. 473
MIDUSS created Sunday, February 07, 2010
Units used: C:\Users\jswtiger\Desktop\Wildcat Lake\MIDUSS\
Job folder: C:\Users\jswtiger\Desktop\Wildcat Lake\MIDUSS\
Output filename: 218173- PostDev-Oct22-25yr.out
Licensee name: Hewlett-Packard Company
Company: GMBP
Date & Time last used: 10/22/2019 at 2:57:05 PM

TIME PARAMETERS
31 10.000 Time Step
360.000 Max. storm length"
2400.000 Max. Hydrograph"
32 STORM Mass Curve"
3 65.300 Rainfall depth"
360.000 Duration"
47 C:\Program Files (x86)\MIDUSS\SCS_6hr_Type2.mrd SCS 6 hour Type

Maximum intensity 104.253 mm/hr"
Total depth 65.300 mm"
6 025hyd Hydrograph extension used in this file"
CATCHMENT 100
1 Triangular SCS"
2 Proportional to %"
100 100- Wilder Lake South"
1 SCS method"
4.250 % Impervious"
70.000 Total Area"
4.000 Flow length"
3.825 Overland Slope"
70.000 Pervious Area"
4.000 Pervious length"
4.000 Pervious slope"
0.425 Impervious Area"
7.778 Impervious length"
4.000 Impervious slope"
0.250 Pervious Manning "n"
0.333 Pervious SCS Curve No."
0.100 Pervious Runoff coefficient"
8.924 Pervious Ia/S coefficient"
0.015 Pervious Initial abstraction"
98.000 Impervious Manning "n"
0.891 Impervious SCS Curve No."
0.100 Impervious Runoff coefficient"
0.518 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"

Catchment 100
Surface Area 3.825 Hectare"
Time of concentration 24.325 minutes"
Time to Centroid 247.284 minutes"
Rainfall depth 65.300 mm"
Rainfall volume 2497.73 C.m"
Rainfall losses 43.552 mm"
Runoff depth 21.748 mm"
Runoff volume 831.86 C.m"
Runoff coefficient 0.333 C.m/sec"
Maximum flow 0.225 C.m/sec"
HYDROGRAPH Add Runoff "
4 Add Runoff" 0.239 0.000 0.000"

```

```

218173- PostDev-Oct2019-25yr.out
-----
HYDROGRAPH Copy to outflow"
8 Copy to outflow" 0.239 0.239 0.000"
HYDROGRAPH " Combine 6"
6 Combine
6 Node #
Maximum flow 0.239 C.m/sec"
Hydrograph volume 1057.956 C.m"
0.239 0.239 0.239"
HYDROGRAPH Start - New Tributary"
2 Start - New Tributary" 0.239 0.239 0.239"
CATCHMENT 101"
1 Triangular SCS"
2 Proportional to %"
101 101 - Developed Area South"
30.000 % Impervious"
4.890 Total Area"
75.000 Flow length"
2.000 Overland Slope"
3.423 Pervious Area"
75.000 Pervious length"
2.000 Pervious slope"
1.467 Impervious Area"
32.143 Impervious length"
2.000 Impervious slope"
0.250 Pervious Manning "n"
0.333 Pervious SCS Curve No."
0.100 Pervious Runoff coefficient"
8.924 Pervious Ia/S coefficient"
0.015 Pervious Initial abstraction"
98.000 Impervious Manning "n"
0.891 Impervious SCS Curve No."
0.100 Impervious Runoff coefficient"
0.518 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"

Catchment 101
Surface Area 3.423 Hectare"
Time of concentration 31.213 minutes"
Time to Centroid 256.305 minutes"
Rainfall depth 65.300 mm"
Rainfall volume 2835.22 C.m"
Rainfall losses 43.520 mm"
Runoff depth 21.780 mm"
Runoff volume 745.52 C.m"
Runoff coefficient 0.334 C.m/sec"
Maximum flow 0.174 C.m/sec"
HYDROGRAPH Add Runoff "
4 Add Runoff" 0.388 0.000 0.239"
Pervious 1.467
Impervious 4.890
Total Area 15.776
Current peak flow 195.371 C.m/sec"
Target outflow 223.777 C.m/sec"
Hydrograph volume 65.300 C.m"
Number of stages 3193.17 metre"
Minimum water level 7.104 metre"
Maximum water level 32.705 metre"
Starting water level 853.73 metre"
Keep Design Data: 1 = True; 0 = False"
Level Discharge Volume"
0 0.388 0.239 0.239"

```

218173- PostDev-Oct2019-25yr.out									
40	423.000	1.00E-04	0.000"	0.000"	Maximum flow	218173- PostDev-Oct2019-25yr.out	0.075	0.067	c.m/sec"
40	423.250	0.00020	15.300"	0.000"	HYDROGRAPH Add	Runoff "0.038	0.067	0.075	c.m/sec"
40	423.500	0.00030	68.600"	0.000"	4 Add	Runoff "	0.075	0.040	0.256"
40	423.750	0.00040	162.000"	0.000"	POND DESIGN				
40	424.000	0.00050	284.900"	0.000"	0.075	Current peak flow	c.m/sec"		
40	424.250	0.00060	438.500"	0.000"	0.030	Target outflow	c.m/sec"		
40	424.500	0.00070	624.800"	0.000"	291.0	Hydrograph volume	c.m"		
40	424.750	0.00080	845.500"	0.000"	11.	Number of stages	metre"		
40	425.000	0.00090	1102.500"	0.000"	423.200	Minimum water level	metre"		
40	425.250	0.00100	1307.500"	0.000"	424.850	Maximum water level	metre"		
40	425.500	0.00110	1531.900"	0.000"	423.200	Starting water level	metre"		
40	425.750	0.00120	1751.900"	0.000"	0	Keep Design Data: 1 = True; 0 = False"			
40	426.000	0.00130	2108.600"	0.000"	Level Discharge	Volume			
40	426.250	0.00140	2508.600"	0.000"	423.350	0.000			
40	426.500	0.00150	2958.600"	0.000"	423.500	1.00E-05			
40	426.750	0.00160	3458.600"	0.000"	423.650	2.00E-05			
40	427.000	0.00170	3958.600"	0.000"	423.800	3.00E-05			
40	427.250	0.00180	4458.600"	0.000"	423.950	4.00E-05			
40	427.500	0.00190	4958.600"	0.000"	424.100	5.00E-05			
40	427.750	0.00200	5458.600"	0.000"	424.250	6.00E-05			
40	428.000	0.00210	5958.600"	0.000"	424.400	0.01320			
40	428.250	0.00220	6458.600"	0.000"	424.550	0.01548			
40	428.500	0.00230	6958.600"	0.000"	424.700	0.01746			
40	428.750	0.00240	7458.600"	0.000"	424.850	0.01944			
40	429.000	0.00250	7958.600"	0.000"	425.000	0.02142			
40	429.250	0.00260	8458.600"	0.000"	425.150	0.02340			
40	429.500	0.00270	8958.600"	0.000"	425.300	0.02538			
40	429.750	0.00280	9458.600"	0.000"	425.450	0.02736			
40	430.000	0.00290	9958.600"	0.000"	425.600	0.02934			
40	430.250	0.00300	10458.600"	0.000"	425.750	0.03132			
40	430.500	0.00310	10958.600"	0.000"	425.900	0.03330			
40	430.750	0.00320	11458.600"	0.000"	426.050	0.03528			
40	431.000	0.00330	11958.600"	0.000"	426.200	0.03726			
40	431.250	0.00340	12458.600"	0.000"	426.350	0.03924			
40	431.500	0.00350	12958.600"	0.000"	426.500	0.04122			
40	431.750	0.00360	13458.600"	0.000"	426.650	0.04320			
40	432.000	0.00370	13958.600"	0.000"	426.800	0.04518			
40	432.250	0.00380	14458.600"	0.000"	426.950	0.04716			
40	432.500	0.00390	14958.600"	0.000"	427.100	0.04914			
40	432.750	0.00400	15458.600"	0.000"	427.250	0.05112			
40	433.000	0.00410	15958.600"	0.000"	427.400	0.05310			
40	433.250	0.00420	16458.600"	0.000"	427.550	0.05508			
40	433.500	0.00430	16958.600"	0.000"	427.700	0.05706			
40	433.750	0.00440	17458.600"	0.000"	427.850	0.05904			
40	434.000	0.00450	17958.600"	0.000"	428.000	0.06102			
40	434.250	0.00460	18458.600"	0.000"	428.150	0.06300			
40	434.500	0.00470	18958.600"	0.000"	428.300	0.06498			
40	434.750	0.00480	19458.600"	0.000"	428.450	0.06696			
40	435.000	0.00490	19958.600"	0.000"	428.600	0.06894			
40	435.250	0.00500	20458.600"	0.000"	428.750	0.07092			
40	435.500	0.00510	20958.600"	0.000"	428.900	0.07290			
40	435.750	0.00520	21458.600"	0.000"	429.050	0.07488			
40	436.000	0.00530	21958.600"	0.000"	429.200	0.07686			
40	436.250	0.00540	22458.600"	0.000"	429.350	0.07884			
40	436.500	0.00550	22958.600"	0.000"	429.500	0.08082			
40	436.750	0.00560	23458.600"	0.000"	429.650	0.08280			
40	437.000	0.00570	23958.600"	0.000"	429.800	0.08478			
40	437.250	0.00580	24458.600"	0.000"	429.950	0.08676			
40	437.500	0.00590	24958.600"	0.000"	430.100	0.08874			
40	437.750	0.00600	25458.600"	0.000"	430.250	0.09072			
40	438.000	0.00610	25958.600"	0.000"	430.400	0.09270			
40	438.250	0.00620	26458.600"	0.000"	430.550	0.09468			
40	438.500	0.00630	26958.600"	0.000"	430.700	0.09666			
40	438.750	0.00640	27458.600"	0.000"	430.850	0.09864			
40	439.000	0.00650	27958.600"	0.000"	431.000	0.10062			
40	439.250	0.00660	28458.600"	0.000"	431.150	0.10260			
40	439.500	0.00670	28958.600"	0.000"	431.300	0.10458			
40	439.750	0.00680	29458.600"	0.000"	431.450	0.10656			
40	440.000	0.00690	29958.600"	0.000"	431.600	0.10854			
40	440.250	0.00700	30458.600"	0.000"	431.750	0.11052			
40	440.500	0.00710	30958.600"	0.000"	431.900	0.11250			
40	440.750	0.00720	31458.600"	0.000"	432.050	0.11448			
40	441.000	0.00730	31958.600"	0.000"	432.200	0.11646			
40	441.250	0.00740	32458.600"	0.000"	432.350	0.11844			
40	441.500	0.00750	32958.600"	0.000"	432.500	0.12042			
40	441.750	0.00760	33458.600"	0.000"	432.650	0.12240			
40	442.000	0.00770	33958.600"	0.000"	432.800	0.12438			
40	442.250	0.00780	34458.600"	0.000"	432.950	0.12636			
40	442.500	0.00790	34958.600"	0.000"	433.100	0.12834			
40	442.750	0.00800	35458.600"	0.000"	433.250	0.13032			
40	443.000	0.00810	35958.600"	0.000"	433.400	0.13230			
40	443.250	0.00820	36458.600"	0.000"	433.550	0.13428			
40	443.500	0.00830	36958.600"	0.000"	433.700	0.13626			
40	443.750	0.00840	37458.600"	0.000"	433.850	0.13824			
40	444.000	0.00850	37958.600"	0.000"	434.000	0.14022			
40	444.250	0.00860	38458.600"	0.000"	434.150	0.14220			
40	444.500	0.00870	38958.600"	0.000"	434.300	0.14418			
40	444.750	0.00880	39458.600"	0.000"	434.450	0.14616			
40	445.000	0.00890	39958.600"	0.000"	434.600	0.14814			
40	445.250	0.00900	40458.600"	0.000"	434.750	0.15012			
40	445.500	0.00910	40958.600"	0.000"	434.900	0.15210			
40	445.750	0.00920	41458.600"	0.000"	435.050	0.15408			
40	446.000	0.00930	41958.600"	0.000"	435.200	0.15606			
40	446.250	0.00940	42458.600"	0.000"	435.350	0.15804			
40	446.500	0.00950	42958.600"	0.000"	435.500	0.16002			
40	446.750	0.00960	43458.600"	0.000"	435.650	0.16200			
40	447.000	0.00970	43958.600"	0.000"	435.800	0.16398			
40	447.250	0.00980	44458.600"	0.000"	435.950	0.16596			
40	447.500	0.00990	44958.600"	0.000"	436.100	0.16794			
40	447.750	0.01000	45458.600"	0.000"	436.250	0.16992			
40	448.000	0.01010	45958.600"	0.000"	436.400	0.17190			
40	448.250	0.01020	46458.600"	0.000"	436.550	0.17388			
40	448.500	0.01030	46958.600"	0.000"	436.700	0.17586			
40	448.750	0.01040	47458.600"	0.000"	436.850	0.17784			
40	449.000	0.01050	47958.600"	0.000"	437.000	0.17982			
40	449.250	0.01060	48458.600"	0.000"	437.150	0.18180			
40	449.500	0.01070	48958.600"	0.000"	437.300	0.18378			
40	449.750	0.01080	49458.600"	0.000"	437.450	0.18576			
40	450.000	0.01090	49958.600"	0.000"	437.600	0.18774			
40	450.250	0.01100	50458.600"	0.000"	437.750	0.18972			
40	450.500	0.01110	50958.600"	0.000"	437.900	0.19170			
40	450.750	0.01120	51458.600"	0.000"	438.050	0.19368			
40	451.000	0.01130	51958.600"	0.000"	438.200	0.19566			
40	451.250	0.01140	52458.600"	0.000"	438.350	0.19764			
40	451.500	0.01150	52958.600"	0.000"	438.500	0.19962			
40	451.750	0.01160	53458.600"	0.000"	438.650	0.20160			
40	452.000	0.01170	53958.600"	0.000"	438.800	0.20358			
40	452.250	0.01180	54458.600"	0.000"	438.950	0.20556			
40	452.500	0.01190	54958.600"	0.000"	439.100	0.20754			
40	452.750	0.01200	55458.600"	0.000"	439.250	0.20952			
40	453.000	0.01210	55958.600"	0.000"	439.400	0.21150			
40	453.250	0.01220	56458.600"	0.000"	439.550	0.21348			
40	453.500	0.01230	56958.600"	0.000"	439.700	0.21546			
40	453.750	0.01240	57458.600"	0.000"	439.850	0.21744			
40	454.000	0.01250	57958.600"	0.000"	440.000	0.21942			
40	454.250	0.01260	58458.600"	0.000"	440.150	0.22140			
40	454.500	0.01270	58958.600"	0.0					







218173- PostDev-Oct2019-25yr.out

30.000	Pervious length"	0.042	0.000	0.266 c.m/sec"	0.266 c.m/sec"
4.000	Pervious slope"	0.000	0.000	0.000	0.000
0.000	Impervious Area"	0.000	0.000	0.000	0.000
0.000	Impervious length"	0.000	0.000	0.000	0.000
4.000	Impervious slope"	0.000	0.000	0.000	0.000
0.250	Pervious Manning "n"	0.000	0.000	0.000	0.000
74.000	Pervious SCS Curve No."	0.000	0.000	0.000	0.000
0.332	Pervious Runoff coefficient"	0.000	0.000	0.000	0.000
0.100	Pervious Ia/S coefficient"	0.000	0.000	0.000	0.000
8.324	Pervious Initial abstraction"	0.000	0.000	0.000	0.000
0.015	Impervious Manning "n"	0.000	0.000	0.000	0.000
98.000	Impervious SCS Curve No."	0.000	0.000	0.000	0.000
0.000	Impervious Runoff coefficient"	0.000	0.000	0.000	0.000
0.000	Impervious Ia/S coefficient"	0.000	0.000	0.000	0.000
0.518	Impervious Initial abstraction"	0.000	0.000	0.000	0.000
	Catchment 600	0.042	0.130	0.266 c.m/sec"	0.266 c.m/sec"
	Surface Area	0.550	0.000	0.550	0.550
	Time of concentration	14.630	0.000	14.630	14.630
	Time to centroid	234.956	0.000	234.956	234.956
	Rainfall depth	65.300	191.157	65.300	65.300
	Rainfall volume	359.15	0.00	359.15	359.15
	Rainfall losses	43.593	0.00	43.593	43.593
	Runoff depth	21.707	53.980	21.707	21.707
	Runoff volume	119.39	0.00	119.39	119.39
	Runoff coefficient	0.332	0.000	0.332	0.332
	Maximum flow	0.042	0.000	0.042	0.042
40	HYDROGRAPH Add Runoff "	0.042	0.130	0.266"	0.266"
	4 " Add Runoff "	0.042	0.130	0.266"	0.266"
40	HYDROGRAPH Copy to outflow"	0.042	0.042	0.266"	0.266"
	8 " Copy to outflow"	0.042	0.042	0.266"	0.266"
40	HYDROGRAPH " Combine 10"	0.042	0.042	0.266"	0.266"
	6 " Combine " Combine 10"	0.042	0.042	0.266"	0.266"
10	Node #"	0.042	0.042	0.266"	0.266"
	To Pond 201"	0.042	0.042	0.266"	0.266"
	Maximum flow	0.042	0.042	0.266"	0.266"
	Hydrograph volume	0.042	0.042	0.266"	0.266"
40	HYDROGRAPH " Confluence 10"	0.042	0.042	0.266"	0.266"
	7 " Confluence " Confluence 10"	0.042	0.042	0.266"	0.266"
10	Node #"	0.042	0.042	0.266"	0.266"
	To Pond 201"	0.042	0.042	0.266"	0.266"
	Maximum flow	0.042	0.042	0.266"	0.266"
	Hydrograph volume	0.042	0.042	0.266"	0.266"
54	POND DESIGN	0.042	0.042	0.266"	0.266"
	0.304 Current peak flow	0.042	0.042	0.266"	0.266"
	0.100 Target outflow	0.042	0.042	0.266"	0.266"
1239.6	Hydrograph volume	0.042	0.042	0.266"	0.266"
10.	Number of stages"	0.042	0.042	0.266"	0.266"
423.300	Minimum water level	0.042	0.042	0.266"	0.266"
424.500	Maximum water level	0.042	0.042	0.266"	0.266"
423.300	Starting water level	0.042	0.042	0.266"	0.266"
0	Keep Design Data: 1 = True; 0 = False"	0.042	0.042	0.266"	0.266"
	Level Discharge	0.042	0.042	0.266"	0.266"
	423.300 0.000	0.042	0.042	0.266"	0.266"
	423.400 0.01130	0.042	0.042	0.266"	0.266"
	423.500 0.02640	0.042	0.042	0.266"	0.266"
	423.600 0.03737	0.042	0.042	0.266"	0.266"
	423.700 0.04577	0.042	0.042	0.266"	0.266"
	423.800 0.05281	0.042	0.042	0.266"	0.266"

218173- PostDev-Oct2019-25yr.out

423.900	0.05904	1170.100"	0.046	0.046	c.m/sec"
424.000	0.3343	1398.300"	0.046	0.046	metre"
424.100	0.8324	1636.400"	0.046	0.046	c.m"
424.200	1.475	1884.800"	0.046	0.046	hours"
	Peak outflow	0.046	0.046	0.000 c.m/sec"	0.000 c.m/sec"
	Maximum level	423.707	0.046	0.046	metre"
	Maximum Storage	758.582	0.046	0.046	c.m"
	Centroidal lag	7	0.046	0.046	hours"
40	HYDROGRAPH " Combine	0.304	0.046	0.000 c.m/sec"	0.000 c.m/sec"
	6 " Combine "	0.304	0.046	0.000 c.m/sec"	0.000 c.m/sec"
	7 " Node #"	0.304	0.046	0.000 c.m/sec"	0.000 c.m/sec"
	Total to Creek"	0.304	0.046	0.000 c.m/sec"	0.000 c.m/sec"
	Maximum flow	0.304	0.046	0.000 c.m/sec"	0.000 c.m/sec"
	Hydrograph volume	2301.263	0.046	0.000 c.m/sec"	0.000 c.m/sec"
38	START/RE-START TOTALS 10"	0.304	0.046	0.000 c.m/sec"	0.000 c.m/sec"
	3 " Runoff Totals on EXIT"	0.304	0.046	0.000 c.m/sec"	0.000 c.m/sec"
	Total Catchment area	21.320	0.304	21.320	21.320
	Total Impervious area	3.692	0.304	3.692	3.692
19	Total % impervious	17.317"	0.304	17.317"	17.317"
	EXIT"				

```

218173- PostDev-Oct2019-50yr.out
-----
MIDUSS Output
MIDUSS version Version 2.25 rev. 473
Units created Sunday, February 07, 2010
Job Folder: C:\Users\jswiger\Desktop\wilder Lake\
Output filename: 218173- PostDev-Oct22-50yr.out.
Licensee name: Company: Hewlett-Packard Company
Date & time last used: 10/22/2019 at 2:55:20 PM
TIME PARAMETERS
10 10.000 Time Step
360.000 Max. Storm Length
2400.000 Max. Hydrograph
3 STORM Mass Curve
3 71.900 Rainfall depth
360.000 Duration
47 C:\Program Files (x86)\MIDUSS\SCS_6hr_Type2.mrd SCS 6 hour Type
Maximum intensity 114.790 mm/hr
Total depth 71.900 mm
6 050hvd Hydrograph extension used in this file
CATCHMENT 100
1 Triangular SCS
2 Proportional to %
100 SCS method
100- Wilder Lake South
4.250 Total Area
4.000 Flow Length
4.000 Overland Slope
3.825 Pervious Area
70.000 Pervious length
4.000 Pervious slope
0.425 Impervious Area
7.778 Impervious length
4.000 Impervious slope
0.250 Pervious Manning 'n'
74.000 Pervious SCS Curve No.
0.362 Pervious Runoff coefficient
0.100 Pervious Ia/S coefficient
8.924 Pervious Initial abstraction
0.015 Pervious Manning 'n'
98.000 Impervious SCS Curve No.
0.895 Impervious Runoff coefficient
0.100 Impervious Ia/S coefficient
0.518 Impervious Initial abstraction
Catchment 100
Surface Area 3.825
Time of concentration 22.541
Time to centroid 244.098
Rainfall depth 71.900
Rainfall volume 2750.18
Rainfall losses 45.897
Runoff depth 26.003
Runoff volume 994.63
Runoff coefficient 0.362
Maximum flow 0.284
HYDROGRAPH Add Runoff
4 Add Runoff 0.299 0.000 0.000

```

```

218173- PostDev-Oct2019-50yr.out
-----
HYDROGRAPH Copy to outflow"
8 Copy to outflow 0.299 0.299 0.000"
HYDROGRAPH " Combine 6" 0.299 0.000"
6 Combine
6 Node #
To Wilder Lake"
Maximum flow 0.299 c.m./sec"
Hydrograph volume 1245.016 c.m."
0.299 0.299 0.299"
HYDROGRAPH start - New Tributary"
2 Start - New Tributary" 0.299 0.299 0.299"
0.299 0.000
CATCHMENT 101"
1 Triangular SCS
2 Proportional to %
1 SCS method"
101 101 - Developed Area South"
30.000 % Impervious
4.890 Total Area"
75.000 Flow length"
2.000 Overland Slope"
3.423 Pervious Area"
75.000 Pervious length"
2.000 Pervious slope"
1.467 Impervious Area"
32.143 Impervious length"
2.000 Impervious slope"
0.250 Pervious Manning 'n'"
74.000 Pervious SCS Curve No."
0.362 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning 'n'"
98.000 Impervious SCS Curve No."
0.895 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.439 0.000 0.299
Catchment 101 Pervious 0.299 c.m./sec"
Surface Area 3.423 Impervious 1.467 hectare"
Time of concentration 28.924 2.205 minutes"
Time to centroid 252.595 194.695 minutes"
Rainfall depth 71.900 71.900 mm"
Rainfall volume 2460.14 105.77 c.m."
Rainfall losses 45.303 7.517 mm"
Runoff depth 25.997 64.383 mm"
Runoff volume 889.88 944.50 c.m."
Runoff coefficient 0.362 0.895 c.m./sec"
Maximum flow 0.220 0.398 c.m./sec"
HYDROGRAPH Add Runoff "
4 Add Runoff 0.439 0.299 0.299"
POND DESIGN"
0.439 Current peak flow c.m./sec"
0.080 Target outflow c.m./sec"
1834.4 Hydrograph volume c.m."
12. Number of stages" metre"
423.000 Minimum water level metre"
425.750 Maximum water level metre"
423.000 Starting water level metre"
0 Keep Design data: 1 = True; 0 = False"
Level Discharge Volume"
Page 2

```

218173- PostDev-Oct2019-50yr.out

423.000	0.000	0.000	
423.250	1.00E-04	15.300	
423.500	0.00020	68.600	
423.750	0.00030	162.000	
424.000	0.00040	284.900	
424.250	0.00050	438.500	
424.500	0.00060	624.800	
424.750	0.00350	845.500	
425.000	0.03310	1102.500	
425.250	0.04240	1397.400	
425.500	0.04540	1731.900	
425.750	1.114	2108.600	
Peak outflow	0.042	C.m/sec	
Maximum level	425.195	metre	
Maximum storage	1332.223	C.m	
Centroidal lag	76.929	hours	
0.439	0.439	0.299 C.m/sec	
HYDROGRAPH "Combine	6"		
6 "Combine "			
6 "Node #"			
To Wilder Lake			
Maximum flow	0.333	C.m/sec	
Hydrograph volume	2502.071	C.m	
0.439	0.439	0.333	
HYDROGRAPH Start - New Tributary			
2 "Start - New Tributary"	0.000	0.042	0.333
CATCHMENT 102			
1 "Triangular SCS"			
2 "Proportional to %"			
102 - Developed Area North"			
30.000	% Impervious		
0.900	Total Area		
45.000	Flow length		
2.000	Overland Slope		
0.630	Pervious Area		
45.000	Pervious length		
2.000	Pervious slope		
0.270	Impervious Area		
19.286	Impervious length		
2.000	Impervious slope		
0.250	Pervious Manning "n"		
74.000	Pervious SCS Curve No. "		
0.361	Pervious Runoff coefficient		
0.100	Pervious Ia/S coefficient		
8.924	Pervious Initial abstraction		
0.015	Impervious Manning "n"		
98.000	Impervious SCS Curve No. "		
0.877	Impervious Runoff coefficient		
0.100	Impervious Ia/S coefficient		
0.518	Impervious Initial abstraction		
0.086	0.000	0.042	0.333 C.m/sec
Catchment 102			
Surface Area	Pervious	Imperious	Total Area "
Time of concentration	0.630	0.270	0.900
Time to centroid	21.289	1.623	11.265
Rainfall depth	242.461	193.925	minutes
Rainfall volume	71.900	71.900	mm
Rainfall losses	452.97	194.13	mm
Runoff depth	45.913	34.798	C.m
Runoff volume	25.987	63.038	mm
Runoff coefficient	163.72	333.92	C.m
	0.361	0.877	0.516

218173- PostDev-Oct2019-50yr.out

Maximum flow	0.048	0.074	0.086	C.m/sec
HYDROGRAPH Add Runoff "				
4 "Add Runoff "	0.086	0.086	0.333	
POND DESIGN				
0.086	Current peak flow	C.m/sec		
0.030	Target outflow	C.m/sec		
333.9	Hydrograph volume	C.m		
11.	Number of stages			
423.200	Minimum water level	metre		
424.850	Minimum water level	metre		
423.200	Starting water level	metre		
0	Keep Design data: 1 = True; 0 = False			
Level Discharge	Volume			
423.350	0.000			
423.500	1.00E-05	9.800		
423.650	2.00E-05	24.900		
423.800	3.00E-05	45.600		
423.950	4.00E-05	72.300		
424.100	5.00E-05	105.600		
424.250	6.00E-05	145.700		
424.400	0.01320	193.300		
424.550	0.01548	248.600		
424.700	0.01746	312.100		
424.850	0.0145	384.400		
Peak outflow	0.014	C.m/sec		
Maximum level	424.471	metre		
Maximum storage	219.526	C.m		
Centroidal lag	6.033	hours		
0.086	0.086	0.014	0.333 C.m/sec	
HYDROGRAPH "Combine	6"			
6 "Combine "				
6 "Node #"				
To Wilder Lake				
Maximum flow	0.339	C.m/sec		
Hydrograph volume	2696.717	C.m		
0.086	0.086	0.014	0.339	
HYDROGRAPH Start - New Tributary				
2 "Start - New Tributary"	0.000	0.014	0.339	
CATCHMENT 200				
1 "Triangular SCS"				
2 "Proportional to %"				
1 "SCS method"				
200 - Creek				
11.000	% Impervious			
4.210	Total Area			
75.000	Flow length			
5.000	Overland Slope			
3.747	Pervious Area			
75.000	Pervious length			
5.000	Pervious slope			
0.463	Impervious Area			
9.270	Impervious length			
5.000	Impervious slope			
0.250	Pervious Manning "n"			
74.000	Pervious SCS Curve No. "			
0.362	Pervious Runoff coefficient			
0.100	Pervious Ia/S coefficient			
8.924	Pervious Initial abstraction			
0.015	Impervious Manning "n"			
98.000	Impervious SCS Curve No. "			
0.821	Impervious Runoff coefficient			

218173- PostDev-Oct2019-50yr.out									
0.100	Impervious Ia/S coefficient"								
0.518	Impervious Initial abstraction"								
	Catchment 200	0.000	0.014	0.339	c.m/sec"				
	Surface Area	3,747	0.463	4,210	hectare"				
	Time of concentration	21,972	0.795	17,333	minutes"				
	Rainfall depth	243,363	192,770	232,279	mm"				
	Rainfall volume	71,900	71,900	71,900	c.m"				
	Rainfall losses	2694.02	332.97	3026.99	mm"				
	Runoff depth	42,893	32,868	42,860	mm"				
	Runoff volume	25,007	35,032	25,640	c.m"				
	Runoff coefficient	974.47	273.38	1247.85	c.m/sec"				
	Maximum flow	0.362	0.821	0.412	c.m/sec"				
	HYDROGRAPH Add, Runoff "	0.282	0.123	0.299					
40	4 Add Runoff "	0.299	0.299	0.014	0.339"				
40	8 HYDROGRAPH Copy to Outflow"	0.299	0.299	0.299	0.339"				
40	6 HYDROGRAPH " Combine 7"	0.299	0.299	0.339"					
40	2 HYDROGRAPH Start - New Tributary"	0.299	0.299	0.299	0.299"				
33	1 CATCHMENT 201	0.299	0.000	0.299	0.299"				
	1 Triangular SCS								
	Proportional to %"								
	201								
	20,000 % Impervious"								
	2,150 Total Area"								
	45,000 Flow length"								
	2,000 Overland Slope"								
	1,720 Pervious Area"								
	45,000 Pervious length"								
	2,000 Pervious slope"								
	11,250 Impervious Area"								
	7,000 Impervious length"								
	0.250 Pervious Manning "n"								
	74,000 Pervious SCS Curve No."								
	0.361 Pervious Runoff coefficient"								
	0.100 Pervious Ia/S coefficient"								
	8,924 Pervious Initial abstraction"								
	0.015 Impervious Manning "n"								
	98,000 Impervious SCS Curve No."								
	0.850 Impervious Runoff coefficient"								
	0.100 Impervious Ia/S coefficient"								
	0.518 Impervious Initial abstraction"								
	Catchment 201	0.168	0.000	0.299	0.299 c.m/sec"				
	Surface Area	1,720	0.430	2,150	hectare"				
	Time of concentration	21,289	1,175	13,843	minutes"				
	Rainfall depth	242,462	193,655	224,394	mm"				
	Rainfall volume	71,900	71,900	71,900	c.m"				
	Rainfall losses	1236.68	309.17	1545.85	mm"				
	Runoff depth	45,913	10,804	38,891	c.m/sec"				
	Runoff volume	25,007	35,032	25,640	c.m"				
	Runoff coefficient	974.47	273.38	1247.85	c.m/sec"				
	Maximum flow	0.362	0.821	0.412	c.m/sec"				
	HYDROGRAPH Add, Runoff "	0.282	0.123	0.299					
40	4 Add Runoff "	0.299	0.299	0.014	0.339"				
40	8 HYDROGRAPH Copy to Outflow"	0.299	0.299	0.299	0.339"				
40	6 HYDROGRAPH " Combine 7"	0.299	0.299	0.339"					
40	2 HYDROGRAPH Start - New Tributary"	0.299	0.299	0.299	0.299"				
33	1 CATCHMENT 201	0.299	0.000	0.299	0.299"				
	1 Triangular SCS								
	Proportional to %"								
	201								
	20,000 % Impervious"								
	2,150 Total Area"								
	45,000 Flow length"								
	2,000 Overland Slope"								
	1,720 Pervious Area"								
	45,000 Pervious length"								
	2,000 Pervious slope"								
	11,250 Impervious Area"								
	7,000 Impervious length"								
	0.250 Pervious Manning "n"								
	74,000 Pervious SCS Curve No."								
	0.361 Pervious Runoff coefficient"								
	0.100 Pervious Ia/S coefficient"								
	8,924 Pervious Initial abstraction"								
	0.015 Impervious Manning "n"								
	98,000 Impervious SCS Curve No."								
	0.850 Impervious Runoff coefficient"								
	0.100 Impervious Ia/S coefficient"								
	0.518 Impervious Initial abstraction"								
	Catchment 201	0.168	0.000	0.299	0.299 c.m/sec"				
	Surface Area	1,720	0.430	2,150	hectare"				
	Time of concentration	21,289	1,175	13,843	minutes"				
	Rainfall depth	242,462	193,655	224,394	mm"				
	Rainfall volume	71,900	71,900	71,900	c.m"				
	Rainfall losses	1236.68	309.17	1545.85	mm"				
	Runoff depth	45,913	10,804	38,891	c.m/sec"				
	Runoff volume	25,007	35,032	25,640	c.m"				
	Runoff coefficient	974.47	273.38	1247.85	c.m/sec"				
	Maximum flow	0.362	0.821	0.412	c.m/sec"				
	HYDROGRAPH Add, Runoff "	0.282	0.123	0.299					
40	4 Add Runoff "	0.299	0.299	0.014	0.339"				
40	8 HYDROGRAPH Copy to Outflow"	0.299	0.299	0.299	0.339"				
40	6 HYDROGRAPH " Combine 7"	0.299	0.299	0.339"					
40	2 HYDROGRAPH Start - New Tributary"	0.299	0.299	0.299	0.299"				
33	1 CATCHMENT 201	0.299	0.000	0.299	0.299"				
	1 Triangular SCS								
	Proportional to %"								
	201								
	20,000 % Impervious"								
	2,150 Total Area"								
	45,000 Flow length"								
	2,000 Overland Slope"								
	1,720 Pervious Area"								
	45,000 Pervious length"								
	2,000 Pervious slope"								
	11,250 Impervious Area"								
	7,000 Impervious length"								
	0.250 Pervious Manning "n"								
	74,000 Pervious SCS Curve No."								
	0.361 Pervious Runoff coefficient"								
	0.100 Pervious Ia/S coefficient"								
	8,924 Pervious Initial abstraction"								
	0.015 Impervious Manning "n"								
	98,000 Impervious SCS Curve No."								
	0.850 Impervious Runoff coefficient"								
	0.100 Impervious Ia/S coefficient"								
	0.518 Impervious Initial abstraction"								
	Catchment 201	0.168	0.000	0.299	0.299 c.m/sec"				
	Surface Area	1,720	0.430	2,150	hectare"				
	Time of concentration	21,289	1,175	13,843	minutes"				
	Rainfall depth	242,462	193,655	224,394	mm"				
	Rainfall volume	71,900	71,900	71,900	c.m"				
	Rainfall losses	1236.68	309.17	1545.85	mm"				
	Runoff depth	45,913	10,804	38,891	c.m/sec"				
	Runoff volume	25,007	35,032	25,640	c.m"				
	Runoff coefficient	974.47	273.38	1247.85	c.m/sec"				
	Maximum flow	0.362	0.821	0.412	c.m/sec"				
	HYDROGRAPH Add, Runoff "	0.282	0.123	0.299					
40	4 Add Runoff "	0.299	0.299	0.014	0.339"				
40	8 HYDROGRAPH Copy to Outflow"	0.299	0.299	0.299	0.339"				
40	6 HYDROGRAPH " Combine 7"	0.299	0.299	0.339"					
40	2 HYDROGRAPH Start - New Tributary"	0.299	0.299	0.299	0.299"				
33	1 CATCHMENT 201	0.299	0.000	0.299	0.299"				
	1 Triangular SCS								
	Proportional to %"								
	201								
	20,000 % Impervious"								
	2,150 Total Area"								
	45,000 Flow length"								
	2,000 Overland Slope"								
	1,720 Pervious Area"								
	45,000 Pervious length"								
	2,000 Pervious slope"								
	11,250 Impervious Area"								
	7,000 Impervious length"								
	0.250 Pervious Manning "n"								
	74,000 Pervious SCS Curve No."								
	0.361 Pervious Runoff coefficient"								
	0.100 Pervious Ia/S coefficient"								
	8,924 Pervious Initial abstraction"								
	0.015 Impervious Manning "n"								
	98,000 Impervious SCS Curve No."								
	0.850 Impervious Runoff coefficient"								
	0.100 Impervious Ia/S coefficient"								
	0.518 Impervious Initial abstraction"								
	Catchment 201	0.168	0.000	0.299	0.299 c.m/sec"				
	Surface Area	1,720	0.430	2,150	hectare"				
	Time of concentration	21,289	1,175	13,843	minutes"				
	Rainfall depth	242,462	193,655	224,394	mm"				
	Rainfall volume	71,900	71,900	71,900	c.m"				
	Rainfall losses	1236.68	309.17	1545.85	mm"				
	Runoff depth	45,913	10,804	38,891	c.m/sec"				
	Runoff volume	25,007	35,032	25,640	c.m"				
	Runoff coefficient	974.47	273.38	1247.85	c.m/sec"				
	Maximum flow	0.362	0.821	0.412	c.m/sec"				
	HYDROGRAPH Add, Runoff "	0.282	0.123	0.299					
40	4 Add Runoff "	0.299	0.299	0.014	0.339"				
40	8 HYDROGRAPH Copy to Outflow"	0.299	0.299	0.299	0.339"				
40	6 HYDROGRAPH " Combine 7"	0.299	0.299	0.339"					
40	2 HYDROGRAPH Start - New Tributary"	0.299	0.299	0.299	0.299"				
33	1 CATCHMENT 201	0.299	0.000	0.299	0.299"				
	1 Triangular SCS								
	Proportional to %"								
	201								
	20,000 % Impervious"								
	2,150 Total Area"								
	45,000 Flow length"								
	2,000 Overland Slope"								
	1,720 Pervious Area"								
	45,000 Pervious length"								
	2,000 Pervious slope"								
	11,250 Impervious Area"								
	7,000 Impervious length"								
	0.250 Pervious Manning "n"								
	74,000 Pervious SCS Curve No."								
	0.361 Pervious Runoff coefficient"								
	0.100 Pervious Ia/S coefficient"								
	8,924 Pervious Initial abstraction"								
	0.015 Impervious Manning "n"								
	98,000 Impervious SCS Curve No."								
	0.850 Impervious Runoff coefficient"	</							

40	HYDROGRAPH " Combine "	8"	
6	Node #		
8	Total to off-site"	0.061	c.m./sec"
	Maximum flow	283.696	c.m."
	Hydrograph volume	0.061	0.061
40	HYDROGRAPH Start - New Tributary"		
2	Start - New Tributary"	0.061	0.061
33	CATCHMENT 401"	0.000	0.061
1	Triangular SCS"		
2	Proportional to %"		
1	SCS method"		
401	401 "To Off Site - West"		
8.000	% Impervious"		
1.890	Total Area"		
75.000	Flow length"		
3.000	Overland Slope"		
1.555	Pervious Area"		
75.000	Pervious length"		
3.000	Pervious Slope"		
0.135	Impervious Area"		
6.522	Impervious length"		
3.000	Impervious slope"		
0.250	Pervious Manning "n"		
74.000	Pervious SCS Curve No. "		
0.361	Pervious Runoff coefficient"		
0.100	Pervious Ia/S coefficient"		
8.924	Pervious Initial abstraction"		
0.015	Impervious Manning "n"		
98.000	Impervious SCS Curve No. "		
0.819	Impervious Runoff coefficient"		
0.100	Impervious Ia/S coefficient"		
0.518	Impervious Initial abstraction"		
	0.111	0.000	0.061
	Catchment 401	Pervious	Impervious
	Surface Area	1.555	0.135
	Time of concentration	25.611	0.750
	Time to Centroid	248.228	23.632
	Rainfall depth	71.900	71.900
	Rainfall volume	1117.90	97.21
	Rainfall losses	45.942	13.048
	Runoff depth	25.958	58.852
	Runoff volume	403.59	79.57
	Runoff coefficient	0.361	0.819
	Maximum flow	0.106	0.036
	HYDROGRAPH Add Runoff "		
4	Add Runoff "	0.111	0.061
	0.111	0.111	0.061
40	HYDROGRAPH Copy to outflow"		
8	Copy to outflow"	0.111	0.061
40	HYDROGRAPH " Combine "	8"	
6	Node #		
8	Total to off-site"		c.m./sec"
	Maximum flow	766.858	c.m."
	Hydrograph volume	0.111	0.170
40	HYDROGRAPH Start - New Tributary"		
2	Start - New Tributary"	0.111	0.170

Page 7

[illegible]



218173- PostDev-Oct2019-50yr.out

30.000 Pervious length" 0.325 c.m/sec" 0.000  
 4.000 Pervious slope" 0.000  
 0.000 Impervious Area" 0.000  
 0.000 Impervious length" 0.000  
 4.000 Impervious slope" 0.000  
 0.250 Pervious Manning 'n'" 0.000  
 74.000 Pervious SCS Curve No. " 0.000  
 0.358 Pervious Runoff coefficient" 0.000  
 0.100 Pervious Ia/S coefficient" 0.000  
 8.924 Pervious Initial abstraction" 0.000  
 0.015 Pervious Manning 'n'" 0.000  
 98.000 Impervious SCS Curve No. 0.000  
 0.000 Impervious Runoff coefficient" 0.000  
 0.100 Impervious Ia/S coefficient" 0.000  
 0.518 Impervious Initial abstraction" 0.000  
 0.050 0.000 0.157  
 Catchment 600 Pervious 0.325 c.m/sec" 0.000  
 Surface Area 0.550 0.550  
 Time of concentration 13.558 13.558  
 Time to Centroid 232.669 232.669  
 Rainfall depth 71.900 71.900  
 Rainfall volume 395.45 395.45  
 Rainfall losses 46.174 46.174  
 Runoff depth 25.880 25.880  
 Runoff volume 141.49 141.49  
 Runoff coefficient 0.358 0.358  
 Maximum flow 0.050 0.050  
 HYDROGRAPH Add Runoff " 0.050  
 4 Add Runoff 0.050 0.157 0.325"  
 HYDROGRAPH Copy to Outflow"  
 8 Copy to Outflow" 0.050 0.050 0.325"  
 HYDROGRAPH " Combine 10" 0.050 0.325"  
 6 Combine " Combine 10" 0.050 0.325"  
 10 Node #"  
 To Pond 201"  
 Maximum flow 0.373 c.m/sec" 0.373  
 Hydrograph volume 1437.055 c.m" 1437.055  
 0.050 0.050 0.373"  
 7 Confluence " Confluence 10" 0.050 0.373"  
 10 Node #"  
 To Pond 201"  
 Maximum flow 0.373 c.m/sec" 0.373  
 Hydrograph volume 1437.055 c.m" 1437.055  
 0.050 0.050 0.000"  
 POND DESIGN"  
 0.373 Current peak flow c.m/sec" 0.373  
 0.100 Target outflow c.m/sec" 0.000  
 1437.1 Hydrograph volume c.m" 1437.1  
 10. Number of stages" 10  
 423.300 Minimum water level metre" 423.300  
 424.500 Maximum water level metre" 424.500  
 423.300 Starting water level metre" 423.300  
 0 Keep Design Data: 1 = True; 0 = False"  
 Level Discharge Volume"  
 423.300 0.000 0.000  
 423.400 0.01130 172.400  
 423.500 0.02640 353.500  
 423.600 0.03737 543.700  
 423.700 0.04577 743.000  
 423.800 0.05281 951.800

218173- PostDev-Oct2019-50yr.out

423.900 0.05904 1170.100" 0.051  
 424.000 0.3343 1398.300" 423.774  
 424.100 0.8324 1636.400" 897.717  
 424.200 1.475 1884.800" 8.027  
 Peak outflow 0.051 c.m/sec"  
 Maximum level 423.774 metre  
 Maximum storage 897.717 c.m"  
 Centroidal lag 0.373 0.051 0.000 c.m/sec"  
 7" 7"  
 HYDROGRAPH " Combine  
 6 Combine " Combine  
 7 Node #"  
 Total to Creek"  
 Maximum flow 0.339 c.m/sec"  
 Hydrograph volume 2684.709 c.m" 0.339  
 0.050 0.051  
 START/RE-START TOTALS 10"  
 3 Runoff Totals on EXIT"  
 Total Catchment area 21.320 hectare"  
 Total Impervious area 13.692 hectare"  
 Total % Impervious 17.317"  
 EXIT"

218173- PostDev-Oct2019-100yr.out  
 MIDUSS Output  
 MIDUSS version Version 2.25 rev. 473  
 MIDUSS created Sunday, February 07, 2010  
 Units used: ie METRIC  
 Job folder: C:\Users\jswiger\Desktop\wilder Lake\MIDUSS  
 Output filename: 218173- PostDev-Oct22-100yr.out  
 Licensee name: Hewlett-Packard Company  
 Date & Time last used: 10/22/2019 at 2:33:24 PM  
 TIME PARAMETERS  
 10 10.000 Time Step  
 360.000 Max. Storm length  
 2400.000 Max. Hydrograph  
 3 3 Mass Curve  
 78.400 Rainfall depth  
 360.000 Duration  
 47 C:\Program Files (x86)\MIDUSS\SCS\_6hr\_Type2.mrd SCS 6 hour Type  
 2 Maximum intensity 125.167 mm/hr  
 Total depth 78.400 mm  
 6 100hyd Hydrograph extension used in this file  
 CATCHMENT 100  
 1 Triangular SCS  
 2 Proportional to %  
 100 100-wilder Lake South  
 % Impervious  
 10.000 Total Area  
 4.250 Flow length  
 70.000 Overland Slope  
 3.825 Pervious Area  
 70.000 Pervious length  
 4.000 Pervious slope  
 0.425 Impervious Area  
 7.778 Impervious length  
 4.000 Impervious slope  
 0.250 Pervious Manning "n"  
 74.000 Pervious SCS Curve No.  
 0.387 Pervious Runoff coefficient  
 0.100 Pervious Ia/S coefficient  
 8.924 Pervious Initial abstraction  
 0.015 Pervious Manning "n"  
 98.000 Impervious SCS Curve No.  
 0.823 Impervious Runoff coefficient  
 0.100 Impervious Ia/S coefficient  
 0.518 Impervious Initial abstraction  
 0.362 0.000 0.000 0.000 c.m/sec  
 Catchment 100  
 Surface Area 5.825 Pervious  
 Time of concentration 21.101  
 Time to Centroid 21.101  
 Rainfall depth 78.400  
 Rainfall volume 2998.80  
 Rainfall losses 48.070  
 Runoff depth 30.330  
 Runoff volume 1160.12  
 Runoff coefficient 0.387  
 Maximum flow 0.345  
 HYDROGRAPH Add Runoff  
 4 Add Runoff 0.362 0.000 0.000  
 Page 1

218173- PostDev-Oct2019-100yr.out  
 HYDROGRAPH Copy to Outflow  
 8 Copy to Outflow 0.362 0.362 0.000  
 HYDROGRAPH 6 Combine 6 0.362 0.000  
 6 Node #  
 6 To Wilder Lake  
 Maximum flow 0.362 c.m/sec  
 Hydrograph volume 1434.355 c.m  
 0.362 0.362  
 HYDROGRAPH Start - New Tributary  
 2 Start - New Tributary 0.362 0.000 0.362 0.362  
 CATCHMENT 101  
 1 Triangular SCS  
 2 Proportional to %  
 1 SCS method  
 101 101 - Developed Area South  
 % Impervious  
 30.000 Total Area  
 4.890 Flow length  
 75.000 Overland Slope  
 3.423 Pervious Area  
 75.000 Pervious length  
 2.000 Pervious slope  
 1.467 Impervious Area  
 32.143 Impervious length  
 2.000 Impervious slope  
 0.250 Pervious Manning "n"  
 74.000 Pervious SCS Curve No.  
 0.387 Pervious Runoff coefficient  
 0.100 Pervious Ia/S coefficient  
 8.924 Pervious Initial abstraction  
 0.015 Pervious Manning "n"  
 98.000 Impervious SCS Curve No.  
 0.899 Impervious Runoff coefficient  
 0.100 Impervious Ia/S coefficient  
 0.518 Impervious Initial abstraction  
 0.490 0.000 0.362  
 Catchment 101  
 Surface Area 3.423 Pervious  
 Time of concentration 27.077  
 Time to Centroid 249.502  
 Rainfall depth 78.400  
 Rainfall volume 2683.63  
 Rainfall losses 48.054  
 Runoff depth 30.346  
 Runoff volume 1038.75  
 Runoff coefficient 0.387  
 Maximum flow 0.263  
 HYDROGRAPH Add Runoff  
 4 Add Runoff 0.490 0.490 0.362 0.362  
 POND DESIGN  
 0.490 Current peak flow c.m/sec  
 0.080 Target outflow c.m/sec  
 2072.5 Hydrograph volume c.m  
 12. Number of stages  
 423.000 Minimum water level metre  
 425.750 Maximum water level metre  
 423.000 Starting water level metre  
 0 Keep Design Data: 1 = True; 0 = False  
 Level Discharge Volume  
 Page 2

218173- PostDev-Oct2019-100yr.out

423.000	0.000	0.000	
423.250	1.00E-04	15.300"	
423.500	0.00020	68.600"	
423.750	0.00030	162.000"	
424.000	0.00040	284.900"	
424.250	0.00050	438.500"	
424.500	0.00060	624.800"	
424.750	0.00350	845.500"	
425.000	0.03510	1102.500"	
425.250	0.04240	1397.400"	
425.500	0.04540	1731.900"	
425.750	1.114	2108.600"	
Peak outflow		0.044	c.m/sec"
Maximum level		425.350	metre"
Maximum storage		1531.441	c.m"
Centroidal lag		69.749	hours"
0.490	0.490	0.044	0.362 c.m/sec"
HYDROGRAPH " Combine		6"	
6			
To Wilder Lake"			
Maximum flow		0.399	c.m/sec"
Hydrograph volume		2976.708	c.m"
0.490	0.490	0.044	0.399"
HYDROGRAPH start - New Tributary"			
2			
Start - New Tributary"		0.000	0.044
0.490			0.399"
CATCHMENT 102"			
1			
Triangular SCS"			
2			
Proportional to %"			
102			
102 - Developed Area North"			
30.000			
% Impervious			
0.900			
Total Area"			
45.000			
Flow length"			
2.000			
Overland Slope"			
0.630			
Pervious Area"			
45.000			
Pervious length"			
2.000			
Pervious slope"			
0.270			
Impervious Area"			
19.286			
Impervious length"			
2.000			
Impervious slope"			
0.250			
Pervious Manning "n"			
74.000			
Pervious SCS Curve No."			
0.386			
Pervious Runoff coefficient"			
0.100			
Pervious Ia/S coefficient"			
8.924			
Pervious Initial abstraction"			
0.015			
Impervious Manning "n"			
98.000			
Impervious SCS Curve No."			
0.880			
Impervious Runoff coefficient"			
0.100			
Impervious Ia/S coefficient"			
0.518			
Impervious Initial abstraction"			
0.098			
Impervious SCS Curve No."			
0.044			
Catchment 102			
Surface Area			
Time of concentration			
19.929			
Time to Centroid			
240.018			
Rainfall depth			
78.400			
Rainfall volume			
493.92			
Rainfall losses			
48.164			
Runoff depth			
30.236			
Runoff volume			
190.49			
Runoff coefficient			
0.386			
0.880			
0.534			

218173- PostDev-Oct2019-100yr.out

Maximum flow			
HYDROGRAPH Add Runoff "0.058		0.080	0.098
4			
Add Runoff"		0.098	0.044
0.098			0.399"
POND DESIGN"			
0.098			
Current peak flow		c.m/sec"	
0.030			
Target outflow		c.m/sec"	
376.8			
Hydrograph volume		c.m"	
11.			
Number of stages"			
423.200			
Minimum water level		metre"	
424.850			
Maximum water level		metre"	
423.200			
Starting water level		metre"	
0			
Keep Design Data: 1 = True; 0 = False"			
Level Discharge			
423.350		0.000	
423.500		1.00E-05	
423.650		2.00E-05	
423.800		3.00E-05	
423.950		4.00E-05	
424.100		5.00E-05	
424.250		6.00E-05	
424.400		0.01320	
424.550		0.01548	
424.700		0.01746	
424.850		0.01945	
Peak outflow		0.015	c.m/sec"
Maximum level		424.545	metre"
Maximum storage		246.818	c.m"
Centroidal lag		6.073	hours"
0.098		0.015	0.399 c.m/sec"
HYDROGRAPH " Combine		6"	
6			
Combine " Node #"			
To Wilder Lake"			
Maximum flow			
Hydrograph volume		0.411	c.m/sec"
0.098		3163.568	c.m"
HYDROGRAPH start - New Tributary"		0.015	0.411"
2			
Start - New Tributary"		0.098	0.015
0.098			0.411"
CATCHMENT 200"			
1			
Triangular SCS"			
2			
Proportional to %"			
1			
SCS method"			
200			
200 - Creek"			
% Impervious			
11.000			
Total Area"			
4.210			
Flow length"			
5.000			
Overland Slope"			
3.747			
Pervious Area"			
75.000			
Pervious length"			
5.000			
Pervious slope"			
0.463			
Impervious Area"			
9.270			
Impervious length"			
5.000			
Impervious slope"			
0.250			
Pervious Manning "n"			
74.000			
Pervious SCS Curve No."			
0.386			
Pervious Runoff coefficient"			
0.100			
Pervious Ia/S coefficient"			
8.924			
Pervious Initial abstraction"			
0.015			
Impervious Manning "n"			
98.000			
Impervious SCS Curve No."			
0.825			
Impervious Runoff coefficient"			

218173- PostDev-Oct2019-100yr.out

0.100	Impervious Initial abstraction"	0.361	0.000	Pervious	0.411	c.m/sec"	
0.518	Catchment 200	3.747	0.015	Impervious	0.463	hectare"	
	Surface Area	20.569	0.767	16.436	minutes"		
	Time of concentration	240.810	192.235	78.400	minutes"		
	Rainfall depth	76.400	363.07	3300.64	c.m"		
	Rainfall volume	2357.57	48.102	34.079	mm"		
	Rainfall losses	48.102	13.736	299.46	mm"		
	Runoff depth	30.298	1135.25	0.825	c.m"		
	Runoff volume	1135.25	0.386	0.435	c.m/sec"		
	Runoff coefficient	0.386	0.134	0.361			
	Maximum flow	0.343					
40	HYDROGRAPH Add Runoff "	0.361	0.361	0.015	0.411"		
	4 Add Runoff "	0.361	0.361	0.015	0.411"		
40	HYDROGRAPH Copy to Outflow"	0.361	0.361	0.361	0.411"		
	8 Copy to Outflow"	0.361	0.361	0.361	0.411"		
40	HYDROGRAPH " Combine 7"	0.361	0.361	0.361	0.411"		
	6 Combine " Combine 7"	0.361	0.361	0.361	0.411"		
	7 Node #"	0.361	0.361	0.361	0.411"		
	Total to Creek"	0.361	0.361	0.361	0.411"		
	Maximum flow	0.361	0.361	0.361	0.411"		
	Hydrograph volume	1434.710	0.361	0.361	0.411"		
40	HYDROGRAPH Start - New Tributary"	0.361	0.361	0.361	0.411"		
	2 Start - New Tributary"	0.361	0.361	0.361	0.411"		
33	CATCHMENT 201"	0.361	0.361	0.361	0.411"		
	1 Triangular SCS"	0.361	0.361	0.361	0.411"		
	2 Proportional to %"	0.361	0.361	0.361	0.411"		
	201"	0.361	0.361	0.361	0.411"		
	20.000 % Impervious"	0.361	0.361	0.361	0.411"		
	2.150 Total Area"	0.361	0.361	0.361	0.411"		
	45.000 Flow length"	0.361	0.361	0.361	0.411"		
	2.000 Overland Slope"	0.361	0.361	0.361	0.411"		
	1.720 Pervious Area"	0.361	0.361	0.361	0.411"		
	45.000 Pervious length"	0.361	0.361	0.361	0.411"		
	2.000 Pervious slope"	0.361	0.361	0.361	0.411"		
	0.430 Pervious Area"	0.361	0.361	0.361	0.411"		
	11.250 Pervious length"	0.361	0.361	0.361	0.411"		
	2.000 Pervious slope"	0.361	0.361	0.361	0.411"		
	0.250 Pervious Manning "n"	0.361	0.361	0.361	0.411"		
	74.000 Pervious SCS Curve No."	0.361	0.361	0.361	0.411"		
	0.386 Pervious Runoff coefficient"	0.361	0.361	0.361	0.411"		
	0.100 Pervious Ia/S coefficient"	0.361	0.361	0.361	0.411"		
	8.924 Pervious Initial abstraction"	0.361	0.361	0.361	0.411"		
	0.015 Pervious Manning "n"	0.361	0.361	0.361	0.411"		
	98.000 Pervious SCS Curve No."	0.361	0.361	0.361	0.411"		
	0.852 Pervious Runoff coefficient"	0.361	0.361	0.361	0.411"		
	0.100 Pervious Ia/S coefficient"	0.361	0.361	0.361	0.411"		
0.518	Impervious Initial abstraction"	0.361	0.361	0.361	0.411"		
	Catchment 201	0.361	0.361	0.361	0.411"		
	Surface Area	1.720	0.430	1.133	hectare"		
	Time of concentration	19.929	1.133	13.244	minutes"		
	Time to Centroid	240.018	193.138	78.400	minutes"		
	Rainfall depth	76.400	337.12	1685.60	c.m"		
	Rainfall volume	1348.48	48.164	40.859	mm"		
	Rainfall losses	48.164	11.638				

218173- PostDev-Oct2019-100yr.out

Runoff depth	30.236	66.762	37.541
Runoff volume	520.06	287.08	807.14
Runoff coefficient	0.386	0.852	0.479
Maximum flow	0.160	0.127	0.201
HYDROGRAPH Add Runoff "	0.201	0.201	0.201
4 Add Runoff "	0.201	0.201	0.201
HYDROGRAPH Copy to Outflow"	0.201	0.201	0.201
8 Copy to Outflow"	0.201	0.201	0.201
HYDROGRAPH " Combine 10"	0.201	0.201	0.201
6 Combine " Combine 10"	0.201	0.201	0.201
10 Node #"	0.201	0.201	0.201
To Pond 201"	0.201	0.201	0.201
Maximum flow	0.201	0.201	0.201
Hydrograph volume	807.135	0.201	0.201
HYDROGRAPH Start - New Tributary"	0.201	0.201	0.201
2 Start - New Tributary"	0.201	0.201	0.201
CATCHMENT 400"	0.201	0.201	0.201
1 Triangular SCS"	0.201	0.201	0.201
2 Proportional to %"	0.201	0.201	0.201
400 % Impervious"	0.201	0.201	0.201
8.000 Total Area"	0.201	0.201	0.201
75.000 Flow length"	0.201	0.201	0.201
2.000 Overland Slope"	0.201	0.201	0.201
0.911 Pervious Area"	0.201	0.201	0.201
75.000 Pervious length"	0.201	0.201	0.201
2.000 Pervious slope"	0.201	0.201	0.201
0.079 Pervious Area"	0.201	0.201	0.201
6.522 Pervious length"	0.201	0.201	0.201
2.000 Pervious slope"	0.201	0.201	0.201
0.250 Pervious Manning "n"	0.201	0.201	0.201
74.000 Pervious SCS Curve No."	0.201	0.201	0.201
0.387 Pervious Runoff coefficient"	0.201	0.201	0.201
0.100 Pervious Ia/S coefficient"	0.201	0.201	0.201
8.924 Pervious Initial abstraction"	0.201	0.201	0.201
0.015 Pervious Manning "n"	0.201	0.201	0.201
98.000 Pervious SCS Curve No."	0.201	0.201	0.201
0.827 Pervious Runoff coefficient"	0.201	0.201	0.201
0.100 Pervious Ia/S coefficient"	0.201	0.201	0.201
0.518 Pervious Initial abstraction"	0.201	0.201	0.201
Catchment 400	0.201	0.201	0.201
Surface Area	0.911	0.079	0.990
Time of concentration	27.077	0.817	22.960
Time to Centroid	249.502	192.382	240.548
Rainfall depth	78.400	78.400	78.400
Rainfall volume	714.07	62.09	776.16
Rainfall losses	48.054	13.526	45.292
Runoff depth	30.346	64.874	33.108
Runoff volume	276.39	51.38	327.77
Runoff coefficient	0.387	0.827	0.422
Maximum flow	0.070	0.023	0.073
HYDROGRAPH Add Runoff "	0.073	0.073	0.073
4 Add Runoff "	0.073	0.073	0.073
HYDROGRAPH Copy to Outflow"	0.073	0.073	0.073
8 Copy to Outflow"	0.073	0.073	0.073

218173- PostDev-Oct2019-100yr.out

40	HYDROGRAPH " Combine	0.073	c.m./sec"
6	Node #	0.073 <td>c.m."</td>	c.m."
8	Total to Off-Site"	0.073 <td>0.073"</td>	0.073"
	Maximum flow	327.773	
	Hydrograph volume	0.073	0.073"
40	HYDROGRAPH Start - New Tributary"	0.073	
2	Start - New Tributary"	0.000	0.073"
33	CATCHMENT 401"		
1	Triangular SCS"		
2	Proportional to %"		
1	SCS method"		
401	401 - To Off Site - West"		
8.000	% Impervious"		
1.690	Total Area"		
75.000	Flow length"		
1.555	Overland Slope"		
3.000	Pervious Area"		
75.000	Pervious length"		
3.000	Pervious slope"		
0.135	Impervious Area"		
6.522	Impervious length"		
3.000	Impervious slope"		
0.250	Pervious Manning "n"		
74.000	Pervious SCS Curve No."		
0.387	Pervious Runoff coefficient"		
0.100	Pervious Ia/S coefficient"		
8.924	Pervious initial abstraction"		
0.015	Impervious Manning "n"		
98.000	Impervious SCS Curve No."		
0.823	Impervious Runoff coefficient"		
0.100	Impervious Ia/S coefficient"		
0.518	Impervious initial abstraction"		
	0.136	0.000	0.073
	Catchment 401		
	Surface Area	1.555	Impervious
	Time of concentration	23.975	0.135
	Time to Centroid	245.327	0.724
	Rainfall depth	78.400	192.088
	Rainfall volume	1218.96	78.400
	Rainfall losses	48.072	106.00
	Runoff depth	30.328	13.910
	Runoff volume	471.54	33.328
	Runoff coefficient	0.387	58.19
	Maximum flow	0.136	0.4.0
	HYDROGRAPH Add Runoff "	0.130	0.823
4	Add runoff "		0.039
	0.136	0.136	0.073
40	HYDROGRAPH Copy to Outflow"		
8	Copy to Outflow"	0.136	0.073"
	HYDROGRAPH " Combine	8"	
6	Combine		
8	Node #		
	Total to off-Site"		
	Maximum flow	0.209	c.m./sec"
	Hydrograph volume	886.507	c.m."
	0.136	0.136	0.209"
40	HYDROGRAPH Start - New Tributary"		
2	Start - New Tributary"	0.136	0.136
	0.136	0.000	0.136
	Page 7		

218173- PostDev-Oct2019-100yr.out

[illegible]

	218173- PostDev-Oct2019-100yr.out	
30.000	pervious length"	
4.000	pervious slope"	
0.000	Impervious Area"	
0.000	pervious length"	
4.000	Impervious slope"	
0.250	Pervious Manning "n"	
74.000	Pervious SCS Curve No."	
0.384	Pervious Runoff coefficient"	
0.100	Pervious Ia/S coefficient"	
8.924	Pervious Initial abstraction"	
0.015	Impervious Manning "n"	
98.000	Impervious SCS Curve No."	
0.000	Impervious Runoff coefficient"	
0.100	Impervious Ia/S coefficient"	
0.518	Impervious Initial abstraction"	
	Catchment 600	
	Surface Area	0.550
	Time of Concentration	12.692
	Time to Centroid	230.611
	Rainfall depth	190.249
	Rainfall volume	78.400
	Rainfall losses	431.20
	Rainfall losses	12.685
	Runoff depth	38.324
	Runoff volume	65.715
	Runoff coefficient	165.42
	Maximum flow	0.384
	HYDROGRAPH Add_Runoff "	0.059
40	4 Add Runoff "	0.059
	HYDROGRAPH Copy to Outflow"	0.197
40	8 Copy to Outflow"	0.398"
	HYDROGRAPH " Combine 10"	0.059
40	6 Combine " Node #"	10"
	To Pond 201"	
	Maximum flow	0.457
	Hydrograph volume	1638.047
	HYDROGRAPH Confluence 10"	0.059
40	7 Confluence " Node #"	10"
	To Pond 201"	
	Maximum flow	0.457
	Hydrograph volume	1638.047
	HYDROGRAPH " Combine 10"	0.059
54	POND DESIGN"	0.457
	Current peak flow c.m/sec"	c.m/sec"
0.100	Target outflow c.m/sec"	c.m"
1638.0	Hydrograph volume	c.m"
10	Number of stages"	
423.300	Minimum water level	metre"
424.500	Maximum water level	metre"
423.300	Starting water level	metre"
	Keep Design Data: I = True; J = False"	
	Elev Discharge Volume"	
	423.300 0.000	
	423.400 0.01130	
	423.500 0.02640	
	423.600 0.03737	
	423.700 0.04577	
	423.800 0.05281	
	218173- PostDev-Oct2019-100yr.out	
423.900	0.05904	1170.100"
424.000	0.3343	1398.300"
424.100	0.8324	1636.400"
424.200	1.475	1884.800"
	Peak outflow	0.055
	Maximum Level	423.842
	Maximum storage	1042.675
	Centroidal lag	8.196
	0.059	0.457
	HYDROGRAPH " Combine 7"	0.055
40	6 Combine " Node #"	7"
	Total to Creek"	
	Maximum flow	0.405
	Hydrograph volume	3072.430
	0.059	0.457
38	START/RE-START TOTALS 10"	0.055
	3 Runoff Totals on EXIT"	
	Total Catchment area	
	Total Impervious area	
	Total % Impervious	
19	EXIT"	
	hectare"	21.320
	hectare"	3.692
	hectare"	17.317"
	Page 10	





218173- PostDev-Oct2019-Haze1.out

```

40 HYDROGRAPH " Combine 6"
6 Node #
6 To Wilder Lake
Maximum flow 9033.765 c.m/sec"
HYDROGRAPH volume 0.582 0.582 c.m/sec"
HYDROGRAPH Start - New Tributary 0.582 0.582 c.m/sec"
2 Start - New Tributary 0.582 0.582 c.m/sec"
CATCHMENT 101
1 Triangular SCS
2 Proportional to %
101 101 - Developed Area South"
30.000 % Impervious
4.890 Total Area"
75.000 Flow length"
2.000 Overland Slope"
3.423 Pervious Area"
75.000 Pervious length"
2.000 Pervious slope"
1.467 Impervious Area"
32.143 Impervious length"
2.000 Impervious slope"
0.250 Pervious Manning 'n'"
74.000 Pervious SCS Curve No.
0.731 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning 'n'"
98.000 Impervious SCS Curve No.
0.970 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.692 0.000 0.582 c.m/sec"
Surface Area 3.423 Impervious Total Area "
Time of concentration 27.748 1.467 4.890 hectare"
Rainfall depth 2527.670 2.981 18.764 minutes"
Rainfall volume 285.008 2271.608 2434.786 mm"
Rainfall losses 0.9256 285.008 285.008 ha-m"
Runoff depth 76.801 1.3937 56.305 mm"
Runoff volume 208.207 276.528 228.703 mm"
Runoff coefficient 0.7127 0.4057 1.1184 ha-m"
Maximum flow 0.731 0.970 0.802 c.m/sec"
HYDROGRAPH Add Runoff " 0.480 0.212 0.692
4 Add Runoff " 0.692 0.692 0.582 0.582"
POND DESIGN"
0.692 Current peak flow c.m/sec"
0.080 Target outflow c.m/sec"
11183.6 Hydrograph volume c.m"
12. Number of stages"
423.000 Minimum water level metre"
423.750 Maximum water level metre"
423.000 Starting water level metre"
0 Keep Design Data: 1 = True; 0 = False"
Level Discharge Volume"
423.000 0.000
423.250 1.00E-04 15.300"
423.500 0.00020 68.600"
Page 3

```

```

218173- PostDev-Oct2019-Haze1.out
423.750 0.00030 162.000"
424.000 0.00040 284.900"
424.250 0.00050 438.500"
424.500 0.00060 624.800"
424.750 0.00080 845.500"
425.000 0.00100 1102.500"
425.250 0.00120 1397.400"
425.500 0.00140 1731.900"
425.750 0.00160 2108.600"
Peak outflow 0.686 c.m/sec"
Maximum level 425.650 metre"
Maximum storage 1957.827 c.m"
Centroidal lag 44.186 hours"
0.692 0.692 0.686 0.582 c.m/sec"
HYDROGRAPH " Combine 6"
6 Node #
6 To Wilder Lake"
Maximum flow 1.257 c.m/sec"
Hydrograph volume 19665.244 c.m"
0.692 0.692 0.686 1.257"
HYDROGRAPH Start - New Tributary"
2 Start - New Tributary 0.692 0.686 1.257"
CATCHMENT 102
1 Triangular SCS"
2 Proportional to %"
1 SCS method"
102 102 - Developed Area North"
30.000 % Impervious
0.900 Total Area"
45.000 Flow length"
2.000 Overland Slope"
0.630 Pervious Area"
45.000 Pervious length"
2.000 Pervious slope"
0.270 Impervious Area"
19.286 Impervious length"
2.000 Impervious slope"
0.250 Pervious Manning 'n'"
74.000 Pervious SCS Curve No.
0.730 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
8.924 Pervious Initial abstraction"
0.015 Impervious Manning 'n'"
98.000 Impervious SCS Curve No.
0.956 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.125 0.000 0.686 1.257 c.m/sec"
Catchment 102 Pervious Impervious Total Area "
Surface Area 0.630 0.270 0.900 hectare"
Time of concentration 20.423 2.194 13.870 minutes"
Rainfall depth 2517.156 2270.825 2428.605 mm"
Rainfall volume 285.008 285.008 285.008 ha-m"
Rainfall losses 1795.55 769.52 2565.07 mm"
Runoff depth 77.039 12.663 57.726 mm"
Runoff volume 207.969 272.345 227.282 mm"
Runoff coefficient 1310.20 735.33 2045.54 c.m"
Maximum flow 0.730 0.956 0.797 c.m/sec"
HYDROGRAPH Add Runoff " 0.087 0.037 0.125
4 Add Runoff " 0.087 0.087 0.125

```

0.125	0.125	0.686	1.257"
0.030	Current peak flow	c.m/sec"	
2043.5	Target outflow	c.m/sec"	
423.500	Hydrograph volume	c.m"	
423.500	Number of stages	metre"	
423.500	Minimum water level	metre"	
423.500	Maximum water level	metre"	
423.500	Starting water level	metre"	
0	Keep Design Data: 1 = True; 0 = False"		
423.350	Level Discharge	Volume	
423.500	0.000	0.000	
423.500	1.00E-05	9.800"	
423.650	2.00E-05	24.900"	
423.800	3.00E-05	45.600"	
423.950	4.00E-05	72.300"	
424.100	5.00E-05	105.600"	
424.250	6.00E-05	145.700"	
424.400	0.01320	193.300"	
424.550	0.01548	248.600"	
424.700	0.01746	312.100"	
424.850	0.01944	384.400"	
424.732	Peak outflow	0.124 c.m/sec"	
357.624	Maximum level	metre"	
437.962	Maximum storage	c.m"	
0.125	Centroidal lag	hours"	
0.125	0.125	0.124	1.257 c.m/sec"
6	HYDROGRAPH " Combine	6"	
6	Node #		
6	To Wilder Lake"		
Maximum flow		1.380 c.m/sec"	
Hydrograph volume		21574.660 c.m"	
0.125	0.125	0.124	1.380"
2	HYDROGRAPH Start - New Tributary"		
0.125	0.125	0.000	1.380"
2	Start - New Tributary"		
0.125	0.125	0.000	1.380"
1	Triangular SCS"		
1	Proportional to %"		
200	200 - Creek"		
11.000	% Impervious"		
4.210	Total Area"		
75.000	Flow length"		
3.747	Overland Slope"		
75.000	Pervious Area"		
3.000	Pervious length"		
0.463	Impervious slope"		
9.270	Impervious length"		
5.000	Impervious Manning "n"		
0.250	Pervious Manning "n"		
74.000	Pervious SCS Curve No."		
0.730	Pervious Runoff coefficient"		
0.100	Pervious Ia/s coefficient"		
8.924	Pervious Initial abstraction"		
0.015	Impervious Manning "n"		
98.000	Impervious SCS Curve No."		
0.893	Impervious Runoff coefficient"		
0.100	Impervious Ia/s coefficient"		
0.518	Impervious Initial abstraction"		
0.573	0.573	0.000	1.380 c.m/sec"

--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--

218173- PostDev-Oct2019-Haze1.out			
40	Maximum flow	0.238	0.294
40	HYDROGRAPH Add "Runoff"	0.238	0.056
40	4 Add Runoff	0.294	0.573
40	HYDROGRAPH Copy to Outflow	0.294	0.573
40	8 Copy to Outflow	0.294	0.573
40	HYDROGRAPH "Combine"	10	0.294
40	6 Combine	10	0.294
40	10 Node #	10	0.294
40	To Pond 201	0.294	0.294
40	Maximum flow	0.294	0.294
40	HYDROGRAPH volume	4717.333	0.294
40	0.294	0.294	0.294
40	HYDROGRAPH Start - New Tributary	0.294	0.294
40	2 Start - New Tributary	0.294	0.294
40	0.294	0.294	0.294
40	CATCHMENT 400	0.294	0.294
40	1 Triangular SCS	0.294	0.294
40	2 Proportional to %	0.294	0.294
40	SCS method	0.294	0.294
40	400 - South to Off Site	0.294	0.294
40	8.000 % Impervious	0.294	0.294
40	75.000 Total Area	0.294	0.294
40	Flow length	0.294	0.294
40	Overland Slope	0.294	0.294
40	Pervious Area	0.294	0.294
40	Pervious length	0.294	0.294
40	Impervious Area	0.294	0.294
40	Impervious slope	0.294	0.294
40	Impervious length	0.294	0.294
40	Impervious slope	0.294	0.294
40	Impervious Manning "n"	0.294	0.294
40	Pervious SCS Curve No.	0.294	0.294
40	Pervious Runoff coefficient	0.294	0.294
40	Pervious Ia/S coefficient	0.294	0.294
40	Pervious Initial abstraction	0.294	0.294
40	Impervious Manning "n"	0.294	0.294
40	Impervious SCS Curve No.	0.294	0.294
40	Impervious Runoff coefficient	0.294	0.294
40	Impervious Ia/S coefficient	0.294	0.294
40	Impervious Initial abstraction	0.294	0.294
40	0.137	0.294	0.294
40	Catchment 400	0.294	0.294
40	Surface Area	0.294	0.294
40	Time of Concentration	0.294	0.294
40	Rainfall depth	0.294	0.294
40	Rainfall volume	0.294	0.294
40	Rainfall losses	0.294	0.294
40	Runoff depth	0.294	0.294
40	Runoff volume	0.294	0.294
40	Runoff coefficient	0.294	0.294
40	Maximum flow	0.294	0.294
40	HYDROGRAPH Add Runoff	0.294	0.294
40	4 Add Runoff	0.294	0.294
40	HYDROGRAPH Copy to Outflow	0.294	0.294
40	8 Copy to Outflow	0.294	0.294
40	HYDROGRAPH "Combine"	8	0.294
40	6 Combine	8	0.294
40	8 Node #	8	0.294

218173- PostDev-Oct2019-Haze1.out			
40	Maximum flow	0.137	0.137
40	HYDROGRAPH volume	2099.570	0.137
40	0.137	0.137	0.137
40	HYDROGRAPH Start - New Tributary	0.137	0.137
40	2 Start - New Tributary	0.137	0.137
40	0.137	0.137	0.137
40	CATCHMENT 401	0.137	0.137
40	1 Triangular SCS	0.137	0.137
40	2 Proportional to %	0.137	0.137
40	SCS method	0.137	0.137
40	401 - To Off Site - West	0.137	0.137
40	8.000 % Impervious	0.137	0.137
40	75.000 Total Area	0.137	0.137
40	Flow length	0.137	0.137
40	Overland Slope	0.137	0.137
40	Pervious Area	0.137	0.137
40	Pervious length	0.137	0.137
40	Impervious Area	0.137	0.137
40	Impervious slope	0.137	0.137
40	Impervious length	0.137	0.137
40	Impervious slope	0.137	0.137
40	Impervious Manning "n"	0.137	0.137
40	Pervious SCS Curve No.	0.137	0.137
40	Pervious Runoff coefficient	0.137	0.137
40	Pervious Ia/S coefficient	0.137	0.137
40	Pervious Initial abstraction	0.137	0.137
40	Impervious Manning "n"	0.137	0.137
40	Impervious SCS Curve No.	0.137	0.137
40	Impervious Runoff coefficient	0.137	0.137
40	Impervious Ia/S coefficient	0.137	0.137
40	Impervious Initial abstraction	0.137	0.137
40	0.235	0.000	0.137
40	Catchment 401	0.137	0.137
40	Surface Area	1.555	1.690
40	Time of Concentration	24.570	1.014
40	Rainfall depth	2523.168	2228.112
40	Rainfall volume	285.008	2494.997
40	Rainfall losses	4431.31	285.008
40	Runoff depth	76.807	385.33
40	Runoff volume	208.202	4816.64
40	Runoff coefficient	323.12	73.243
40	Maximum flow	0.731	211.765
40	HYDROGRAPH Add Runoff	0.219	3578.84
40	4 Add Runoff	0.235	0.867
40	HYDROGRAPH Copy to Outflow	0.235	0.743
40	8 Copy to Outflow	0.235	0.017
40	HYDROGRAPH "Combine"	8	0.137
40	6 Combine	8	0.137
40	8 Node #	8	0.137
40	Total to Off-Site	0.372	0.372
40	Maximum flow	5678.406	0.372
40	HYDROGRAPH volume	0.235	0.235
40	HYDROGRAPH Start - New Tributary	0.235	0.372
40	2 Start - New Tributary	0.235	0.372
40	0.235	0.000	0.235
40	CATCHMENT 400	0.235	0.235
40	1 Triangular SCS	0.235	0.235
40	2 Proportional to %	0.235	0.235

218173- PostDev-Oct2019-Hazel.out

Line	Code	Description	Value	Unit
1	SCS method			
500	% - to golf course"			
25.000	% Impervious"			
1.690	Total Area"			
45.000	Flow length"			
4.000	Overland Slope"			
1.268	Pervious Area"			
45.000	Pervious length"			
4.000	Pervious slope"			
0.423	Impervious Area"			
15.000	Impervious length"			
4.000	Impervious slope"			
4.000	Pervious Manning "n"			
74.000	Pervious SCS Curve No. "			
0.730	Pervious Runoff coefficient"			
0.100	Pervious Ia/S coefficient"			
8.924	Pervious Initial abstraction"			
0.015	Impervious Manning "n"			
98.000	Impervious SCS Curve No. "			
0.928	Impervious Runoff coefficient"			
0.100	Impervious Ia/S coefficient"			
0.518	Impervious Initial abstraction"			
0.000	Catchment 300			
0.226	Pervious			
0.235	Impervious			
0.372	0.423			
1.533	2255.282			
285.008	1204.16			
76.946	264.484			
208.063	1117.45			
2637.19	0.928			
0.730	0.171			
0.055				
4	HYDROGRAPH Add Runoff "			
4	Add Runoff "			
0.226	0.226			
0.235	0.372"			
HYDROGRAPH Copy to Outflow"				
8	Copy to Outflow"			
0.226	0.226			
0.226	0.372"			
HYDROGRAPH " Combine 10"				
6	Combine " Combine 10"			
10	Node #"			
To Pond 201"				
Maximum flow				
Hydrograph volume				
0.226	0.226			
8471.973	0.518			
0.226	0.518"			
HYDROGRAPH Start - New Tributary"				
2	Start new tributary			
0.226	0.000			
0.226	0.226			
0.518"				
CATCHMENT 600"				
1	Triangular SCS"			
2	Proportional to %"			
SCS method				
1	SCS method			
600	% Impervious"			
0.000	% Total Area"			
0.550	Flow length"			
30.000	Overland Slope"			
4.000	Pervious Area"			
0.550	Pervious length"			
30.000	Pervious slope"			
4.000	Impervious Area"			
0.000	Impervious slope"			

Page 9

218173- PostDev-Oct2019-Hazel.out

[illegible]

Page 10 of 10

```

218173- PostDev-Oct2019-Haze1.out
424.200 1.475 1884.800"
Peak outflow 0.584 c.m/sec"
Maximum level 424.050 metre"
Maximum storage 1517.557 c.m"
Centroidal lag 43.210 hours"
0.075 0.584 0.000 c.m/sec"
HYDROGRAPH Combine
6 Combine 7"
7 Node #
Total to Creek"
Maximum flow 1.148 c.m/sec"
Hydrograph volume 18584.656 c.m"
0.075 0.590 1.148"
3 START/RE-START TOTALS 10"
Total Catchment area 21.320 hectare"
Total Impervious area 3.692 hectare"
Total % impervious 17.317"
EXIT"

```



**APPENDIX C:**  
**SWM FACILITY STAGE-STORAGE-DISCHARGE CALCULATIONS**

**Wilder Lake Subdivision  
Township of Southgate  
Our File: 218173  
November 2019**

**CATCHMENT 102 - Block 30 Stormwater Management Pond - Infiltration Pond with Outlet**

<b>Elevation (m)</b>	<b>Depth (m)</b>	<b>Width</b>	<b>Length</b>	<b>Active area (m<sup>2</sup>)</b>	<b>Active Volume (m<sup>3</sup>)</b>	<b>Acc. Active Storage (m<sup>3</sup>)</b>	
423.35	0.00	2.00	24.50	49	0.0	0.0	Pond Bottom
423.50	0.15	3.20	25.70	82	9.8	9.8	
423.65	0.30	4.40	26.90	118	15.0	24.9	
423.80	0.45	5.60	28.10	157	20.7	45.6	
423.95	0.60	6.80	29.30	199	26.7	72.3	Orifice Invert
424.10	0.75	8.00	30.50	244	33.2	105.6	
424.25	0.90	9.20	31.70	292	40.2	145.7	T/G - Low Side
424.40	1.05	10.40	32.90	342	47.5	193.3	
424.55	1.20	11.60	34.10	396	55.3	248.6	
424.70	1.35	12.80	35.30	452	63.6	312.1	Overflow Weir
424.85	1.50	14.00	36.50	511	72.2	384.4	Top of Bank

**Orifice Outlet**

Orifice Dia.: 100 mm  
Orifice Area: 0.008 m<sup>2</sup>  
Coefficient: 0.60  
Invert Elev: 423.95 m

**Overflow Weir**

Weir Inv. = 424.700 m  
Weir L = 5.000 m  
2g = 19.620  
Max. H = 0.150 m

**Stage-Storage-Discharge Table**

<b>Elevation (m)</b>	<b>Stage (m)</b>	<b>Storage (m<sup>3</sup>)</b>	<b>Flow from CB Orifice (m<sup>3</sup>/s)</b>	<b>Overflow Weir (m<sup>3</sup>/s)</b>	<b>Total Discharge (m<sup>3</sup>/s)</b>	
423.35	0.00	0.0	0.0000	0.000	0.0000	Pond Bottom
423.50	0.15	9.8	0.0000	0.000	0.0000	
423.65	0.30	24.9	0.0000	0.000	0.0000	
423.80	0.45	45.6	0.0000	0.000	0.0000	
423.95	0.60	72.3	0.0000	0.000	0.0000	Orifice Invert
424.10	0.75	105.6	0.0000	0.000	0.0000	
424.25	0.90	145.7	0.0000	0.000	0.0000	T/G - Low Side
424.40	1.05	193.3	0.0132	0.000	0.0132	
424.55	1.20	248.6	0.0155	0.000	0.0155	
424.70	1.35	312.1	0.0175	0.000	0.0175	Overflow Weir
424.85	1.50	384.4	0.0192	0.495	0.5145	Top of Pond Bank

**Wilder Lake Subdivision  
Township of Southgate  
Our File: 218173  
November 2019**

**CATCHMENT 101 - Block 31 Stormwater Management Pond - Infiltration Pond with Outlet**

<b>Elevation (m)</b>	<b>Depth (m)</b>	<b>Active area (m<sup>2</sup>)</b>	<b>Acc. Active Volume (m<sup>3</sup>)</b>	<b>Storage (m<sup>3</sup>)</b>	
423.00	0.00	10	0.0	0.0	Pond Bottom
423.25	0.25	112	15.3	15.3	Orifice Invert
423.50	0.50	315	53.4	68.6	
423.75	0.75	432	93.4	162.0	
424.00	1.00	551	122.9	284.9	
424.25	1.25	678	153.6	438.5	
424.50	1.50	812	186.3	624.8	T/G- Low Side
424.75	1.75	954	220.8	845.5	
425.00	2.00	1102	257.0	1102.5	
425.25	2.25	1257	294.9	1397.4	
425.50	2.50	1419	334.5	1731.9	Overflow Weir
425.75	2.75	1588	711.3	2108.6	Top of Pond Bank

**Orifice Outlet**

Orifice Dia.:	125	mm
Orifice Area:	0.012	m <sup>2</sup>
Coefficient:	0.60	
Invert Elev:	423.25	m

**Overflow Weir**

Weir Inv. =	425.500	m
Weir L. =	5.000	m
2g =	19.620	
Max. H =	0.250	m

**Stage-Storage-Discharge Table**

<b>Elevation (m)</b>	<b>Stage (m)</b>	<b>Storage (m<sup>3</sup>)</b>	<b>Flow from CB Orifice (m<sup>3</sup>/s)</b>	<b>Overflow Weir (m<sup>3</sup>/s)</b>	<b>Total Discharge (m<sup>3</sup>/s)</b>	
423.00	0.00	0.0	0.0000	0.000	0.0000	Pond Bottom
423.25	0.25	15.3	0.0000	0.000	0.0000	Orifice Invert
423.50	0.50	68.6	0.0000	0.000	0.0000	
423.75	0.75	162.0	0.0000	0.000	0.0000	
424.00	1.00	284.9	0.0000	0.000	0.0000	
424.25	1.25	438.5	0.0000	0.000	0.0000	
424.50	1.50	624.8	0.0000	0.000	0.0000	T/G - Low Side
424.75	1.75	845.5	0.0391	0.000	0.0391	
425.00	2.00	1102.5	0.0424	0.000	0.0424	
425.25	2.25	1397.4	0.0454	0.000	0.0454	
425.50	2.50	1731.9	0.0482	0.000	0.0482	Overflow Weir
425.75	2.75	2108.6	0.0509	1.066	1.1165	Top of Pond Bank

**Wilder Lake Subdivision  
Township of Southgate  
Our File: 218173  
November 2019**

**CATCHMENT 201 - Block 32 Stormwater Management Pond - Wet Pond**

<b>Elevation (m)</b>	<b>Depth (m)</b>	<b>W</b>	<b>L</b>	<b>Active area (m<sup>2</sup>)</b>	<b>Active Volume (m<sup>3</sup>)</b>	<b>Acc. Active Storage (m<sup>3</sup>)</b>	
423.30	0.00	30.00	56.00	1680	0.0	0.0	Culvert invert
423.40	0.10	31.00	57.00	1767	172.4	172.4	
423.50	0.20	32.00	58.00	1856	181.2	353.5	
423.60	0.30	33.00	59.00	1947	190.2	543.7	
423.70	0.40	34.00	60.00	2040	199.4	743.0	
423.80	0.50	35.00	61.00	2135	208.8	951.8	
423.90	0.60	36.00	62.00	2232	218.4	1170.1	Overflow Weir
424.00	0.70	37.00	63.00	2331	228.2	1398.3	
424.10	0.80	38.00	64.00	2432	238.2	1636.4	
424.20	0.90	39.00	65.00	2535	248.4	1884.8	Top of Pond Bank

**Orifice Outlet**

Orifice Dia.:	200	mm
Orifice Area:	0.031	m <sup>2</sup>
Coefficient:	0.60	
Invert Elev:	423.30	m

**Overflow Weir**

Weir Inv. =	423.900	m
Weir L =	5.000	m
2g =	19.620	
Max. H =	0.100	m

**Stage-Storage-Discharge Table**

<b>Elevation (m)</b>	<b>Stage (m)</b>	<b>Storage (m<sup>3</sup>)</b>	<b>Flow from CB Orifice (m<sup>3</sup>/s)</b>	<b>Overflow Weir (m<sup>3</sup>/s)</b>	<b>Total Discharge (m<sup>3</sup>/s)</b>	
423.30	0.00	0.0	0.000	0.000	0.000	Culvert invert
423.40	0.10	172.4	0.011	0.000	0.011	
423.50	0.20	353.5	0.026	0.000	0.026	
423.60	0.30	543.7	0.037	0.000	0.037	
423.70	0.40	743.0	0.046	0.000	0.046	
423.80	0.50	951.8	0.053	0.000	0.053	
423.90	0.60	1170.1	0.059	0.000	0.059	Overflow Weir
424.00	0.70	1398.3	0.065	0.270	0.334	
424.10	0.80	1636.4	0.070	0.762	0.832	
424.20	0.90	1884.8	0.075	1.401	1.475	Top of Pond Bank

**APPENDIX D:**  
**MIDUSS MODELLING - ENHANCED GRASS SWALES**

218173-EGS-25mm-updatedCatchments.out

MIDUSS Output  
 MIDUSS version Version 2.25 rev. 473  
 MIDUSS created Sunday, February 07, 2010  
 Units used: C:\Users\jswiger\Desktop\Wilder Lake\ MIDUSS  
 Job folder: 218173-EGS-25mm-updatedCatchments.out  
 Output filename: 218173-EGS-25mm-updatedCatchments.out  
 Licensee name: Hewlett-Packard Company, GMBH  
 Date & time last used: 11/7/2019 at 3:14:25 PM

TIME PARAMETERS

31 5.000 Time Step  
 240.000 Max. Storm Length  
 3600.000 Max. Hydrograph

32 640.170 Coefficient A  
 5.901 Constant B  
 0.840 Exponent C  
 0.375 Fraction R  
 240.000 Duration  
 1.000 Time step multiplier

Maximum intensity 70.168 mm/hr  
 Total depth 25.126 mm  
 6 025hyd Hydrograph extension used in this file  
 HYDROGRAPH start New Tributary  
 2 Start - New Tributary 0.000 0.000

33 CATCHMENT 500  
 1 Triangular SCS  
 2 Proportional to %  
 1 SCS method  
 500 Catchment 500 east of road  
 13.000 % Impervious  
 1.000 Total Area  
 45.000 Flow length  
 3.000 Overland Slope  
 0.870 Pervious Area  
 45.000 Pervious length  
 3.000 Pervious slope  
 0.130 Impervious Area  
 6.724 Impervious length  
 3.000 Impervious slope  
 0.250 Pervious Manning  
 75.000 Pervious SCS Curve No.  
 0.109 Pervious Runoff coefficient  
 8.467 Pervious Ia/S coefficient  
 0.015 Pervious Initial abstraction  
 98.000 Pervious SCS Curve No.  
 0.777 Pervious Runoff coefficient  
 0.100 Pervious Ia/S coefficient  
 0.518 Pervious Initial abstraction

Catchment 500 0.000 0.000  
 Surface Area 0.870  
 Time of concentration 41.914  
 Time to Centroid 188.309  
 Rainfall depth 25.126  
 Rainfall volume 218.59  
 Rainfall losses 22.387  
 Runoff depth 2.738  
 Runoff volume 23.82

0.000 c.m/sec  
 0.130  
 0.961  
 112.491  
 149.196  
 25.126  
 32.66  
 5.599  
 19.526  
 4.921  
 25.38  
 49.21  
 C.m"

Page 1

218173-EGS-25mm-updatedCatchments.out

Runoff coefficient 0.109 0.777 0.196  
 Maximum flow 0.004 0.021 0.021  
 HYDROGRAPH Add Runoff " 0.004 0.021 0.021  
 4 Add Runoff " 0.021 0.021 0.000

52 CHANNEL DESIGN  
 0.021 Current peak flow c.m/sec  
 Manning " n"  
 0 Cross-section type: 0=trapezoidal; 1=general  
 1.500 Basewidth metre  
 3.000 Left bank slope  
 3.000 Right bank slope  
 0.700 Channel depth metre  
 3.700 Gradient %  
 Depth of flow 0.028 metre  
 Velocity 0.482 m/sec  
 Channel capacity 7.831 c.m/sec  
 Critical depth 0.027 metre

53 ROUTE Channel Route 120  
 120.00 Channel Route 120 Reach length (metre)  
 0.498 X-factor <= 0.5  
 186.567 K-lag (seconds)  
 0.000 Default(0) on user spec.(1) values used  
 0.500 X-factor <= 0.5  
 30.000 K-lag (seconds)  
 0.500 Beta weighting factor  
 150.000 Routing time step (seconds)  
 1 No. of sub-reaches  
 Peak outflow 0.021 0.017 0.017  
 HYDROGRAPH Combine 100 0.017 0.000 c.m/sec  
 6 Combine " Node #"  
 100

40 Maximum flow 0.017 c.m/sec  
 Hydrograph volume 49.206 c.m"  
 0.021 0.017 0.017  
 HYDROGRAPH Start - New Tributary  
 2 Start - New Tributary 0.000 0.017 0.017

33 CATCHMENT 501  
 1 Triangular SCS  
 2 Proportional to %  
 1 SCS method  
 501 Catchment 500 west of road  
 21.000 % Impervious  
 0.520 Total Area  
 30.000 Flow length  
 3.000 Overland Slope  
 0.411 Pervious Area  
 30.000 Pervious length  
 3.000 Pervious slope  
 0.109 Impervious Area  
 7.975 Impervious length  
 3.000 Impervious slope  
 0.250 Pervious Manning  
 75.000 Pervious SCS Curve No.  
 0.109 Pervious Runoff coefficient  
 8.467 Pervious Ia/S coefficient  
 0.015 Pervious Initial abstraction  
 98.000 Pervious SCS Curve No.  
 0.783 Pervious Runoff coefficient

Page 2



218173-EGS-25mm-updatedCatchments.Out

0.100	Impervious Ia/s coefficient"	0.017	0.017	0.017	c.m/sec"
0.518	Impervious Initial abstraction"	0.018	0.000	0.017	c.m/sec"
	Catchment 501				
	Surface Area	0.411	0.109	0.520	hectare"
	Time of concentration	32.863	1.064	11.987	minutes"
	Time to Centroid	176.350	112.658	134.536	minutes"
	Rainfall depth	25.126	25.126	25.126	mm"
	Rainfall volume	103.522	27.44	130.655	c.m"
	Rainfall losses	24.388	3.443	18.850	mm"
	Runoff depth	2.738	19.883	6.296	mm"
	Runoff volume	11.25	21.49	32.74	c.m"
	Runoff coefficient	0.109	0.783	0.251	c.m/sec"
	Maximum flow				
	HYDROGRAPH Add Runoff "	0.002	0.017	0.018	c.m/sec"
40	4 Add Runoff "	0.018	0.018	0.017	
	CHANNEL DESIGN"				
52	0.018 Current peak flow	c.m/sec"			
	0.035 Manning "n"				
	0 Cross-section type: 0=trapezoidal; 1=general"				
	1.250 Basewidth metre"				
	3.000 Left bank slope"				
	3.000 Right bank slope"				
	0.700 Channel depth metre"				
	3.700 Gradient %"				
	Depth of flow	0.028	metre"		
	Velocity	0.484	m/sec"		
	Channel capacity	7.148	c.m/sec"		
	Critical depth	0.027	metre"		
53	ROUTE Channel Route 120"				
	X-factor <= 0.5"				
	K-lag (seconds)"				
	Default(0) or user spec.(1) values used"				
	0.000 X-factor <= 0.5"				
	0.500 K-lag (seconds)"				
	0.500 Beta weighting factor"				
	150.000 Routing time step (seconds)"				
	1 No. of sub-reaches"				
	Peak outflow	0.018	0.014	0.017	c.m/sec"
40	6 Combine "	100"			
	HYDROGRAPH "	Combine	100"		
	100 Node #"				
	Maximum flow				
	Hydrograph volume	0.031	c.m/sec"		
	0.018 81.946				
40	HYDROGRAPH Start - New Tributary"	0.018	0.014	0.031	
	2 Start - New Tributary"	0.018	0.000	0.031	
33	CATCHMENT 201"				
	1 Triangular SCS"				
	2 Proportional to %"				
	1 SCS method"				
	201 Catchment 201 north of the road"				
	33.000 % Impervious"				
	0.600 Total Area"				
	40.000 Flow length"				
	2.000 Overland Slope"				
	0.402 Pervious Area"				
	40.000 Pervious length"				

218173-EGS-25mm-updatedCatchments.Out

2.000	Pervious slope"				
0.198	Impervious Area"				
19.701	Impervious length"				
2.000	Impervious slope"				
0.250	Pervious Manning "n"				
75.000	Pervious SCS Curve No."				
0.109	Pervious Runoff coefficient"				
8.467	Pervious Ia/s coefficient"				
8.467	Pervious Initial abstraction"				
0.015	Impervious Manning "n"				
98.000	Impervious SCS Curve No."				
0.800	Impervious Runoff coefficient"				
0.100	Impervious Ia/s coefficient"				
0.518	Impervious Initial abstraction"				
	0.032 0.000				
	Catchment 201				
	Surface Area	0.402	Pervious	0.014	0.031 c.m/sec"
	Time of concentration	44.106			hectare"
	Time to Centroid	191.205			minutes"
	Rainfall depth	25.126			mm"
	Rainfall volume	101.00			c.m"
	Rainfall losses	22.388			mm"
	Runoff depth	2.737			mm"
	Runoff volume	11.00			c.m"
	Runoff coefficient	0.109			c.m/sec"
	Maximum flow				
	HYDROGRAPH Add Runoff "	0.002	0.032	0.032	
40	4 Add Runoff "	0.032	0.032	0.014	0.031
	CHANNEL DESIGN"				
	0.032 Current peak flow	c.m/sec"			
	0.035 Manning "n"				
	0 Cross-section type: 0=trapezoidal; 1=general"				
	0.750 Basewidth metre"				
	3.000 Left bank slope"				
	3.000 Right bank slope"				
	0.700 Channel depth metre"				
	1.000 Gradient %"				
	Depth of flow	0.075	metre"		
	Velocity	0.437	m/sec"		
	Channel capacity	3.018	c.m/sec"		
	Critical depth	0.053	metre"		
53	ROUTE Channel Route 120"				
	X-factor <= 0.5"				
	K-lag (seconds)"				
	Default(0) or user spec.(1) values used"				
	0.000 X-factor <= 0.5"				
	30.000 K-lag (seconds)"				
	0.500 Beta weighting factor"				
	150.000 Routing time step (seconds)"				
	1 No. of sub-reaches"				
	Peak outflow	0.032	0.028	0.028	c.m/sec"
	HYDROGRAPH "	Combine	100"		0.031 c.m/sec"
40	6 Combine "	100			
	100 Node #"				
	Maximum flow				
	Hydrograph volume	0.032	0.032	0.059	c.m/sec"
	0.032 132.762				c.m"
	HYDROGRAPH Start - New Tributary"				0.059"
	2 Start - New Tributary"				

218173-EGS-25mm-updatedCatchments.Out  
0.032 0.000 0.028 0.059"

CATCHMENT 202"  
1 Triangular SCS"  
2 Proportional to %"  
SCS method"  
202 Catchment 201 - south of the road"  
15.000 % Impervious"  
1.500 Total Area"  
40.000 Flow Length"  
2.000 Overland Slope"  
1.275 Pervious Area"  
40.000 Pervious Length"  
2.000 Pervious Slope"  
9.059 Impervious Area"  
2.000 Impervious Length"  
2.000 Impervious Slope"  
75.000 Pervious Manning "n"  
0.109 Pervious SCS Curve No."  
0.109 Pervious Runoff coefficient"  
0.100 Pervious Ia/S coefficient"  
8.467 Pervious Initial abstraction"  
0.015 Impervious Manning "n"  
98.000 Impervious SCS Curve No."  
0.786 Impervious Runoff coefficient"  
0.100 Impervious Ia/S coefficient"  
0.518 Impervious Initial abstraction"  
CATCHMENT 202 Pervious Total Area " hectare"  
Surface Area 1.275 1.500  
Time of concentration 44.106 20.028  
Time to Centroid 191.205 147.229  
Rainfall depth 25.126 25.126  
Rainfall volume 56.53 376.88  
Rainfall losses 22.388 19.836  
Runoff depth 2.737 19.751  
Runoff volume 34.90 44.44  
Runoff coefficient 0.109 0.786  
Maximum flow 0.005 0.036  
4 Add Runoff " 0.036 0.028 0.059"

CHANNEL DESIGN"  
0.036 Current peak flow c.m/sec"  
0.035 Manning "n"  
0.750 Cross-section type: 0=trapezoidal; 1=general"

0.750 Base width metre"  
3.000 Left bank slope"  
3.000 Right bank slope"  
1.000 Channel depth metre"  
Depth of flow 0.080 metre"  
Velocity 0.453 m/sec"  
Channel capacity 3.018 c.m/sec"  
Critical depth 0.057 metre"  
ROUTE Channel Route 120"  
Channel Route 120 Reach length (metre)"  
X-factor <= 0.5"  
K-lag (seconds)"  
Default(0) or user spec.(1) values used"  
0.500 X-factor <= 0.5"  
30.000 K-lag (seconds)"  
0.500 Beta weighting factor"  
150.000 Routing time step (seconds)"  
Page 5

218173-EGS-25mm-updatedCatchments.Out  
0.032 0.036 0.030 0.030 0.059 c.m/sec"

1 No. of sub-reaches"  
Peak outflow 0.036 0.036 100" 0.030 c.m/sec"  
HYDROGRAPH Combine 100" 0.030 c.m/sec"  
6 Node #"  
100 Maximum flow 0.089 c.m/sec"  
Hydrograph volume 212.105 c.m"  
0.036 0.030 0.089"  
HYDROGRAPH Start - New Tributary"  
2 Start - New Tributary"  
0.036 0.000 0.030 0.089"  
CATCHMENT 203"  
1 Triangular SCS"  
2 Proportional to %"  
SCS method"  
203 203 - Parking Lot"  
90.000 % Impervious"  
0.300 Total Area"  
66.000 Flow Length"  
2.000 Overland Slope"  
0.030 Pervious Area"  
66.000 Pervious Length"  
2.000 Pervious Slope"  
594.000 Impervious Area"  
2.000 Impervious Length"  
2.000 Impervious Slope"  
75.000 Pervious Manning "n"  
0.109 Pervious SCS Curve No."  
0.100 Pervious Runoff coefficient"  
8.467 Pervious Ia/S coefficient"  
0.015 Impervious Initial abstraction"  
98.000 Impervious Manning "n"  
0.807 Impervious SCS Curve No."  
0.100 Impervious Runoff coefficient"  
0.518 Impervious Initial abstraction"  
CATCHMENT 203 Pervious Total Area " hectare"  
Surface Area 0.030 0.270 0.300  
Time of concentration 59.565 15.961  
Time to Centroid 211.840 135.206  
Rainfall depth 25.126 25.126  
Rainfall volume 67.847 75.38  
Rainfall losses 22.387 4.857  
Runoff depth 2.738 20.288  
Runoff volume 0.82 54.72  
Runoff coefficient 0.109 0.807  
Maximum flow 0.000 0.026 0.026  
4 Add Runoff " 0.026 0.026 0.030 0.089"  
CHANNEL DESIGN"  
0.026 Current peak flow c.m/sec"  
0.035 Manning "n"  
0.750 Cross-section type: 0=trapezoidal; 1=general"  
0.750 Base width metre"  
3.000 Left bank slope"  
3.000 Right bank slope"  
0.700 Channel depth metre"  
1.000 Gradient %"  
Depth of flow 0.067 metre"  
Page 6

```

218173-EGS-25mm-updatedCatchments.Out
Velocity 0.409 m/sec"
Channel capacity 3.018 c.m/sec"
Critical depth 0.047 metre"
ROUTE Channel Route 45"
45.00 Channel Route 45 Reach length (metre)"
0.458 X-factor <= 0.5"
82.493 K-lag (seconds)"
0.000 Default(0) or user spec.(1) values used"
0.500 X-factor <= 0.5"
30.000 K-lag (seconds)"
0.500 Beta weighting factor"
75.000 Routing time step (seconds)"
1 No. of sub-reaches"
Peak outflow 0.024 c.m/sec"
HYDROGRAPH " Combine 100" 0.024 0.089 c.m/sec"
100 Node #"
Maximum flow 0.105 c.m/sec"
Hydrograph volume 267.651 c.m"
0.026 0.105"
38 START/RE-START TOTALS 203"
3 Runoff Totals on EXIT"
Total Catchment area 3.920 hectare"
Total Impervious area 0.932 hectare"
Total % impervious 23.781"
EXIT"
19

```

## **APPENDIX E: MAINTENANCE CHECKLIST**

## STORMWATER MANAGEMENT FACILITY MAINTENANCE CHECKLIST

**WILDER LAKE SUBDIVISION  
263512 SOUTHGATE ROAD 26  
SOUTHGATE  
GMBP FILE: 218173**

To ensure that the stormwater management system continues to function as designed and constructed, we recommend that the following inspections and maintenance activities be completed by the owner on an annual basis.

	Yes	No	Date Completed
1. Is there any noticeable damage to the facility structures (i.e. catchbasins, overflow weirs)?  If yes, complete the necessary repairs and/or installation of replacement structures.	<input type="checkbox"/>	<input type="checkbox"/>	
2. Inspect on-site catchbasins. Is cleanout required for: - accumulated sediment - trash/litter - debris (i.e. sediment, garbage, leaves, etc.). - any indication of a spill (frothy water, oily sheen)  If yes, arrange for clean-out and disposal of contents as required.	<input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/>	<input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/>	
3. Inspect Stormwater Management Facility. Is cleanout required for: - accumulated sediment - trash/litter - debris (i.e. sediment, leaves, etc.). - any indication of a spill (frothy water, oily sheen)  If yes, arrange for clean-out and disposal of contents as required.	<input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/>	<input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/>	
4. Inspect the water level in the SWM pond. - Has the pond completely drained within 48 hours after a storm?  If not, inspect the downstream structures and remove and dispose of any accumulated sediment, trash/litter and debris.	<input type="checkbox"/>	<input type="checkbox"/>	
5. Are any components of structures missing parts or are damaged?	<input type="checkbox"/>	<input type="checkbox"/>	

If yes, complete the necessary repairs or replacement in a timely manner.			
---	--	--	--

Should an accidental spill occur on-site, the Emergency Procedures and the site specific Spill Control and Clean Up would be implemented. As required by the Technical Standards and Safety Authority, the Spills Action Centre (Hotline #1-800-268-6060) would be contacted when necessary.

Signature of Inspector \_\_\_\_\_.

Date of Inspection \_\_\_\_\_.



**APPENDIX F:**  
**DRIVEWAY CULVERT CALCULATIONS AND STORM SEWER**  
**DESIGN SHEET**

### **Driveway Culvert Sizing**

Flows in Catchment 101 and 102 are conveyed only in ditches and culverts. Flows in Catchment 201 and 500 are conveyed in ditches and culverts and then conveyed in a storm sewer system.

The largest upstream area that flows through a driveway culvert is in Catchment 101 on the south-east side of roadway before the culvert that outlets to the Block 31 SWM Pond. This catchment area is 1.83 ha. The slope along this roadway is 0.5%. Driveway culvert diameters are 375 mm as per Southgate Municipal Standards minimum sizing.

Storm	Proposed Post-Development Flow Rate (m <sup>3</sup> /s)	Maximum Flow in Culvert <sup>1</sup> (m <sup>3</sup> /s)	Weir Flow over Driveway (m <sup>3</sup> /s)	Flow Depth over Driveway <sup>2</sup> (m)
10 Year	0.121	0.124	0.0	0.0
Regional (Hurricane Hazel)	0.259	0.124	0.135	0.082

- 1) With 0.5% slope, the maximum capacity of the culvert is 0.124 m<sup>3</sup>/s, based on MIDUSS modelling.
- 2) Weir flow calculation, assuming the driveway can be modelled as a rectangular weir with a width of 4 m.

The ditch was modelled as a channel using MIDUSS with 3:1 side slopes, total depth of 0.93 m, and a manning 'n' of 0.040. The peak flow rate of 0.259 m<sup>3</sup>/s would result in a flow depth of 0.387 m. Therefore the culverts and driveway weirs determine the flow.

Therefore the flow can be conveyed in the ditches, culverts, and over driveway low points, without ponding on private property.


### **Storm Sewer Sizing (Catchment 201 and 500)**

See attached Storm Sewer design sheet, completed with the 5 year storm and the 100 year storm

Pike Lake Loam – Hydrologic Soil Group B - loam

From MTO Design Chart 1.07, for 7% sloping lawns with soil between sandy and clay, runoff coefficient c 0.15 for pervious area. For impervious land, c = 0.9. With 30% imperviousness, c = 0.30, which is near the middle of the range for suburban residential areas.

The storm sewer is designed to convey up to and including the 100 year storm.

Design Storm			A	B	Where Q = peak flow in litres per second (L/s) A = area in hectares (ha) I = rainfall intensity in millimetres per hour (mm/h) C = runoff coefficient A, B = design storm coefficients Tc = time of concentration in minutes (MIN. 10.0 minutes)	ESTIMATED DESIGN FLOWS														DESIGN:		PROJECT:		SHEET NO. 1 of 1	
			21.9	-0.701																IE & JS GM BluePlan Engineering Ltd		Wilder Lake Subdivision			
5 yr			27.5	-0.693																					
10 yr			31.2	-0.689																					
25 yr			35.9	-0.686																					
50 yr			39.4	-0.683																					
100 yr			42.9	-0.682															DATE: OCT 2019		PROJECT: 218173				
LOCATION					AREA (ha)				Individual 2.78 AC	Cumulative 2.78 AC	Time of Concentration Tc (minutes)	Rainfall Intensity I (mm/h)	Peak Flow Q (L/s)	Constant Flow (L/s)	Cumulative Constant Flow (L/s)	Total Flow (L/S)	SEWER DATA								
Catchment	STREET	FROM (STRUCTURE)	TO (STRUCTURE)	R =	R =	R =	R =	Type of Pipe									Roughness Coefficient (n)	Diameter (mm)	Slope (%)	Length (m)	Capacity (L/s)	Full Flow Velocity (m/s)	Time of Flow (minutes)		
				0.30																					
5 year																									
500		CB-1	CB-2	1.15				0.96	0.96	10.00	95	91				91	PERF PE	0.0130	300	0.60	15.0	75	1.06	0.24	
500		CB-2	CBMH-3	0.57				0.48	1.43	10.24	94	134				134	PERF PE	0.0130	375	1.30	100.0	200	1.81	0.92	
201		CB-5	CB-4	0.62				0.52	0.52	10.00	95	49				49	PERF PE	0.0130	300	0.50	14.5	68	0.97	0.25	
201		CB-4	CBMH-3	1.43				1.19	1.71	10.25	94	160				160	PERF PE	0.0130	450	0.50	27.0	202	1.27	0.36	
600		CBMH-3	Outlet	0.20				0.17	3.31	11.16	88	292				292	PERF PE	0.0130	450	7.50	10.0	701	4.91	0.03	
100 yr																									
500		CB-1	CB-2	1.15				0.96	0.96	10.00	146	140				140	PERF PE	0.0130	300	2.50	15.0	153	2.16	0.12	
500		CB-2	CBMH-3	0.57				0.48	1.43	10.12	144	207				207	PERF PE	0.0130	375	1.40	100.0	207	1.88	0.89	
201		CB-5	CB-4	0.62				0.52	0.52	10.00	146	75				75	PERF PE	0.0130	300	1.00	14.5	97	1.37	0.18	
201		CB-4	CBMH-3	1.43				1.19	1.71	10.18	144	246				246	PERF PE	0.0130	450	1.00	27.0	285	1.79	0.25	
600		CBMH-3	Outlet	0.20				0.17	3.31	11.00	136	452				452	PERF PE	0.0130	450	3.50	10.0	533	3.35	0.05	

**APPENDIX G:  
GEOTECHNICAL INVESTIGATION, CMT ENGINEERING INC.**

# **GEOTECHNICAL INVESTIGATION**

**WILDER LAKE DEVELOPMENT  
263530 SOUTHGATE ROAD 26  
TOWNSHIP OF SOUTHGATE, ONTARIO**

**CMT Project 18-368.R01**

**Prepared for:**

**H. Bye Construction Ltd.**

**December 12, 2018**





*CMT Engineering Inc.*

**CONSULTING ENGINEERS**

1011 Industrial Crescent, Unit 1  
St. Clements, Ontario N0B 2M0  
Tel: 519-699-5775  
Fax: 519-699-4664  
www.cmtinc.net

December 12, 2018

18-368.R01

H. Bye Construction Ltd.  
397 Church Street North  
Mount Forest, Ontario  
N0G 2L0

Attention: Mr. Harry Bye, P.Eng.

Dear Sir:

**Re: Geotechnical Investigation  
Wilder Lake Development  
263530 Southgate Road 26, Southgate  
Township of Southgate, Ontario**

As requested, CMT Engineering Inc. conducted a geotechnical investigation at the above-referenced site, and we are pleased to present the enclosed report.

We trust that this information meets your present requirements and we thank you for allowing us to undertake this project. Should you have any questions, please do not hesitate to contact our office.

Yours very truly,

A handwritten signature in blue ink, appearing to read 'Shawn Wheatley', with a stylized flourish extending from the end.

Shawn Wheatley, M.Eng.

ks



## TABLE OF CONTENTS

	<u>Page</u>
1.0 INTRODUCTION .....	1
2.0 EXISTING SITE CONDITIONS .....	1
3.0 FIELD AND LABORATORY PROCEDURES .....	1
4.0 SUBSOIL CONDITIONS .....	2
4.1. Topsoil .....	3
4.2. Fill .....	3
4.3. Silty Gravelly Sand .....	3
4.4. Gravel and Sand .....	3
4.5. Sand .....	4
4.6. Sandy Gravel/Gravel .....	4
4.7. Sand and Silt .....	4
4.8. Groundwater .....	4
5.0 DISCUSSION AND RECOMMENDATIONS .....	5
5.1. Serviceability and Ultimate Limit Pressure .....	6
5.2. Seismic Site Classification .....	8
5.3. Soil Design Parameters .....	8
5.4. Site Preparation .....	8
5.4.1. Topsoil Stripping/Vegetation Grubbing .....	9
5.4.2. Fill/Unsuitable Soil Removal .....	9
5.4.3. Removal/Relocation of Existing Services .....	10
5.4.4. Site Grading .....	10
5.5. Foundation Subgrade Preparation .....	12
5.6. Slab-on-Grade/Modulus of Subgrade Reaction .....	13
5.7. Excavations .....	14
5.8. Construction Dewatering Considerations .....	14
5.9. Service Pipe Bedding .....	15
5.10. Perimeter Building Drainage, Foundation Wall Backfill and Trench Backfill .....	15
5.11. Pavement Design/Drainage .....	17
5.12. Excess Soil Management .....	20
5.12.1 Chemical Testing was NOT Undertaken .....	20
5.12.2 TCLP Requirement .....	20
5.13. Infiltration and Coefficient of Permeability/T-Time .....	21
5.14. Radon .....	22
6.0 SITE INSPECTIONS .....	22
7.0 LIMITATIONS OF THE INVESTIGATION .....	22
Drawing 1 - Site Location Map	
Drawing 2 - Site Plan Showing Borehole Locations	
Appendix A - Borehole Logs	
Appendix B - Grain Size Analyses	
Appendix C - Well Records	

## **1.0 INTRODUCTION**

The services of CMT Engineering Inc. (CMT Inc.) were retained by Mr. Harry Bye, P.Eng. of H. Bye Construction Ltd., to conduct a geotechnical investigation for a proposed residential subdivision located at 263530 Southgate Road 26 in Southgate, Ontario. The geotechnical investigation was carried out in conjunction with a hydrogeology study completed by GM BluePlan Engineering Limited, which will be provided by GM BluePlan under separate cover. The location of the site is shown on Drawing 1.

It is understood that the subject property has been previously developed as a golf course resort and that some of the golf lands will be redeveloped to an estate residential development, with some of the lots fronting onto Wilder Lake. Preliminary plans for the project comprise the construction of twenty-five (25) estate residential lots that will be serviced by individual wells and septic systems.

The purpose of the geotechnical investigation was to assess the existing soil and groundwater conditions encountered in the boreholes. Included in the assessment are the soil classification and groundwater observations, as well as comments and recommendations regarding geotechnical resistance (bearing capacity); serviceability limit states (anticipated settlement); dewatering considerations; site classification for seismic site response; recommendations for site grading, site servicing, excavations and backfilling; recommendations for slab-on-grade construction; pavement design/drainage; soil design properties; storm water infiltration; percolation rate (T-time); and a summary of the laboratory results.

## **2.0 EXISTING SITE CONDITIONS**

The site is currently the Homestead Resort, and mainly comprises a golf course with light vegetation as well as mature trees. The site generally slopes down to the east towards Wilder Lake, with over 5.0 m change in elevation. The site is bounded by Wilder Lake to the east, Southgate Township Road 26 (Homestead Road) to the north, agricultural land to the west, and forested land to the south.

## **3.0 FIELD AND LABORATORY PROCEDURES**

The drilling field investigation was conducted on July 27, 2018, and comprised the advancement of six (6) boreholes (referenced as Boreholes 1 to 6), utilizing a Geoprobe 7822DT drillrig operated by employees of CMT Drilling Inc.

The borehole depths ranged from 6.10 m (20.0 ft) to 7.62 m (25.0 ft) below the existing ground surface elevations. Soil sampling was undertaken utilizing the Standard Penetration Test (SPT), as well as Macro Core (MC5) systems for Boreholes 1 to 6. Standard Penetration Testing (SPT) was generally conducted at 0.76 m (2.5 ft) intervals to a depth of 3.66 m (12.0 ft), after which SPT sampling was conducted at 1.5 m (5.0 ft) intervals to borehole termination. MC5 continuous sampling was conducted between the 1.5 m (5.0 ft) SPT sampling intervals. Technical staff from

CMT Inc. observed the drilling operation and collected and logged the recovered soil samples. A small portion of each sample was placed in a sealed, marked jar for moisture content determinations.

As requested, representative samples from the following boreholes and depths were submitted to our laboratory for grain size analyses:

- Borehole 1 - depth 1.52 m to 2.13 m (5.0 ft to 7.0 ft)
- Borehole 2 - depth 1.22 m to 2.13 m (4.0 ft to 7.0 ft)
- Borehole 3 - depth 0.76 m to 1.37 m (2.5 ft to 4.5 ft)
- Borehole 4 - depth 0.76 m to 1.37 m (2.5 ft to 4.5 ft)
- Borehole 5 - depth 0.76 m to 1.37 m (2.5 ft to 4.5 ft)
- Borehole 6 - depth 0.76 m to 1.37 m (2.5 ft to 4.5 ft)

CMT Inc. may be contacted for additional laboratory testing on bagged samples should it be required. Samples are normally kept for three months, unless other arrangements are made.

The borehole logs are provided in Appendix A and the resulting grain size analyses can be found in Appendix B.

As requested, each borehole was equipped with 38 mm diameter monitoring well comprising a 3.0 m long prepack screen with a sand filter, then riser pipe backfilled with bentonite, a J-plug and then a locking monument style protective cover. The monitoring wells were installed according with the Ontario Water Resources Act, Regulation 903 (O.Reg. 903) by well technicians licensed by the Ministry of the Environment (MOE), working for a contractor also licensed by the MOE. The monitoring wells are registered with the MOE in accordance with O.Reg. 903 and must be decommissioned in accordance with Reg. 903 when they are no longer required. CMT Drilling Inc. can provide the decommissioning services. The well log records are provided in Appendix C.

Wilson Ford Surveying conducted a survey of the site, including ground surface elevations at the borehole locations. The ground surface elevations at the borehole locations ranged from 425.26 m to 430.64 m. The locations of the boreholes are shown on Drawing 2.

#### **4.0 SUBSOIL CONDITIONS**

The soils encountered in the boreholes are described briefly below and a more detailed stratigraphic description is provided on the borehole logs in Appendix A.

#### **4.1. Topsoil**

Very loose to loose, dark brown, silty, organic topsoil was encountered at the surface of all boreholes. The topsoil was considered moist. The topsoil ranged in thickness from 150 mm to 250 mm (average 200 mm) at the borehole locations. Given the variation in topsoil thicknesses at the borehole locations, and the undulating nature of the topography, some variance in the topsoil thickness should be expected outside of the sampled areas.

#### **4.2. Fill**

Dark brown silt fill was encountered underlying the topsoil in Borehole 2. The fill was considered loose, with an SPT N-value of 7 blows per 0.30 m. The fill was considered moist, with moisture contents ranging from 6.4% to 8.3% (average 7.4%).

#### **4.3. Silty Gravelly Sand**

Brown silty sand and gravel or silty gravelly sand, with trace clay, was encountered underlying the topsoil in Boreholes 1, 3 and 4. Occasional sand layers were encountered in the silty gravelly sand in Borehole 4. The silty gravelly sand was considered very loose to compact, with SPT-N values ranging from 2 to 13 blows per 0.30 m (average 8 blows per 0.30 m). The silty gravelly sand was considered moist, with moisture contents ranging from 6.8% to 11.7% (average 9.5%).

#### **4.4. Gravel and Sand**

Brown gravel and sand with trace silt and clay was encountered underlying the topsoil in Borehole 6, the silty gravelly sand as well as the sand in Borehole 1, the sandy gravel in Borehole 4, and the sand and silt in Borehole 2. Occasional cobbles were encountered in the gravel and sand in Borehole 6, and occasional thin silt layers were encountered in the gravel and sand in Borehole 1. The gravel and sand was considered compact to very dense, with SPT N-values ranging from 10 to in excess of 100 blows per 0.30 m (average 52 blows per 0.30 m). The gravel and sand was considered damp to saturated, with moisture contents ranging from 2.4% to 14.4% (average 5.7%).

#### **4.5. Sand**

Brown medium-fine to coarse sand was encountered underlying the gravel and sand in Borehole 1. The sand was considered compact, with SPT N-values ranging from 18 to 23 blows per 0.30 m (average 20 blows per 0.30 m). The sand was considered damp to moist, with moisture contents ranging from 2.2% to 6.3% (average 4.3%).

#### **4.6. Sandy Gravel/Gravel**

Brown to grey sandy gravel with some sand to sandy, some silt, trace clay, and occasional cobbles, was encountered underlying the fill in Borehole 2, underlying the silty gravelly sand in Borehole 4, underlying the gravel and sand in Borehole 4, and underlying the topsoil in Borehole 5. Occasional sand layers and clay layers were encountered in the sandy gravel in Borehole 3. Brown gravel with some sand and silt, trace clay and cobbles, was encountered under the silty sand and gravel in Borehole 3. The sandy gravel and gravel was considered compact to very dense, with SPT N-values ranging from 10 to in excess of 100 blows per 0.30 m (average 33 blows per 0.30 m). The sandy gravel was considered damp to saturated, with moisture contents ranging from 2.1% to 18.8% (average 8.8%).

#### **4.7. Sand and Silt**

Brown sand and silt with trace gravel to gravelly, and trace clay was encountered underlying the gravel and sand in Boreholes 1 and 6, as well as underlying the sandy gravel in Borehole 2. The sand and silt contained occasional thin clay layers in Borehole 1, and occasional cobbles in Borehole 2. The sand and silt was considered compact to dense, with SPT N-values ranging from 13 to 31 blows per 0.30 m (average 23 blows per 0.30 m). The sand and silt was considered wet to saturated, with moisture contents ranging from 8.2% to 16.4% (average 11.0%).

#### **4.8. Groundwater**

As requested, all six (6) boreholes were each equipped with a monitoring well. The monitoring wells were installed and registered in accordance with the Ontario Water Resources Act, Regulation 903 (O.Reg. 903) by well technicians licensed by the Ministry of the Environment (MOE), working for a contractor also licensed by the MOE.

GM BluePlan Engineering Limited staff measured the water levels on August 2, 2018. The following table summarizes the borehole number, ground surface elevation, elevation of water in the monitoring well, cave elevation, zone of saturation and the bottom of borehole elevation for each borehole.

<b>Borehole No.</b>	<b>Ground Surface Elevation (m)</b>	<b>Measured Elevation of Water in Monitoring Well August 2, 2018 (m)</b>	<b>Zone of Saturation Elevation (m)</b>	<b>Bottom of Borehole Elevation (m)</b>
1	425.64	420.97	420.76 to termination	419.54
2	430.64	(dry to bottom)	--	423.02
3	425.90	421.83	423.61 to 423.00 421.33 to termination	419.80
4	425.54	(dry to bottom)	--	417.92
5	425.26	421.88	422.21 to termination	419.16
6	427.30	421.64	422.27 to termination	421.20

It should be noted that the very dense and/or fine-grained, less permeable soils encountered in the boreholes have the potential to create perched water conditions. Perched water conditions are generally dependent on the amount of precipitation, control of surface water, as well as the time of year, and can fluctuate significantly in elevation and volume. As such, provisions for site dewatering should be part of the site development and construction process. Recommendations with respect to dewatering conditions are provided in Section 5.8 of this report.

## **5.0 DISCUSSION AND RECOMMENDATIONS**

It is understood that the subject property has been previously developed as a golf course resort and that some of the golf lands will be redeveloped to an estate residential development, with some of the lots fronting onto Wilder Lake. Preliminary plans for the project comprise the construction of twenty-five (25) estate residential lots that will be serviced by individual wells and conventional septic systems.

Utilizing the information gathered during the geotechnical investigation and assuming that the borehole information is representative of the subsoil conditions throughout the site, the following comments and recommendations are provided.



### 5.1. Serviceability and Ultimate Limit Pressure

Based on the information obtained from the boreholes, the following table provides the Serviceability Limit States (SLS) and Ultimate Limit States (ULS) pressures at the various elevations, including soil types:

Borehole No.	Ground Surface Elevation (m)	SLS kPa (psf)	ULS kPa (psf)	Estimated Founding Elevations (m)	Depth Below Existing Grade to the Highest Founding Elevation (m)	Soil Type
1	425.64	150 (3,000)	225 (4,500)	424.88 to 419.54 (termination)	0.76	Gravel and Sand/Sand/Sand and Silt
2	430.64	150 (3,000)	225 (4,500)	429.42 to 423.02 (termination)	1.22	Sandy Gravel/Sand and Silt/Gravel and Sand
3	425.90	150 (3,000)	225 (4,500)	425.75 to 419.80 (termination)	0.15	Silty Sand and Gravel/Gravel
4	425.54	150 (3,000)	225 (4,500)	425.34 to 417.92 (termination)	0.20	Silty Gravelly Sand/Sandy Gravel/Gravel and Sand
5	425.26	150 (3,000)	225 (4,500)	425.11 to 419.16 (termination)	0.15	Sandy Gravel
6	427.30	150 (3,000)	225 (4,500)	427.05 to 421.20 (termination)	0.25	Gravel and Sand/Sand and Silt

Based on the bearing capacities and elevations provided in the table above, suitable founding elevations for conventional foundations designed with a minimum bearing capacity of 150 kPa (3,000 psf) at SLS and 225 kPa (4,500 psf) at ULS range below elevations 424.88 m to 429.42 m for Boreholes 1 to 6. It should be noted that the above-referenced elevations of soils capable of supporting foundations designed with a bearing capacity of 150 kPa (3,000 psf) at SLS and 225 kPa (4,500 psf) at ULS corresponds with depths ranging from approximately 0.15 m to 1.22 m below the existing ground surface at the borehole locations.

Soils capable of supporting foundations are generally encountered below the topsoil and upper zone of fill or loose native soils. Therefore, the topsoil, fill, and relatively loose native soils must be subexcavated in the areas of the proposed structures. The founding soil must be assessed at the time of construction by qualified geotechnical personnel in order to confirm their founding suitability. Alternatively, loose native soils encountered directly underlying the topsoil could be subjected to further compactive effort and testing, provided they contain no organic or other deleterious material, and provided the moisture content is suitable to achieve the specified compaction of 100% SPMDD.

Should footings be designed to be constructed at elevations higher than the elevations indicated in the table above, then structural fill will be required in order to achieve the design grades for the proposed foundations. The serviceability limit pressure for structural fill placed and compacted in accordance with Section 5.4.4 of this report and constructed on approved competent native soils is estimated to be at least 150 kPa (3,000 psf) at SLS and 225 kPa (4,500 psf) at ULS. Alternatively, footings could be stepped down to bear on approved undisturbed founding soils.

Footings may be placed at a higher elevation relative to another footing provided that the slope between the outside face of the footings are separated by a minimum slope of 10 horizontal to 7 vertical (10H:7V) with an imaginary line projected from the underside of the footings. This must be taken into account for any deep structures such as sump pits.

With respect to the Serviceability Limit State (SLS), the total and differential footing settlements are not expected to exceed the generally acceptable limits of 25 mm (1") and 19 mm (3/4") respectively.

All exterior footings must be provided with a minimum of 1.2 m of soil cover or equivalent thermal insulation (sufficient thermal insulation is required to protect all footings and slab-on-grades during construction until such a time that the structure is heated) in order to provide protection from frost action.

It should be noted that depending on the final grades, the native founding soils may be in a wet to saturated state; therefore, dewatering during construction may be required (see Section 5.8 of this report) along with the potential construction of a mud mat or granular drainage layer.

At the time of investigation, the proposed founding elevations were not available. CMT Inc. would be pleased to review design drawings when they become available and provide further recommendations with respect to bearing and foundation elevations.

### 5.2. Seismic Site Classification

The site classification for seismic response in Table 4.1.8.4 of the 2012 Ontario Building Code relates to the average properties of the upper 30 m of strata. The information obtained in the geotechnical field investigation was gathered from the upper 7.62 m of strata. Based on the information gathered in the geotechnical field investigation, the site classification for seismic site response would be considered Site Class D (stiff soil) for structures founded on the native soils at the recommended founding elevations provided in Section 5.1 of this report or on structural fill placed in accordance with section 5.4.4 of this report. The structural engineer responsible for the design of the structure should review the earthquake loads and effects.

### 5.3. Soil Design Parameters

The following table provides the soil design parameters for imported granular fill, as well as the native soils encountered on-site. The soil design parameters can be utilized for the design of perimeter shoring, foundations and retaining walls, as required:

Soil Type	Soil Density (kg/m <sup>3</sup> )	Friction Angle (Degree)	Coefficient of Active Pressure (K <sub>a</sub> )	Coefficient of Passive Pressure (K <sub>p</sub> )	Coefficient of At-Rest Pressure (K <sub>0</sub> )	Coefficient of Friction (μ)
Imported Gran 'A'/Gran 'B' (OPSS 1010)	2,100	34 °	0.28	3.54	0.44	0.45
Silty Gravelly Sand	2,000	32 °	0.30	3.25	0.47	0.41
Gravel and Sand	1,950	34 °	0.28	3.54	0.44	0.45
Sand	1,850	31 °	0.32	3.12	0.48	0.40
Sandy Gravel	1,900	31 °	0.32	3.12	0.48	0.40
Sand and Silt	1,800	32 °	0.30	3.25	0.47	0.41

### 5.4. Site Preparation

The site preparation for the proposed new residential development will include topsoil stripping, vegetation grubbing, the subexcavation of all fill and relatively loose native soils deemed not capable of supporting the design bearing capacity (or remedial action which may include air drying and the application of further compactive effort), the removal or relocation of any existing services (field tiles), followed by the placement of structural fill (as required) and site grading to achieve proposed grades.

#### **5.4.1. Topsoil Stripping/Vegetation Grubbing**

All topsoil must be removed from within the proposed building, road and driveway envelopes to expose approved competent subgrade soils. The topsoil may be used in landscaped areas where some settlement can be tolerated; otherwise it should be properly disposed of off-site. Due to the undulating topography and erosion, it should be expected that the thickness of topsoil may vary significantly throughout the site.

All vegetation and trees (including tree root structures as well as any loose soils that are typically associated with root structures) must be removed from within the proposed building and driveway envelopes to expose approved competent subgrade soils.

The volume of topsoil removed during the stripping process can be influenced by the equipment utilized for the stripping process as well as the moisture conditions at the time of stripping. If an excavator with a smooth bucket is utilized for stripping, there would generally be less potential for topsoil to become intermixed with the underlying, generally loose to very loose subsoil and therefore less concern of over-excavation to remove all topsoil. If the topsoil is stripped with wheeled equipment or bulldozers, then there is an increased potential for the topsoil and subsoil to become intermixed, subsequently requiring additional excavation to remove all topsoil. This is further influenced by rutting which can occur during wet conditions.

#### **5.4.2. Fill/Unsuitable Soil Removal**

The existing fill typically exists in a loose state and is not considered structurally sound to support foundations or the slab-on-grade for the proposed buildings. Therefore, all existing fill (including any existing trench backfill and sand from existing sand traps), as well as any native soil that has inadequate bearing capacity or has been disturbed by the golf course construction processes and is deemed unsuitable to support foundations or slab-on-grades, must be subexcavated from within the proposed building envelope and exterior sidewalks or concrete slabs to expose approved competent subgrade soils. It would also be sound construction practice to subexcavate all existing unsuitable fill from paved driveway areas; however, this may not be cost-effective. At a minimum, thorough inspection will be required at the time of construction to assess the existing fill to ensure there is no buried topsoil or other deleterious materials within the subgrade. Remedial action will also be required to further consolidate the existing fill if it is decided to leave it in place. If the existing fill is left in place, provisions for the alterations to the design of the pavement structure should be included in the tender documents.

Review of the subgrade and potential changes to the design of the pavement structure, as required, will be addressed at the time of construction.

Prior to reusing excavated material on-site as potential bulk fill in driveways and parking lots, thorough field inspection and approval from qualified geotechnical personnel would be required to ensure that existing fill materials do not comprise organics, topsoil or other deleterious materials.

#### **5.4.3. Removal/Relocation of Existing Services**

Any existing buried pipes that are no longer deemed necessary should be removed. This includes irrigation pipes, and any existing field tiles or existing subdrains that may be present. Any existing tile drains that may be located within the proposed building envelope, hard-surfaced areas and septic system areas must be completely removed to a minimum distance of 15.0 m (50.0 ft) outside of the building envelope. Ideally, depending on flow direction, any existing tile drains (if present) should be redirected outside of the building envelope, hard-surfaced areas and septic system areas in order to maintain flow and prevent subsurface accumulation of water. The locations of existing field tiles are commonly identified by relatively straight lines of buried topsoil within the subgrade soils and/or water boiling out of the ground following excavation. Any piping that is left in place that is no longer active must be completely sealed with watertight mechanical covers, concrete or grout at termination points to prevent the migration of soils into pipe voids, which may result in potential settlement. All existing trench backfill material associated with any existing buried pipes must be subexcavated and the subsequent excavation must be backfilled with approved soils placed in accordance with Section 5.4.4 of this report.

The monitoring wells that have been installed to determine static water levels can be decommissioned by an MOE licensed well contractor with a Class 1, Class 2 or Class 3 license in accordance with Reg. 903. CMT Drilling Inc. would be pleased to provide these services when the monitoring wells are no longer required.

#### **5.4.4. Site Grading**

Following stripping of the topsoil, the removal of all trees roots (including all relatively loose soils associated with the tree roots) as well as the subexcavation of any fill and relatively soft native soils deemed unsuitable of supporting the design bearing capacity, the exposed subgrade must be proof-rolled, and any soft or unstable areas (including sand trap excavations) must be further subexcavated and replaced with approved fill materials.

The proposed finished floor elevations were not known at the time of preparation of this report; however, it is expected that some structural fill placement may be required. The fill materials required to achieve the design site grades should be placed according to the following procedures:

- Prior to placement of any structural fill or bulk fill, the subgrade for the proposed buildings and paved areas must be prepared large enough to accommodate a 1:1 slope commencing a distance of 1.0 m beyond the outside edge of the proposed foundation and pavement/concrete edge down to the approved competent founding soils;
- Soils approved for use as structural fill must be placed in loose lifts not exceeding 0.3 m (12") in depth for granular soils (recommended fill material) and 0.2 m (8") in depth for silts and clays (not recommended for this application), or the capacity of the compactor (whichever is less);
- Native granular soils as well as imported granular fill materials (OPSS 1010 Type I or Type III Granular 'B') can be compacted utilizing adequate heavy vibratory smooth drum or padfoot compaction equipment (compaction with padfoot equipment requires that structural fill be placed a minimum 0.3 m above the design footing grade in order to account for the disturbance of the founding soils from the pads on the compaction equipment);
- Approved fill materials must be at suitable moisture contents to achieve the specified compaction. The wet to saturated soils encountered in the boreholes would generally be considered difficult for use as structural fill as they would require extensive air-drying in order to achieve the specified density. Soil moisture will also be dependent on weather conditions at the time of construction;
- Approved structural fill materials that will support structures (including house foundations, interior slab-on-grades, sidewalks, large expansive exterior slabs, decks and exterior stairs) must be compacted to a minimum of 100% standard Proctor maximum dry density (SPMDD);
- Approved bulk fill (exterior foundation wall backfill in landscaped areas, bulk fill for paved areas) must be compacted to a minimum 95% SPMDD;
- Granular 'B' subbase and Granular 'A' base materials for paved areas must be compacted to 100% SPMDD;
- It is recommended that compactive effort be applied to bulk fill in landscaped areas in order to reduce the effects of long term settlement.

Based on the subsurface conditions observed in the boreholes, wet soils may be encountered, depending on the depth of excavation. As such, for soils excavated from the zone of saturation, significant air-drying along with working of the soils may be required in order to achieve the specified compaction of 100% SPMDD for structural fill and 95% SPMDD for bulk fill for roads and driveways. Utilizing the existing soils during site grading may be more achievable if work is completed during the generally drier summer months. Reuse of excavated soils on-site will be subject to approval from qualified geotechnical personnel.

### **5.5. Foundation Subgrade Preparation**

The native soils encountered in the boreholes are sensitive to change in moisture content and can become loose/soft if the soils are subjected to additional water or precipitation, as well as severe drying conditions. The native subgrade soils could also be easily disturbed if traveled on during construction. Once they become disturbed they are no longer considered adequate for the support of shallow foundations. To ensure and protect the integrity of the founding soils during construction operations, the following is recommended:

- During construction, the subgrade should be sloped or ditched to a sump (as required) located outside the building footprint (if feasible) in the excavation to promote surface drainage of rainwater or seepage and the collected water should be pumped out of the excavation (the environmental consultant must be consulted prior to any on-site water being pumped and/or discharged to municipal outlets to ensure that proper procedures are followed). It is critical that all water be controlled (not allowed to pond) and that the subgrade and foundation preparation commence in dry conditions;
- Should the native subgrade soils at the design founding elevation in the proposed building envelope(s) comprise saturated soils, as was observed in some of the boreholes, then a granular drainage layer, constructed in accordance with Section 9.14.4 of the current Ontario Building Code (OBC) may be required;
- Construction equipment travel and foot traffic on the founding soils should be minimized;
- If construction is to be undertaken during subzero weather conditions, the founding native soils and any potential fill materials must be maintained above freezing;
- Prior to pouring concrete for the footings, the footing area must be cleaned of all disturbed or caved materials;



- The foundation formwork and concrete should be installed as soon as practical following the excavation, inspection and approval of the founding soils. The longer that the excavated soils remain open to weather conditions and groundwater seepage, the greater the potential for construction problems to occur;
- If it is expected that the founding soils will be left open to exposure for an extended period of time, it is recommended that a 75 mm concrete mud slab be poured in order to protect the structural integrity of the founding soils.

#### 5.6. Slab-on-Grade/Modulus of Subgrade Reaction

Prior to the placement of the granular base for the slab-on-grade construction, the subgrade soils should be proof-rolled. Any soft or weak zones, as well as any potential unsuitable fill in the subgrade (field tile trenches), should be subexcavated and backfilled with approved fill materials (see Section 5.4.4 of this report).

The following table provides the modulus of subgrade reaction (k) for imported granular fill, as well as the native soils encountered on-site:

Soil Type	Modulus of Subgrade Reaction (k)
Imported Sand and Gravel (OPSS 1010)	81,000 kN/m <sup>3</sup> (300 lb/in <sup>3</sup> )
Silty Gravelly Sand	20,000 kN/m <sup>3</sup> (75 lb/in <sup>3</sup> )
Gravel and Sand	81,000 kN/m <sup>3</sup> (300 lb/in <sup>3</sup> )
Sand	68,000 kN/m <sup>3</sup> (250 lb/in <sup>3</sup> )
Sandy Gravel	81,000 kN/m <sup>3</sup> (300 lb/in <sup>3</sup> )
Sand and Silt	61,000 kN/m <sup>3</sup> (225 lb/in <sup>3</sup> )

Floor slabs can be founded on a minimum thickness of 100 mm (4") of coarse, clean granular material containing not more than 10% of material that will pass a 4 mm sieve in accordance with the current OBC. The clean granular material should be consolidated to prevent future settlement.

It is recommended that areas of extensive exterior slab-on-grade (sidewalks and accessibility ramps) be constructed with a Granular 'B' subbase (450 mm) and a Granular 'A' base (150 mm), as well as incorporating subdrains, to promote rapid drainage and reduce the effects of frost heaving. This is particularly critical at barrier-free access points. Alternatively, structural frost slabs could be designed and constructed, or sufficient thermal insulation could be provided, at all door entrances and areas of barrier-free access.

### 5.7. Excavations

All excavations must be carried out in accordance with Ontario Regulation 213/91 (Reg 213/91) of the Occupational Health and Safety Act and Regulations for Construction Projects.

**Type 3 Soils** - In general, the existing native soils as well as the fill materials encountered in the boreholes in a drained state (not wet or saturated), would be classified as Type 3 soils under Reg 213/91. The Type 3 soils must be sloped from the bottom of the excavation at a minimum gradient of 1 horizontal to 1 vertical. All saturated soils encountered must be treated as Type 4 soils, as described below.

**Type 4 Soils** - In general, any wet to saturated soils would be classified as Type 4 soils under Reg 213/91. Type 4 soils must be sloped from the bottom of the excavation at a minimum gradient of 3 horizontal to 1 vertical.

If it is not practical to excavate according to the above requirements, then a trench support system (designed in accordance with the Ontario Health and Safety Act Regulations) may be utilized.

It should be noted that the native sandy gravel, as well as the gravel and sand soils encountered in the boreholes, contained large cobbles and became very dense in places (SPT N-values in excess of 50 blows per 0.30 m) and may prove difficult to excavate with conventional excavating equipment, impacting the production schedule. It is imperative that if the very dense soils are utilized as fill, the material must be broken down (pulverized) to minimize void space and reduce the potential for settlement.

### 5.8. Construction Dewatering Considerations

Wet to saturated soils were encountered in Boreholes 1, 3, 5 and 6 as described in Section 4.8 of this report. It should be noted that the relatively fine-grained and/or dense soils may have the potential to create perched water conditions. Groundwater conditions (particularly perched water) are generally dependent on the amount of precipitation, control of surface water, as well as the time of year, and can fluctuate significantly in elevation and volume. As such, provisions for site dewatering should be part of the site development and construction process.

Seepage control requirements during construction will depend upon the area of work on the site, the depth of the excavations, the time of year, the amount of precipitation and the control of surface water. As such, it is critical that provisions for site dewatering be part of the site development and construction process. As required, seepage should generally be adequately controlled using conventional construction dewatering techniques such as pumping from sump pits. However, if heavy seepage occurs, it may be necessary to increase the number of pumps during construction.

Dewatering should be performed in accordance with OPSS 517 and the control of water must be in accordance with OPSS 518. It is the responsibility of the contractor to propose a suitable dewatering system based on the groundwater elevation at the time of construction. Collected water should discharge a sufficient distance away from the excavation to prevent re-entry. Sediment control measures must be installed at the discharge point of the dewatering system to avoid any potential adverse impacts on the environment.

### **5.9. Service Pipe Bedding**

The native soils encountered in the geotechnical investigation are generally considered suitable for indirect support of the site service pipes. Should instability due to saturated soil conditions be encountered, it may be necessary to increase the thickness of the granular base and utilize 19 mm clear stone to create an adequate supporting base for the service pipes and/or manholes. Pipe embedment, cover and backfill for both flexible and rigid pipes should be in accordance with all current and applicable OPSD, OPSS and OBC standards and guidelines and as follows:

**Flexible Pipes** – The pipe bedding should be shaped to receive the bottom of the pipe. If necessary, pipe culvert frost treatment should be undertaken in accordance with OPSD-803.031. The trench excavations should be symmetrical with respect to the centre-line of the pipe. The granular material placed under the haunches of the pipe must be compacted to 95% SPMDD prior to the continued placement and compaction of the embedment material. The homogeneous granular material used for embedment should be placed and compacted uniformly around the pipe. Should wet conditions be encountered at the base of the trench, then the pipe bedding should consist of 19 mm clear stone (meeting OPS Specifications) wrapped completely in a geotextile fabric such as Terrafix 270 or equivalent. The general contractor is responsible to protect service piping from damage by heavy equipment.

**Rigid Pipes** - In general, the pipe installation recommendations for rigid pipes are the same as those for flexible pipes, except that the minimum bedding depth below a rigid pipe should be  $0.15D$  (where  $D$  is the pipe diameter). In no case should this dimension be less than 150 mm or greater than 300 mm.

### **5.10. Perimeter Building Drainage, Foundation Wall Backfill and Trench Backfill**

In order to assist in maintaining dry buildings with respect to surface water seepage, it is recommended that exterior grades around the buildings be sloped down and away at a 2% gradient or more, for a distance of at least 1.5 m. Any surface discharge rainwater leaders must be constructed with solid piping that discharges with positive drainage at least 1.5 m away from the building foundations and/or beyond sidewalks to a drainage swale or appropriate storm drainage system.

The founding elevations for the proposed structures were not available at the time of preparation of this report. CMT Inc. can provide further recommendations for building drainage once the design drawings are completed and the founding elevations have been confirmed.

It should be noted that based on the observations in the boreholes, there is potential for perched water conditions. The construction of foundations, slabs-on-grade, and deep structures such as sump pits within or below zones of saturation will require design of site-specific waterproofing systems constructed in accordance with the 2012 OBC. If required, it would be recommended that a waterproofing supplier/specialist be consulted to recommend an appropriate product and installation requirements that would be suited to this site. It is recommended that a good quality sump pump be utilized, and that the system be equipped with a battery back-up in the event of power failure, (keeping in mind that a battery back-up system does not typically have a long run time).

If it is expected that the new residences will have basements, exterior perimeter weeping tile systems comprising perforated drainage pipe with a factory installed filter sock, bedded in 19 mm clear crushed stone and wrapped in geotextile filter fabric such as Terrafix 270R (or equivalent), must be installed at an elevation that is below the proposed basement slab-on-grade elevation and provided with positive drainage into a sump pit. The portion of the piping that connects the exterior weeping tile system into the sump pit must comprise solid piping to prevent exterior water from being introduced into the interior subslab stone. It may be prudent to install perforated drainage pipe in the interior basement as well to provide an outlet for any water that may collect in the subslab stone. It is also recommended that a capped cleanout port(s) be extended up to the ground surface elevation to provide future access (if required). The rainwater leaders must not be connected to the perimeter weeping tile system.

In order to reduce the effects of surficial frost heave in areas that will be hard surfaced, it is recommended that the exterior foundation backfill consist of free-draining granular material such as approved on-site sand and gravel or imported Granular 'B' Type I or Type III (OPSS 1010), with a maximum aggregate size not exceeding 100 mm, and that it extend a minimum lateral distance of 600 mm out from the foundation walls and/or beyond perimeter sidewalks and entranceway slabs. It is critical that particles greater than 100 mm in diameter are not in contact with the foundation wall to prevent point loading and overstressing. The backfill material used against the foundation walls must be placed so that the allowable lateral capacities of the foundation walls are not exceeded. Where only one side of a foundation wall will be backfilled, and the height of the wall is such that lateral support is required, or where the concrete strength has not been achieved, the wall must be braced or laterally supported prior to backfilling. In situations where both sides of the wall are backfilled, the backfill should be placed in equal lifts, not exceeding 200 mm differential on each side during backfill operations and the backfill should be compacted to a minimum of 100% SPMDD.

The native mineral soils, as well as approved fill materials (non-organic) are generally considered suitable for reuse as trench backfill and bulk fill in the driveways and parking lots; however, the wet soils may require air-drying in order to achieve the specified compaction. Air-drying cannot typically be achieved during winter construction; therefore, depending on the time of year that construction takes place, it may be more feasible to utilize an imported granular fill for this project.

Backfilling operations should be carried out with the following minimum requirements:

- Adequate heavy smooth drum or padfoot vibratory compaction equipment (suited to soil type) should be used for the compaction and to break down any large blocky pieces of soil;
- Loose lift thicknesses should not exceed 0.3 m (12") for granular soils or 0.2 m (8") for silt soils or the capacity of the compactor (whichever is less);
- The soils must be at suitable moisture contents to achieve compaction to a minimum 95% SPMDD in non-structural bulk fill areas. Service trenches excavated within the zone of influence of footings for structures must be compacted to a minimum of 100% SPMDD;
- It is recommended that inspection and testing be carried out during construction to confirm backfill quality, thickness and to ensure that compaction requirements are achieved;
- Service trench backfill materials may consist of approved excavated soils with no particles greater than 100 mm and no topsoil or other deleterious materials;
- If construction operations are undertaken in the winter, strict consideration should be given to the condition of the backfill material to make certain that frozen material is not used.

#### **5.11. Pavement Design/Drainage**

All topsoil, vegetation (including tree roots and all loose/disturbed soils associated with tree roots) must be subexcavated from within driveway and road areas. It is recommended to either subexcavate any existing soft subgrade materials or provide further consolidation with vibratory compaction equipment in order to prepare a proper, stable subgrade. Prior to placement of the granular base, the subgrade must be proof-rolled, and any soft or unstable areas should be subexcavated and replaced with suitable drier materials. The subgrade should be graded smooth (free of depressions) and properly crowned to ensure positive drainage, with a minimum grade of 3% toward the drainage outlet or curbline/edge of asphalt (provided that collection and proper gravity drainage to a suitable outlet is provided). When service pipes are installed, pipe bedding and backfilling should be undertaken as indicated in Sections 5.9 and 5.10 of this report.

Rapid drainage of the pavement structure is critical to ensure long-term performance. The requirement for subdrains will be dependent on the composition of the prepared pavement subgrade soils. If the subgrade soils comprise fine-grained, frost-susceptible soils, then it is recommended to install minimum 100 mm diameter perforated subdrains to collect and redirect water beneath the pavement surface (provided positive drainage to a suitable outlet can be provided). Subdrains should be designed and installed in accordance with OPSS 405 and OPSD 216.021. If Granular 'A' bedding (OPSS 1010) is utilized, the subdrains should be equipped with a factory installed filter sock. If 19 mm clear stone (OPSS 1004) is utilized as bedding for the subdrain, then the bedding must be wrapped completely with geotextile filter fabric such as Terrafix 270R (or equivalent) and a factory installed filter sock is not required. Installation of rigid subdrains allows for better grade control and less potential for damage during installation; however, it would be expected that there would be higher cost implications associated with the installation of rigid subdrains over flexible subdrains. Positive drainage through grade control of subdrains is critical, as improperly installed subdrains can turn drainage systems into reservoirs, which can fuel frost heave. The subdrains will hasten the removal of water, thereby reducing the risk and effects of frost heaving and load transfer in saturated conditions. It is suggested that subdrains be along the curb lines. It is also recommended to install subdrains through any areas that cannot tolerate differential frost heave such as accessibility ramps/sidewalks. The subdrains should be installed in a 0.3 m (1.0 ft) by 0.3 m (1.0 ft) trench in the subgrade and bedded approximately 50 mm (2") above the bottom of the trench. The subgrade must be prepared with positive drainage to the subdrains and the subdrains must be installed with positive drainage into a catch basin structure or other suitable outlet.

Should the subgrade soils comprise free-draining granular soils (minimum 1.0 m thick with positive drainage at the interface with any relatively impermeable soils), then the installation of subdrains may not be required.

The subgrade soils could be easily disturbed if traveled on during construction. As such, where this material will be exposed, it is recommended that the granular subbase be placed immediately upon completion of the subgrade preparation to protect the integrity of the subgrade soils.

Should wet conditions be encountered during construction, site assessments may be required to determine what options can be undertaken to construct a modified pavement base. These options may include subexcavation of wet soils and increasing the thickness of the granular base, the use of reinforcing geotextiles, or a combination of both.

It is expected that driveways and parking lots will experience mostly light residential traffic (personal vehicles) and some heavy traffic (delivery trucks, as well as maintenance and emergency vehicles). Based on the anticipated vehicle loading, the following pavement design is provided:

<b>Materials</b>	<b>Recommended Thickness for New Pavement</b>
Asphaltic Concrete	HL3 40 mm (1.5") HL4 or HL8 - 50 mm (2.0")
Granular 'A' Base (OPSS 1010)	150 mm (6.0")
Granular 'B' Subbase (OPSS 1010)	450 mm (18.0")

Given the potential for wet subgrade conditions, site assessments may be required at the time of construction to determine what options can be undertaken to construct a stable road and driveway base. These options may include subexcavation and increasing the thickness of the Granular 'B' subbase, the use of reinforcing geotextile and/or geogrid, or a combination of all. As such, it is recommended that provisions for subexcavation and disposal of wet soils, importing and placing additional Granular 'B' (OPSS 1010), as well as supply and placement of a reinforcing geotextile (Terrafix 200W or equivalent) and geogrid (Tensar TX160 or equivalent) should be included in the tender documents.

Construction joints in the surface asphalt must be offset a minimum of 150 mm to 300 mm (6" to 12") from construction joints in the binder asphalt so that longitudinal joints do not coincide.

Where new asphalt is joined into existing asphalt, it is recommended that the existing asphalt be sawcut in a straight line prior to being milled to a depth of 40 mm and a width of 150 mm as per OPSD 509.010. It is recommended that a tackcoat in conformance with OPSS 308 be applied to the edge and surface of all milled asphalt prior to placement of new asphalt.

The granular base and subbase materials must conform to the physical property and gradation requirements of OPSS 1010 and must be compacted to 100% SPMDD. Asphaltic concrete should be supplied, placed and compacted to a minimum 92.0% Marshall maximum relative density, in accordance with OPSS 1150 and OPSS 310.

The pavement should be designed to ensure that water will not pond on the pavement surface. If the surface asphalt is not placed within a reasonable time following placement of the binder asphalt, it is recommended that the catch basin lids are set at a lower elevation or apertures provided to allow surface water to drain into the catch basins and not accumulate around the catch basins. The strength of the pavement structure relies on all of the components to be in place in order to provide the design strength; therefore, it is



strongly recommended that the surface asphalt be placed shortly after placement of the binder asphalt so as to avoid undue stress on the binder asphalt by not having the complete pavement structure in place.

It should be noted that currently, asphalt mixes tend to be more flexible, and as such, there is a tendency for damage to occur from vehicles turning their steering wheels or applying excessive brake pressure. The damage can occur from both passenger vehicles as well as delivery trucks and the condition is further intensified during hot weather.

## **5.12. Excess Soil Management**

### **5.12.1. Chemical Testing was NOT Undertaken**

Generally, if surplus soils are to be exported off-site, it will be necessary to perform chemical analysis of the soils. Chemical analysis was not undertaken as part of this geotechnical investigation. Should chemical analysis tests be required, the required tests vary and will be dependent on the disposal site utilized by the general contractor.

Most commonly, the soils are tested for the following:

- F1-F4, VOC's, BTEX as per O. Reg. 153/04 as amended by R511
- SVOC as per O. Reg. 153/04 as amended by R511
- Metals/Inorganics as per O. Reg. 153/04 amended by R511

The chemical analysis results are then compared to Ontario Regulation 153/04 - as amended by O.Reg. 511 – April 15, 2011 Standards = [Suite] – ON-511-T1/T2-SOIL-RPI. Specifically, the results were compared in *T1-Soil-Res/Park/Inst/Ind/Com/Commu Property Use*, *T2-Soil-Res/Park/Inst. Property Use (Coarse)* and *T3-Soil-Res/Park/Inst. Property Use (Fine)*.

### **5.12.2. TCLP Requirement**

If soils are transported to a land fill facility, additional chemical testing in accordance with Ontario Regulation 347, Schedule 4, as amended to Ontario Regulation 558/00, dated March 2001, Toxicity Characteristic Leaching Procedure (TCLP) will be required.

When transporting soils off-site, the following is recommended:

- All chemical analyses and environmental assessment reports must be fully disclosed to the receiving site owners/authorities, whom must agree to receive the material;
- An environmental consultant must confirm the land use at the receiving site is compatible to receive the material;
- An environmental consultant must monitor the transportation and placement of the materials to ensure that the material is placed appropriately at the pre-approved site;
- The excess materials may not be transported to a site that has previously had a Record of Site Condition (RSC) filed, unless the material meets the criteria outlined in the RSC.

It should be noted that landfill sites will generally only accept laboratory test results that have been completed within 30 days of exporting. Therefore, it is recommended that provisions for chemical analysis be included in the tender documents. It should also be noted that the laboratory testing generally takes five (5) working days to process with a regular turnaround time.

#### **5.13. Infiltration and Coefficient of Permeability/T-Time**

As part of the geotechnical investigation, gradation analyses were performed on samples of the native sand, gravel/sandy gravel, silty gravelly sand, as well as the gravel and sand soils. The following table provides the borehole number, sample depth, estimated coefficient of permeability (k) and percolation rate (T-time), as well as soil type:

<b>Borehole No.</b>	<b>Depth (m)</b>	<b>Estimated Coefficient of Permeability (k) cm/s</b>	<b>Estimated Percolation Rate (T) min/cm</b>	<b>Soil Type</b>
1	1.52 – 2.13	$3.2 \times 10^{-3}$	10	Sand, some silt, trace clay and gravel (SM)
2	1.22 – 2.13	$8.4 \times 10^{-4}$	12	Sandy gravel, some silt, trace clay (GM)
3	0.76 – 1.37	$3.9 \times 10^{-3}$	8	Gravel, some sand and silt, trace clay (GM)
4	0.76 – 1.37	$4.8 \times 10^{-4}$	15	Silty, gravelly sand, trace clay (SM)
5	0.76 – 1.37	$5.3 \times 10^{-3}$	8	Sandy gravel, trace silt and clay (GW-GM)
6	0.76 – 1.37	$8.0 \times 10^{-3}$	8	Gravel and sand, trace silt and clay (GP-GM)

#### **5.14. Radon**

According to information provided by Health Canada, radon is a radioactive gas that is naturally formed through the breakdown of uranium in soil, rock and water. When radon escapes the earth in the outdoors, it mixes with fresh air, resulting in concentrations that are too low to be of concern. However, when radon enters an enclosed space, such as a building, high concentration of radon can accumulate and become a health concern. Health Canada indicates that most homes have some level of radon in them. Unfortunately, it is not possible to predict before construction whether or not a new home will have high radon levels as radon can only be detected by radon measurement devices, which would be installed in a home, post construction. Section 9.13.4.1 Soil Gas Control of the current 2012 Ontario Building Code (OBC) states that "*Where methane or radon gases are known to be a problem, construction shall comply with the requirements for soil gas control in MMAH Supplementary Standard SB-9, Requirements for Soil Gas Control*".

### **6.0 SITE INSPECTIONS**

Qualified geotechnical personnel should supervise excavation inspections as well as compaction testing for structural filling, site grading and site servicing. This will ensure that footings are founded in the proper strata and that proper material and techniques are used and the specified compaction is achieved. CMT Engineering Inc. would be pleased to review the design drawings and provide an inspection and testing program for the construction of the proposed development.

### **7.0 LIMITATIONS OF THE INVESTIGATION**

This report is intended for the Client named herein and for their Client. The report should be read in its entirety, and no portion of this report may be used as a separate entity. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties.

The recommendations made in this report are in accordance with our present understanding of the project. We request that we be permitted to review our recommendations when the drawings and specifications are complete, or if the proposed construction should differ from that mentioned in this report.

It is important to emphasize that a soil investigation is, in fact, a random sampling of a site and the comments are based on the results obtained at the test locations only. It is therefore assumed that these results are representative of the subsoil conditions across the site. Should any conditions at the site be encountered which differ from those found at the test locations, we request that we be notified immediately in order to permit a reassessment of our recommendations.

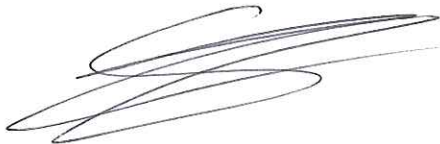
It should be noted that this report specifically addresses geotechnical aspects of the project and does not include any investigations or assessments relating to potential subsurface contamination. As such, there should be no assumptions or conclusions derived from this report with respect to potential soil or water contamination. Soil or water contamination is generally caused by the presence of xenobiotic (human-made) chemicals or other alteration processes in the natural soil and groundwater environment. If necessary, the investigation, assessment and rehabilitation of soil and water contaminants should be undertaken by qualified environmental specialists.

The samples obtained during the geotechnical investigation will be stored for a period of three months, after which time they will be disposed of unless alternative arrangements are made.

We trust that this report meets with your present requirements. Should you have any questions, please do not hesitate to contact our office.

Prepared by:

Reviewed by:



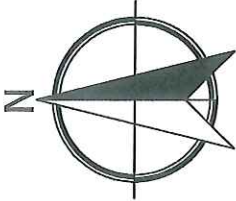
Shawn Wheatley, M.Eng.




Robert Koopmans, P.Eng.  
Consulting Engineer

ks

NOTES:  
Base map provided by Google Maps



REVISIONS		
NO.	DESCRIPTION	DATE

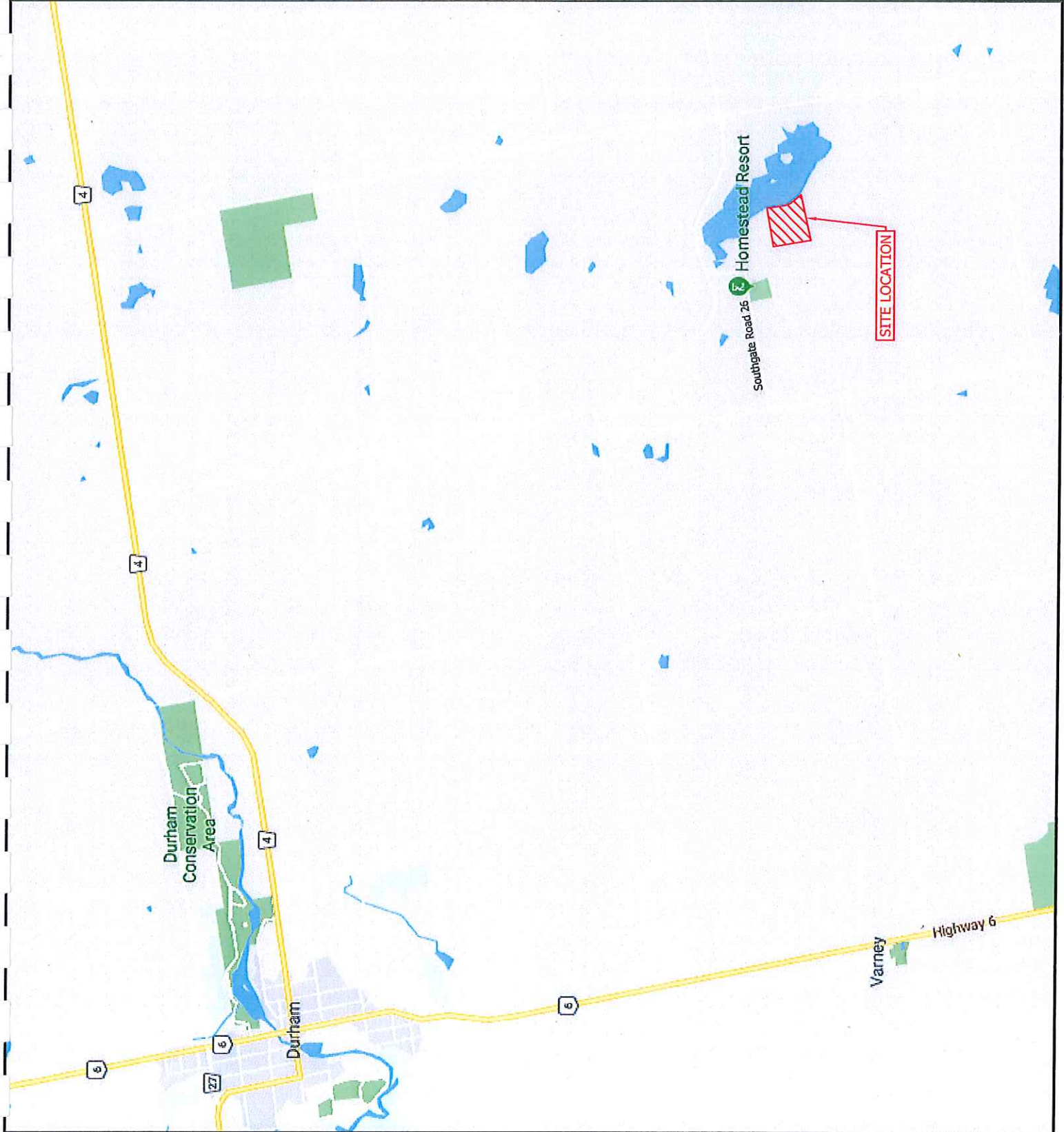


**CMT ENGINEERING INC.**  
 1011 Industrial Crescent, Unit 1  
 St. Clements, Ontario N0B 2M0  
 Tel: 519-699-5775  
 Fax: 519-699-4664  
[www.cmtinc.net](http://www.cmtinc.net)

**PROJECT:**  
 Wilder Lake Subdivision  
 263530 Southgate Road 26,  
 Township of Southgate, Ontario

**DRAWING TITLE:**  
 SITE LOCATION MAP

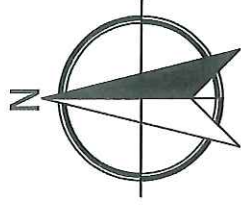
PROJECT NO.:	DATE:
18-368	December 4, 2018
SCALE:	DRAWING NO.
N.T.S.	1





NOTES:

1. This drawing is for information purposes only. Locations and sizes of existing and proposed structures are approximate only, and should not be used for construction.
2. Base drawing is provided by Grey County GIS Viewer



NO.	DESCRIPTION	DATE

REVISIONS

**CMT ENGINEERING INC.**  
 1011 Industrial Crescent, Unit 1  
 St. Catharines, Ontario N0B 2M0  
 Tel: 905-689-5775  
 Fax: 905-689-4664  
 www.cmtinc.net

PROJECT:

Wilder Lake Subdivision  
 263530 Southgate Road 26,  
 Township of Southgate, Ontario

DRAWING TITLE:

SITE PLAN SHOWING  
 BOREHOLE LOCATIONS

PROJECT NO.:	DATE:
18-368	December 4, 2018
SCALE:	DRAWING NO.
N.T.S.	2





**APPENDIX A**  
**BOREHOLE LOGS**



# BOREHOLE 1

Page 1 of 1

Date Drilled: July 27, 2018  
Rig: Geoprobe 7822DT  
Contractor: CMT Drilling Inc.  
Drilling Method: SPT

Elevation: 425.64 m  
Logged by: TS

Project No.: 18-368  
Project: Wilder Lake Subdivision  
Location: 263530 Southgate Road 26,  
Southgate, Ontario

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wp 10 20 30 40	Pocket Penetrometer kPa 100 200 300 400	SPT (N) Blows/0.3 m 20 40 60 80
ft m									
0					Ground Surface (m) 425.64				
1	SS		1		<b>TOPSOIL</b> Very loose, dark brown, silty organic topsoil, moist (230mm)				
2					424.88				
3	SS		2		<b>SILTY GRAVELLY SAND</b> Loose, brown silty gravelly sand, trace clay, moist				
4					424.12				
5					<b>GRAVEL AND SAND</b> Dense, brown gravel and sand, trace silt and clay, damp				
6	SS		3		<b>SAND</b> Compact, brown medium/fine sand, some silt, trace clay, moist				
7					423.05				
8	SS		4		Becoming coarse sand, damp				
9					2.59				
10					<b>GRAVEL AND SAND</b> Very dense, brown gravel and sand, trace silt and clay, damp				
11	SS		5		Occasional thin silt layers				
12									
13	MC5		6						
14									
15									
16	SS		7		Becoming saturated				
17					420.76				
18					4.88				
19	MC5		8		<b>SAND AND SILT</b> Compact, brown sand and silt, trace gravel and clay, saturated, rapidly dilating				
20					420.15				
21					5.49				
22					419.54				
					6.10				
					End of Borehole				

CMT ENGINEERING INC.  
1011 Industrial Crescent, Unit 1  
St. Clements, Ontario N0B 2M0  
phone 519-699-5775 fax 519-699-4664  
www.cmtinc.net





# BOREHOLE 2

Page 1 of 1

Date Drilled: July 27, 2018  
Rig: Geoprobe 7822DT  
Contractor: CMT Drilling Inc.  
Drilling Method: SPT

Elevation: 430.64 m  
Logged by: TS

Project No.: 18-368  
Project: Wilder Lake Subdivision  
Location: 263530 Southgate Road 26,  
Southgate, Ontario

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa 100 200 300 400
							10 20 30 40	SPT (N) Blows/0.3 m 20 40 60 80
0					Ground Surface (m) 430.64			
0					0.00			
1	SS		1		<b>TOPSOIL</b> Loose, dark brown, silty organic topsoil, moist (250mm)		8.3	7
2								
3	SS		2		<b>FILL</b> Loose, dark brown silt fill, some topsoil, moist		6.4	19
4					429.42			
5					1.22			
6	SS		3		<b>SANDY GRAVEL</b> Compact, brown sandy gravel, some silt, trace clay, occasional cobbles, moist		9.5	26
7					428.35			
8	SS		4		Becoming dense		8.0	33
9					2.29			
10	SS		5				8.6	50 (2")
11					426.68			
12	MC5		6		<b>SAND AND SILT</b> Compact, brown, sand and silt, some gravel, trace clay, occasional cobbles, wet		10.4	
13					3.96			
14	SS		7				10.2	26
15								
16	MC5		8				10.9	
17					424.54			
18	SS		9		Becoming dense			31
19					6.10			
20	MC5		10					
21					423.32			
22					7.32			
23					<b>GRAVEL AND SAND</b> Dense, brown gravel and sand, trace silt and clay, damp		2.8	
24					432.02			
25					7.62			
26								
27					End of Borehole			

CMT ENGINEERING INC.  
1011 Industrial Crescent, Unit 1  
St. Clements, Ontario NOB 2M0  
phone 519-699-5775 fax 519-699-4664  
www.cmtinc.net



# BOREHOLE 3

Page 1 of 1

Date Drilled: July 27, 2018  
Rig: Geoprobe 7822DT  
Contractor: CMT Drilling Inc.  
Drilling Method: SPT

Elevation: 425.90 m  
Logged by: TS

Project No.: 18-368  
Project: Wilder Lake Subdivision  
Location: 263530 Southgate Road 26,  
Southgate, Ontario

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl 10 20 30 40	Pocket Penetrometer kPa 100 200 300 400 SPT (N) Blows/0.3 m 20 40 60 80
0					Ground Surface (m) 425.90 0.00			
1	SS		1		<b>TOPSOIL</b> Loose, dark brown, silty organic topsoil, moist (150mm)			
2					425.14 0.76			
3	SS		2		<b>SILTY SAND AND GRAVEL</b> Compact, brown silty sand and gravel, trace clay, moist			
4								
5					<b>GRAVEL</b> Compact, brown gravel, some sand and silt, trace clay, occasional cobbles, moist			
6	SS		3					
7					423.61 2.29			
8	SS		4		Becoming very moist to wet, occasional sand layer			
9								
10								
11	SS		5					
12								
13	MC5		6					
14					421.33 4.57			
15					Occasional clay layer, wet			
16	SS		7					
17					420.72 5.18			
18					Becoming saturated			
19	MC5		8					
20					419.80 6.10			
21					End of Borehole			
22								

CMT ENGINEERING INC.  
1011 Industrial Crescent, Unit 1  
St. Clements, Ontario N0B 2M0  
phone 519-699-5775 fax 519-699-4664  
www.cmtinc.net





# BOREHOLE 4

Page 1 of 1

Date Drilled: July 27, 2018  
Rig: Geoprobe 7822DT  
Contractor: CMT Drilling Inc.  
Drilling Method: SPT

Elevation: 425.54 m  
Logged by: TS

Project No.: 18-368  
Project: Wilder Lake Subdivision  
Location: 263530 Southgate Road 26,  
Southgate, Ontario

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [----X----] Wl 10 20 30 40	Pocket Penetrometer kPa 100 200 300 400	SPT (N) Blows/0.3 m 20 40 60 80
0					Ground Surface (m) 425.54				
0					0.00				
1	SS		1		<b>TOPSOIL</b> Loose, dark brown, silty organic topsoil, moist (200mm)		8.2	9	
2									
3	SS		2		<b>SILTY GRAVELLY SAND</b> Compact, brown silty gravelly sand, trace clay, moist Occasional sand layers		6.8	13	
4									
5					424.02				
6	SS		3		<b>SANDY GRAVEL</b> Compact, brown sandy gravel, some silt, trace clay, occasional cobbles, moist		8.7	15	
7									
8	SS		4				6.5	24	
9									
10					422.49				
11	SS		5		Becoming dense		8.3		50 (2")
12									
13									
14	MC5		6				4.0		
15					421.27				
16					4.27				
17	SS		7		<b>GRAVEL AND SAND</b> Dense, brown gravel and sand, trace silt and clay, damp		8.3	47	
18									
19					420.05				
20	MC5		8		<b>SANDY GRAVEL</b> Dense, brown sandy gravel, some silt, trace clay, occasional cobbles, moist		12.4		
21									
22					419.44				
23					6.10				
24					Becoming very moist				
25	MC5		9				10.4		
					417.92				
					7.62				
					End of Borehole				

CMT ENGINEERING INC.  
1011 Industrial Crescent, Unit 1  
St. Clements, Ontario NOB 2M0  
phone 519-699-5775 fax 519-699-4664  
www.cmtinc.net



# BOREHOLE 5

Page 1 of 1

Date Drilled: July 27, 2018  
Rig: Geoprobe 7822DT  
Contractor: CMT Drilling Inc.  
Drilling Method: SPT

Elevation: 425.26 m  
Logged by: TS

Project No.: 18-368  
Project: Wilder Lake Subdivision  
Location: 263530 Southgate Road 26,  
Southgate, Ontario

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa 100 200 300 400
							10 20 30 40	SPT (N) Blows/0.3 m 20 40 60 80
0					Ground Surface (m) 425.26			
0					0.00			
1	SS		1		<b>TOPSOIL</b> Loose, dark brown, silty organic topsoil, moist (150mm)		4.7	11
2					<b>SANDY GRAVEL</b> Dense, brown sandy gravel, trace silt and clay, occasional cobbles, damp		3.2	41
3	SS		2				2.1	37
4							3.2	24
5	SS		3					
6								
7	SS		4					
8								
9	SS		5		Becoming compact			
10								
11	SS		6		Becoming saturated			
12								
13	SS		7					
14								
15	MC5		8					
16								
17	SS		9		Becoming grey			
18								
19	MC5		10					
20								
21								
22								

CMT ENGINEERING INC.  
1011 Industrial Crescent, Unit 1  
St. Clements, Ontario N0B 2M0  
phone 519-699-5775 fax 519-699-4664  
www.cmtinc.net





# BOREHOLE 6

Page 1 of 1

Date Drilled: July 27, 2018  
Rig: Geoprobe 7822DT  
Contractor: CMT Drilling Inc.  
Drilling Method: SPT

Elevation: 427.30 m  
Logged by: TS

Project No.: 18-368  
Project: Wilder Lake Subdivision  
Location: 263530 Southgate Road 26,  
Southgate, Ontario

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa 100 200 300 400
							10 20 30 40	SPT (N) Blows/0.3 m 20 40 60 80
0					Ground Surface (m) 427.30			
1	SS		1		<b>TOPSOIL</b> Loose, dark brown, silty organic topsoil, moist (250mm)		2.4	10
2								
3	SS		2		<b>GRAVEL AND SAND</b> Compact, brown gravel and sand, trace silt and clay, occasional cobbles, moist		2.7	27
4								
5								
6	SS		3				3.3	24
7								
8					425.01			
9	SS		4		Becoming very dense		2.8	50 (6")
10								
11	SS		5				3.0	50 (2")
12								
13	MC5		6				3.8	
14								
15								
16	SS		7				10.1	13
17					422.27			
18					<b>SAND AND SILT</b> Compact, brown sand and silt, some gravel, wet		10.3	
19	MC5		8		Becoming gravelly		8.2	
20					421.66			
21					421.20			
22					End of Borehole 6.10			

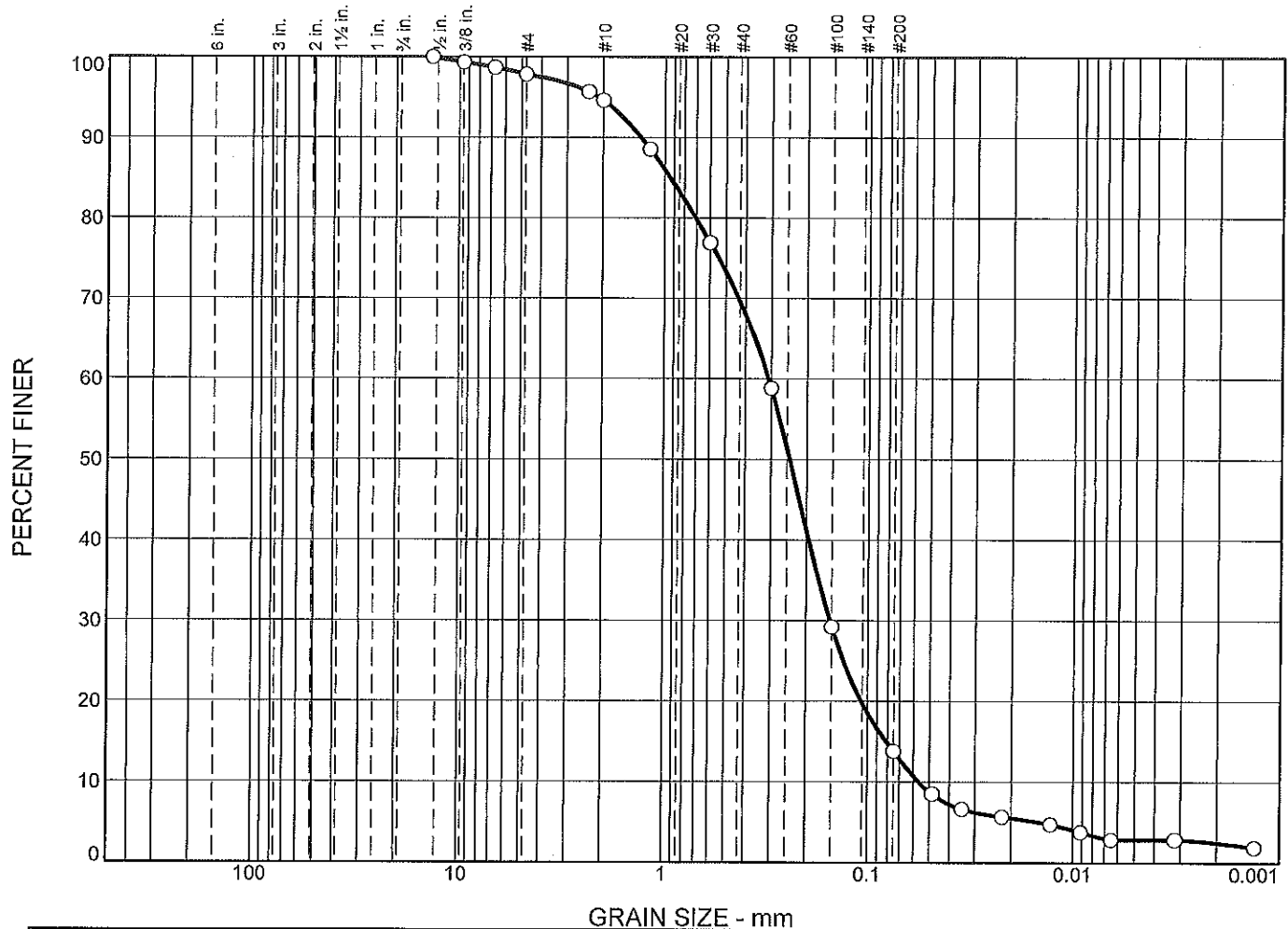
CMT ENGINEERING INC.  
1011 Industrial Crescent, Unit 1  
St. Clements, Ontario NOB 2M0  
phone 519-699-5775 fax 519-699-4664  
www.cmtinc.net



**APPENDIX B**  
**GRAIN SIZE ANALYSES**



# Particle Size Distribution Report



GRAIN SIZE - mm							
% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	2.1	3.3	25.1	55.7	11.3	2.5

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH1	3	1.52-2.13m	sand, some silt, trace clay and gravel	SM
				Estimated Percolation Rate; T = 10 min/cm	
				Estimated Coefficient of Permeability; k = 3.2 x 10 <sup>-3</sup> min/cm	
				Tested by JM of CMT Engineering Inc., August 1, 2018	

**CMT Engineering Inc.**

**St. Clements, ON**

**Client:** H. Bye Construction

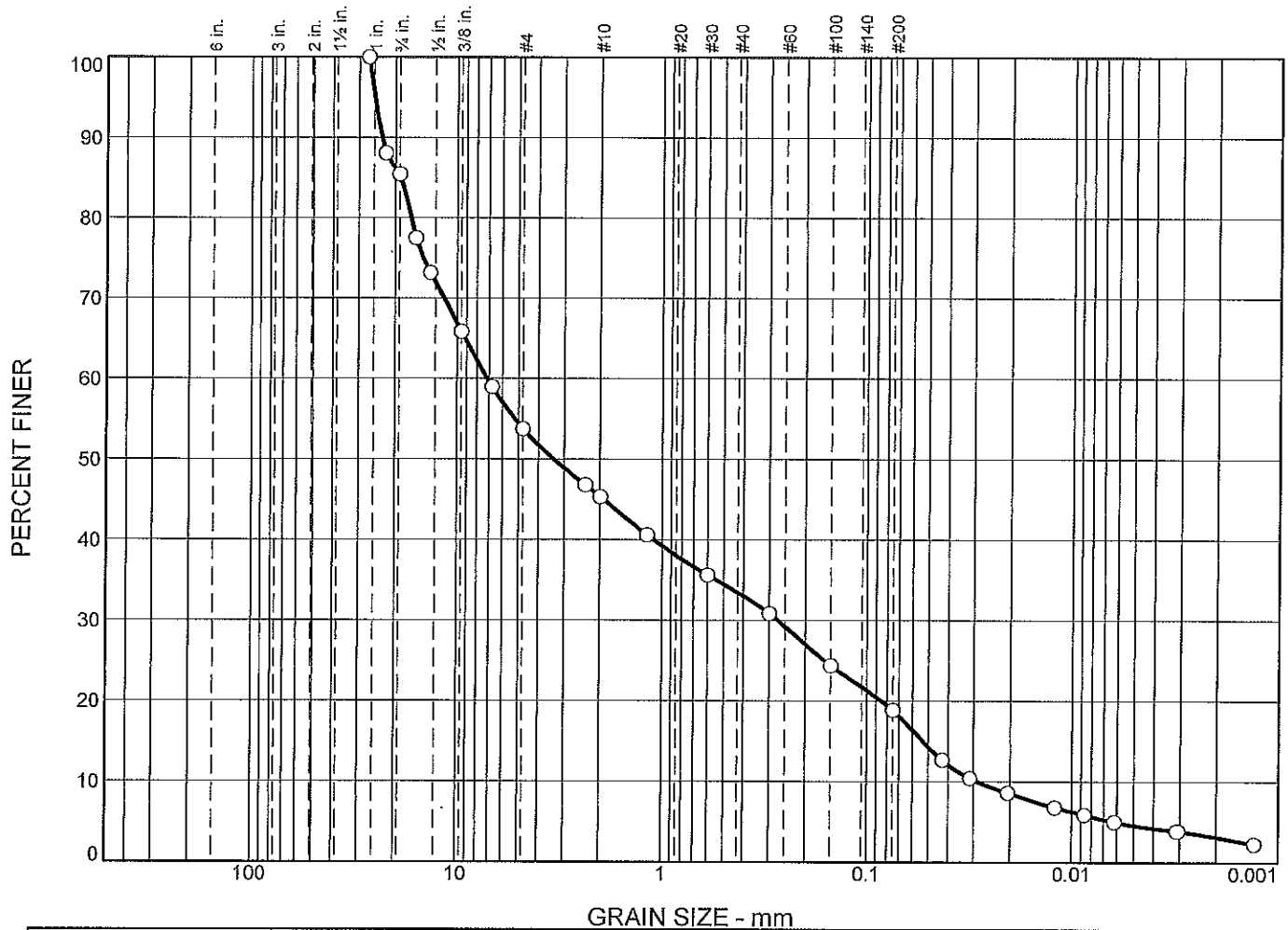
**Project:** Wilder Lake Subdivision

263530 Southgate Road 26, Southgate Twp, Ontario

**Project No.:** 18-368

**Figure 1**

# Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	14.5	31.8	8.4	11.9	14.6	15.7	3.1

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH2	3	1.22-2.13m	sandy gravel, some silt, trace clay	GM
				Estimated Percolation Rate; T = 12 min/cm	
				Estimated Coefficient of Permeability; k = 8.4 x 10 <sup>-4</sup> min/cm	
				Tested by JM of CMT Engineering Inc., August 1, 2018	

**CMT Engineering Inc.**

**St. Clements, ON**

**Client:** H. Bye Construction

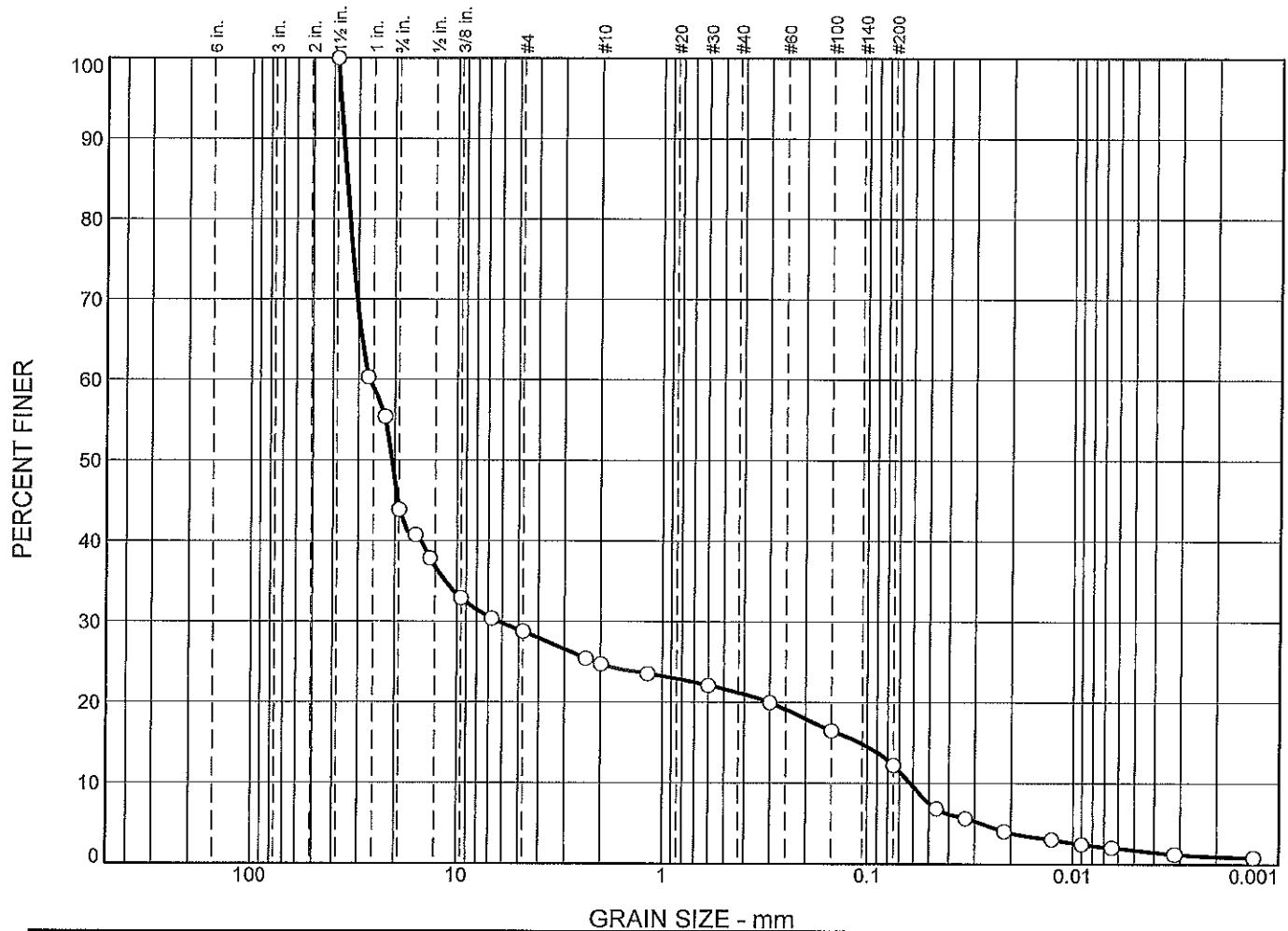
**Project:** Wilder Lake Subdivision

263530 Southgate Road 26, Southgate Twp, Ontario

**Project No.:** 18-368

**Figure** 2

# Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	56.1	15.1	4.1	3.5	9.0	11.3	0.9

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH3	2	0.76-1.37m	gravel, some sand and silt, trace clay	GM
				Estimated Percolation Rate; T = 8 min/cm	
				Estimated Coefficient of Permeability; k = 3.9 x 10 <sup>-3</sup> min/cm	
				Tested by JM of CMT Engineering Inc., August 1, 2018	

**CMT Engineering Inc.**

**St. Clements, ON**

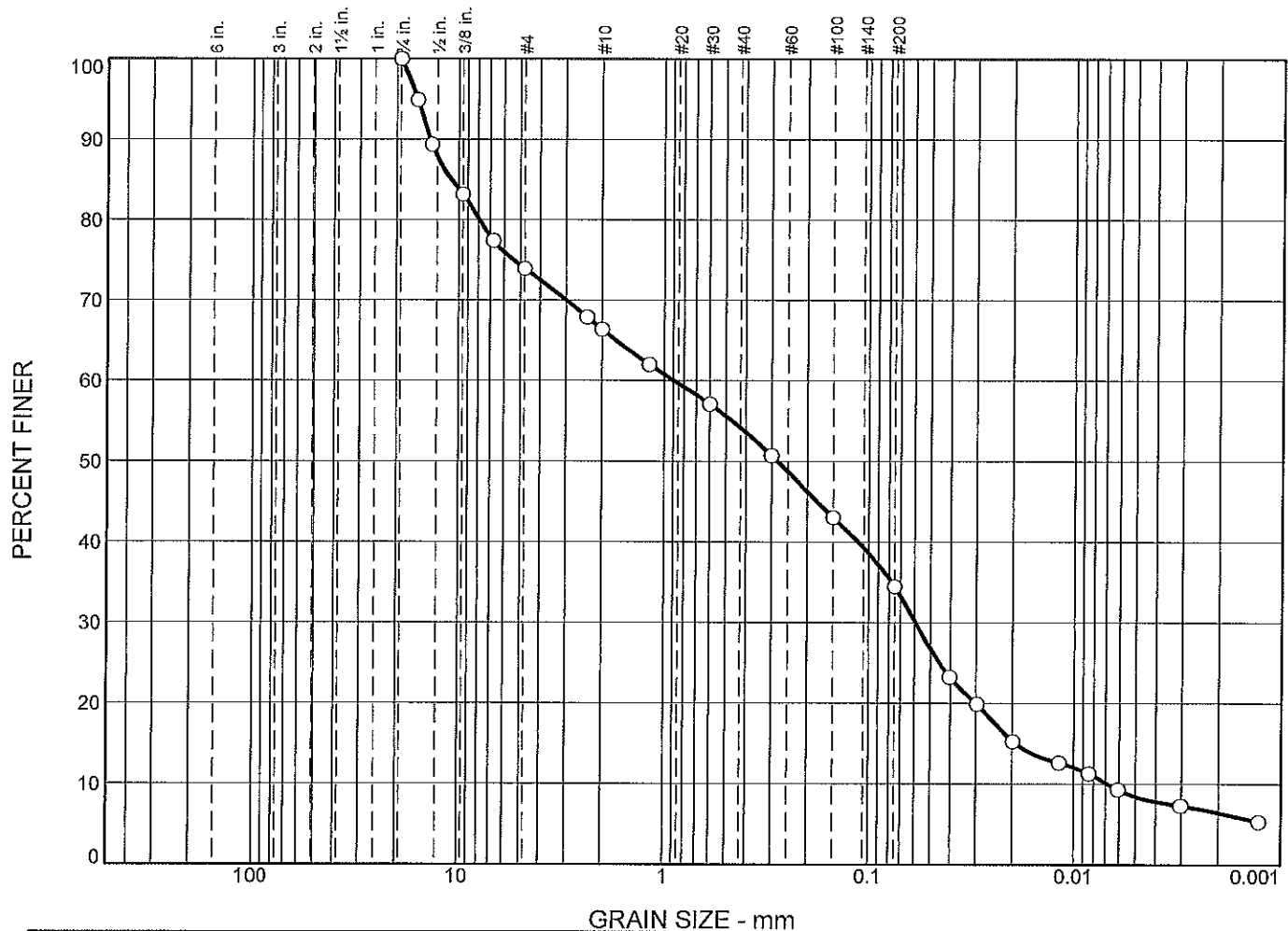
**Client:** H. Bye Construction

**Project:** Wilder Lake Subdivision  
263530 Southgate Road 26, Southgate Twp, Ontario

**Project No.:** 18-368

**Figure** 3

# Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	26.1	7.5	12.3	19.7	28.0	6.4

## SOIL DATA

SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH4	2	0.76-1.37m	silty, gravelly sand, trace clay	SM
				Estimated Percolation Rate; T = 15 min/cm	
				Estimated Coefficient of Permeability; k = 4.8 x 10 <sup>-4</sup> min/cm	
				Tested by JM of CMT Engineering Inc., August 1, 2018	

**CMT Engineering Inc.**

**St. Clements, ON**

**Client:** H. Bye Construction

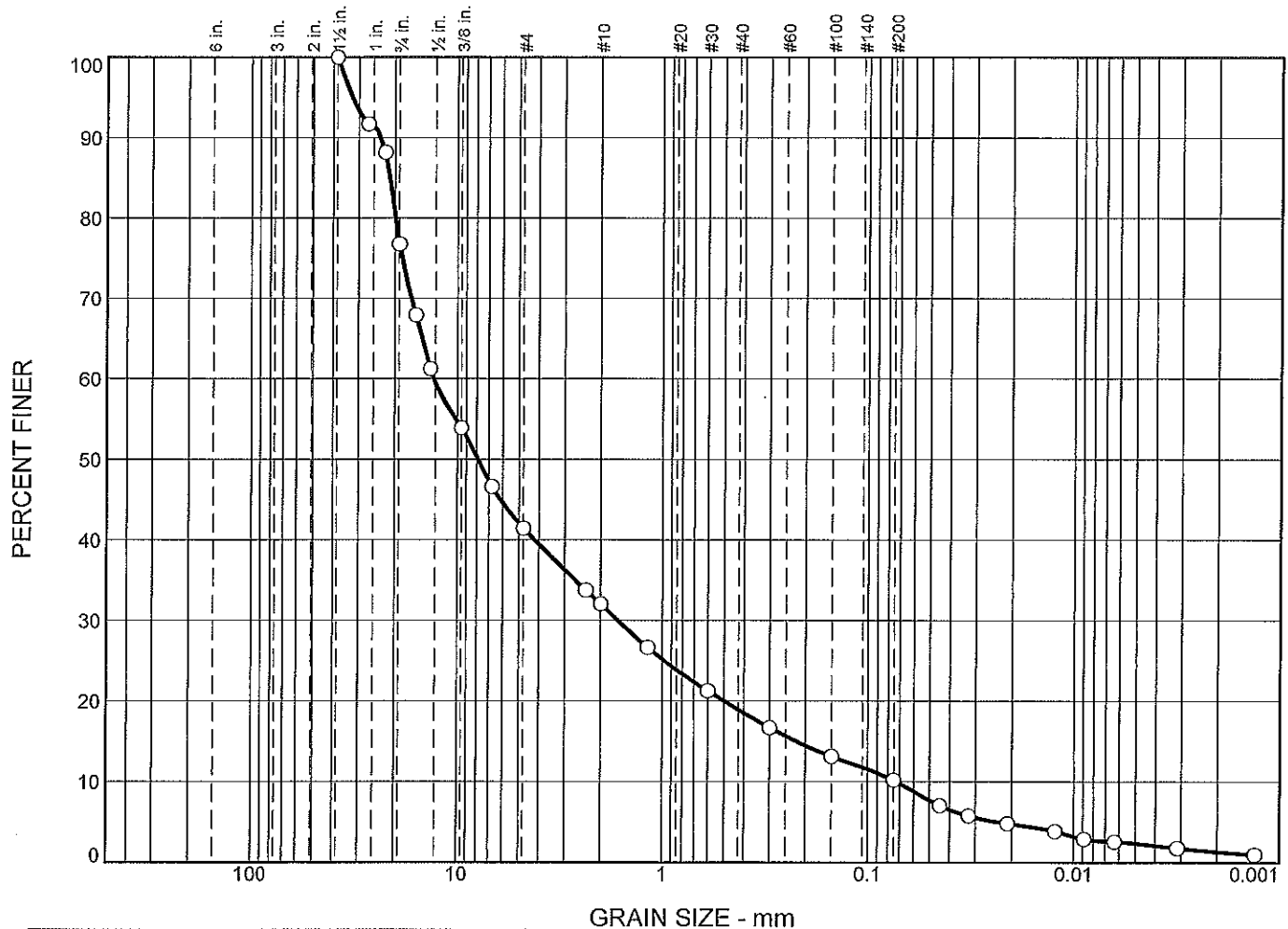
**Project:** Wilder Lake Subdivision

263530 Southgate Road 26, Southgate Twp, Ontario

**Project No.:** 18-368

**Figure** 4

# Particle Size Distribution Report



GRAIN SIZE - mm								
	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	23.2	35.4	9.4	13.1	8.7	8.9	1.3

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH5	2	0.76-1.37m	sandy gravel, trace silt and clay	GW-GM
				Estimated Percolation Rate; T = 8 min/cm	
				Estimated Coefficient of Permeability; k = 5.3 x 10 <sup>-3</sup> min/cm	
				Tested by JM of CMT Engineering Inc., August 1, 2018	

**CMT Engineering Inc.**

**St. Clements, ON**

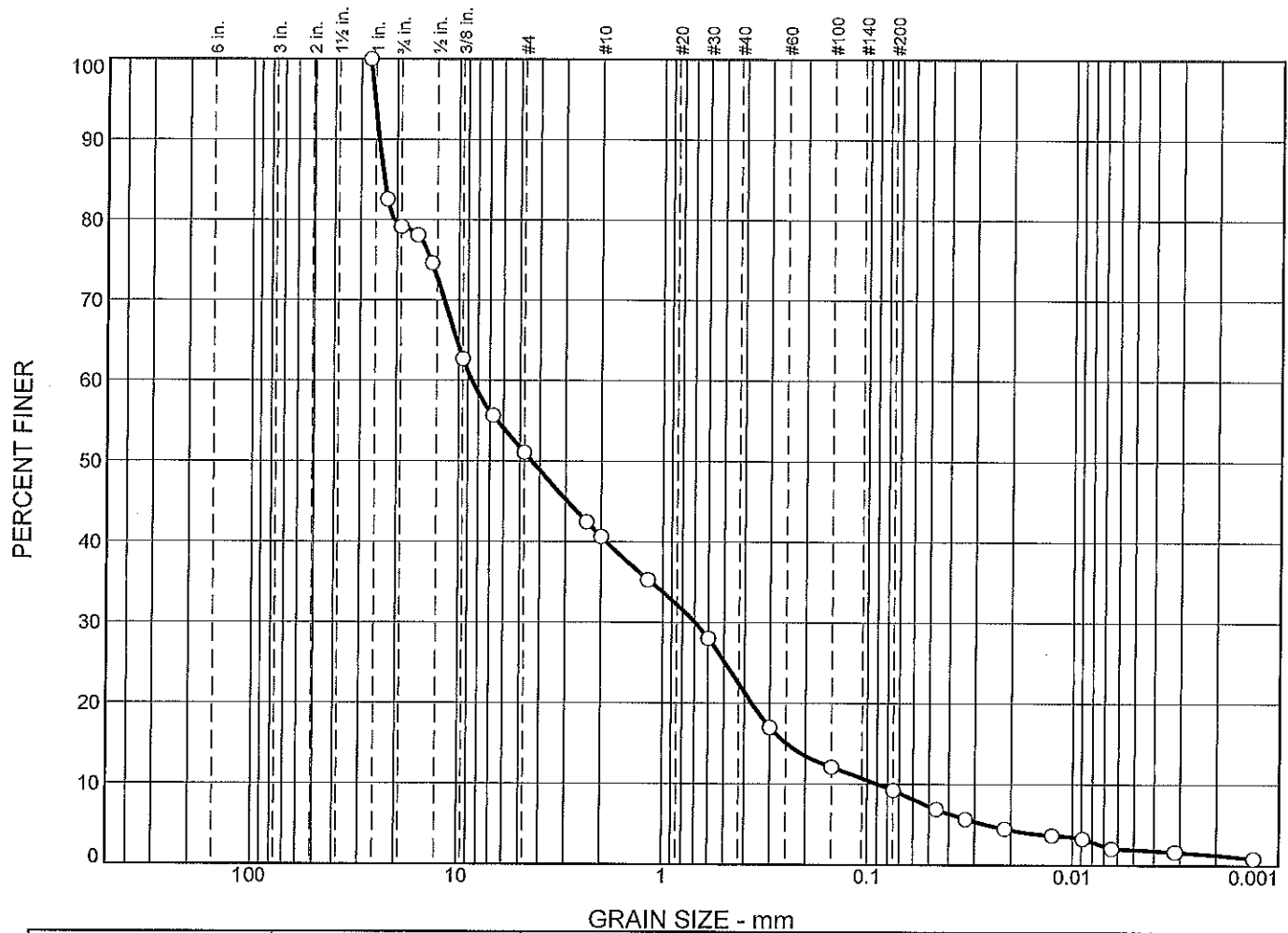
**Client:** H. Bye Construction

**Project:** Wilder Lake Subdivision  
263530 Southgate Road 26, Southgate Twp, Ontario

**Project No.:** 18-368

**Figure** 5

# Particle Size Distribution Report



GRAIN SIZE - mm

	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	20.8	28.2	10.4	18.2	13.2	8.0	1.2

## SOIL DATA

SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH6	2	0.76-1.37m	gravel and sand, trace silt and clay	GP-GM
				Estimated Percolation Rate; T = 8 min/cm	
				Estimated Coefficient of Permeability; k = 8.0 x 10 <sup>-3</sup> min/cm	
				Tested by JM of CMT Engineering Inc., August 1, 2018	

**CMT Engineering Inc.**

**St. Clements, ON**

**Client:** H. Bye Construction

**Project:** Wilder Lake Subdivision

263530 Southgate Road 26, Southgate Twp, Ontario

**Project No.:** 18-368

**Figure** 6

**APPENDIX C**  
**WELL RECORDS**





Ministry of the Environment and Climate Change

Well Tag No. (Place Sticker and/or Print Below)

Well Record

Regulation 903 Ontario Water Resources Act

Page 1 of 1

Measurements recorded in: ☒ Metric ☐ Imperial

A249064

Well Owner's Information

First Name

Last Name / Organization

E-mail Address

☐ Well Constructed by Well Owner

Mailing Address (Street Number/Name)

Municipality

Province

Postal Code

Telephone No. (inc. area code)

395 CHURCH ST N

MOUNT FOREST

ON

N0G2L4C5

1A323115210

Well Location

Address of Well Location (Street Number/Name)

Township

Lot

Concession

263530 SOUTHWEST TWP RD 210

County/District/Municipality

City/Town/Village

Province

Postal Code

DURHAM

Ontario

N0G1R0

UTM Coordinates Zone, Easting

Northing

Municipal Plan and Sublot Number

Other

NAD 83

17

520781

148879108

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m)
BLACK	TOPSOIL			0 0.3
BROWN	SAND	GRAVEL		0.3 5.1

Annular Space

Depth Set at (m)	To	Type of Sealant Used (Material and Type)	Volume Placed (m³)
0	1.5	3/8 HUEPUG	2.1
1.5	5.1	#2 SAND	2.1

Method of Construction

☐ Cable Tool ☐ Diamond ☐ Public ☐ Commercial ☐ Not used  
☐ Rotary (Conventional) ☐ Jetting ☐ Domestic ☐ Municipal ☐ Dewatering  
☐ Rotary (Reverse) ☒ Driving ☐ Livestock ☐ Test Hole ☒ Monitoring  
☐ Boring ☐ Digging ☐ Irrigation ☐ Cooling & Air Conditioning  
☐ Air percussion ☐ Industrial ☐ Other, specify

Well Use

☐ Water Supply ☐ Replacement Well ☐ Test Hole  
☐ Recharge Well ☐ Dewatering Well ☒ Observation and/or Monitoring Hole  
☐ Alteration (Construction) ☐ Abandoned, Insufficient Supply  
☐ Abandoned, Poor Water Quality ☐ Abandoned, other, specify  
☐ Other, specify

Construction Record - Casing

Inside Diameter (cm)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm)	Depth (m)	From	To	Status of Well
3.8	PLASTIC		0	2.1		<input type="checkbox"/> Water Supply <input type="checkbox"/> Replacement Well <input type="checkbox"/> Test Hole <input type="checkbox"/> Recharge Well <input type="checkbox"/> Dewatering Well <input checked="" type="checkbox"/> Observation and/or Monitoring Hole <input type="checkbox"/> Alteration (Construction) <input type="checkbox"/> Abandoned, Insufficient Supply <input type="checkbox"/> Abandoned, Poor Water Quality <input type="checkbox"/> Abandoned, other, specify <input type="checkbox"/> Other, specify

Construction Record - Screen

Outside Diameter (cm)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m)	From	To
4	PLASTIC	10	2.1	5.1	

Water Details

Water found at Depth (m)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	Depth (m)	To	Diameter (cm)
		0	5.1	10

Well Contractor and Well Technician Information

Business Name of Well Contractor

Well Contractor's Licence No.

CNT DRILLING INC.

7131616

Business Address (Street Number/Name)

Municipality

101 INDUSTRIAL CRES

WATERLOO

Province

Postal Code

Business E-mail Address

ON

N0B2M0

CNTAL@CNTAL.NS

Bus. Telephone No. (inc. area code)

Name of Well Technician (Last Name, First Name)

5166957761

BLACK CHRIS

Well Technician's Licence No.

Signature of Technician and/or Contractor

Date Submitted

1371111

CS-BR

20180808

Comments:

See MAP PER 115

Well owner's information package delivered ☐ Yes ☐ No

Date Package Delivered

20180727

Date Work Completed

20180727

Ministry Use Only

Audit No. 2273195

Received

050EE (2014/11)

Contractor's Copy

© Queen's Printer for Ontario, 2014

Measurements recorded in: ☒ Metric ☐ Imperial

Well Tag No. (Place Sticker and/or Print Below)

A249699

## Well Record

Regulation 903 Ontario Water Resources Act

Page { of }

### Well Owner's Information

First Name	Last Name / Organization	E-mail Address				<input type="checkbox"/> Well Constructed by Well Owner
	H.B.E CONSTRUCTION LTD					
Mailing Address (Street Number/Name)		Municipality	Province	Postal Code	Telephone No. (inc. area code)	
395 CHURCH ST N		MOUNT FOREST	ON	N6B 2L4	519 323 1520	

### Well Location

Address of Well Location (Street Number/Name) 263530 SOUTHWATE TWP. RD 26		Township		Lot		Concession	
County/District/Municipality		City/Town/Village DURHAM				Province Ontario	
Postal Code N6G 1R0		UTM Coordinates		Zone		Easting	
		Northing		Municipal Plan and Sublot Number		Other	
NAD 83		12		552088		11041088	

**Overburden and Bedrock Materials/Abandonment Sealing Record** (see instructions on the back of this form)

[illegible]

## Annular Space

Depth Set at (m) From	To	Type of Sealant Used (Material and Type)	Volume Placed (m³)
0	2.1	3/8 HOLEPLUG	2.1
2.1	5.7	#2 SAND	

### Results of Well Yield Testing

After test of well yield, water was:		Draw Down		Recovery	
<input type="checkbox"/> Clear and sand free <input type="checkbox"/> Other, specify _____		Time (min)	Water Level (m/fi)	Time (min)	Water Level (m/fi)
If pumping discontinued, give reason:		Static Level			
		1		1	
Pump intake set at (m/fi)		2		2	
Pumping rate (l/min / GPM)		3		3	
Duration of pumping		4		4	
_____ hrs + _____ min		5		5	
Final water level end of pumping (m/fi)		10		10	
If flowing give rate (l/min / GPM)		15		15	
Recommended pump depth (m/fi)		20		20	
		25		25	
Recommended pump rate		30		30	
(l/min / GPM)		40		40	
Well production (l/min / GPM)		50		50	
Disinfected?		60		60	
<input type="checkbox"/> Yes <input type="checkbox"/> No					

## Construction Record - Casing

Inside Diameter (in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (in)	Depth (ft)		<input type="checkbox"/> Water Supply <input type="checkbox"/> Replacement Well <input type="checkbox"/> Test Hole <input type="checkbox"/> Recharge Well <input type="checkbox"/> Dewatering Well <input checked="" type="checkbox"/> Observation and/or Monitoring Hole <input type="checkbox"/> Alteration (Construction) <input type="checkbox"/> Abandoned,
			From	To	
38	PLASTIC		0	2.7	

## Construction Record - Screen

Outside Diameter (mm)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (mm)	
			From	To
4	PLASTIC	10	27	57

☐ Abandoned; F-001  
☐ Water Quality  
☐ Abandoned, other, specify \_\_\_\_\_  
☐ Other, specify \_\_\_\_\_

### Water Details

Water found at Depth _____ (m/f) <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify _____	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested
Water found at Depth _____ (m/f) <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify _____	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested
Water found at Depth _____ (m/f) <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify _____	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested

## Hole Diameter

Depth From	Depth (m)	Diameter (cm)
0	5.7	10

### Well Contractor and Well Technician Information

Business Name of Well Contractor	Well Contractor's Licence No.
CMT DRILLING INC	713 16 16
Business Address (Street Number/Name)	Municipality
101 INDUSTRIAL CRES	WATERLOO

Province	Postal Code	Business E-mail Address
CN	N0B2Z0B	Online not
Bus. Telephone No. (inc. area code)		Name of Well Technician (Last Name, First Name)
514 694 5745		BLACK, CHRIS
Well Technician's Licence No.	Signature of Technician and/or Contractor	Date Submitted
37111	CS BCL	20180809

## Ministry Use Only

Well owner's information package delivered <input type="checkbox"/> Yes <input type="checkbox"/> No	Date Package Delivered	<b>Ministry Use Only</b> Audit No. <b>Z273194</b> Received
	Date Work Completed <b>20180727</b>	

Measurements recorded in: ☐ Metric ☐ Imperial

Page of

A249065

## Well Owner's Information

First Name	Last Name / Organization	E-mail Address	<input type="checkbox"/> Well Constructed by Well Owner	
	H. BHE CONSTRUCTION LTD			
Mailing Address (Street Number/Name)	Municipality	Province	Postal Code	Telephone No. (inc. area code)
395 CHURCH ST N	MOUNT FOREST	ON	N0G2L0	51932311520

## Well Location

Address of Well Location (Street Number/Name)	Township	Lot	Concession
263530 SOUTHGATE TWP RD 26			
County/District/Municipality	City/Town/Village	Province	Postal Code
	DURHAM	Ontario	N0G1R0
UTM Coordinates	Zone	Easting	North
NAD 83	17	520511	648187900
Municipal Plan and Sublot Number		Other	

## Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m)
BLACK	TOPSOIL		Loose	0 to 0.3
BROWN	SAND	GRAVEL		0.3 to 6

Annular Space			Results of Well Yield Testing						
Depth Set at (m/l) From		Type of Sealant Used (Material and Type)	Volume Placed (m <sup>3</sup> /l)	After test of well yield, water was: <input type="checkbox"/> Clear and sand free <input type="checkbox"/> Other, specify _____		Draw Down		Recovery	
	To					Time (min)	Water Level (m/l)	Time (min)	Water Level (m/l)
0	2.1	3/8 HOLEPLUG	2.1						
2.1	6	#2 SAND							
									</

Method of Construction	Well Use
<input type="checkbox"/> Cable Tool <input type="checkbox"/> Rotary (Conventional) <input type="checkbox"/> Rotary (Reverse) <input type="checkbox"/> Boring <input type="checkbox"/> Air percussion <input type="checkbox"/> Other, specify	<input type="checkbox"/> Diamond <input type="checkbox"/> Jetting <input checked="" type="checkbox"/> Driving <input type="checkbox"/> Digging <input type="checkbox"/> Public <input type="checkbox"/> Domestic <input type="checkbox"/> Livestock <input type="checkbox"/> Irrigation <input type="checkbox"/> Industrial <input type="checkbox"/> Other, specify

Construction Record - Casing				Status of Well	
Inside Diameter (cm)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Well Thickness (cm)	Depth (m)	From	To
38	PLASTIC		0	3	

Construction Record - Screen				Status of Well	
Outside Diameter (cm)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m)	From	To
4	PLASTIC	10	3	6	

Water Details		Hole Diameter	
Water found at Depth (m)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	Depth (m)	Diameter (cm)
		From	To
Water found at Depth (m)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	0	6
Water found at Depth (m)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify		10

Well Contractor and Well Technician Information			
Business Name of Well Contractor		Well Contractor's Licence No.	
CMT DRILLING INC		713 16 16	
Business Address (Street Number/Name)		Municipality	
1011 INDUSTRIAL CRES		WATERLOO	
Province	Postal Code	Business E-mail Address	
ON	N0B2L0	CMT INC. NET	
Bus. Telephone No. (inc. area code)		Name of Well Technician (Last Name, First Name)	
5146985775		BLACK, CHRIS	
Well Technician's Licence No.		Signature of Technician and/or Contractor	
371111		C2 BOR	
Date Submitted			
20180808			

Map of Well Location			
Please provide a map below following instructions on the back.			
SEE MAP FOR #3			

Comments:	Well owner's information package delivered <input type="checkbox"/> Yes <input type="checkbox"/> No	Date Package Delivered YYY Y MM DD 2018 08 08	Date Work Completed 2018 08 27
Ministry Use Only		Audit No. 2273197	
Resolved			



Measurements recorded in: ☒ Metric ☐ Imperial

Page 1 of 1

Well Owner's Information

First Name	Last Name / Organization	E-mail Address	<input type="checkbox"/> Well Constructed by Well Owner
	H. Bye Construction Ltd.		
Mailing Address (Street Number/Name)	Municipality	Province	Postal Code
395 Church St N	Mount Forest	ON	N0B6Z1L0
Telephone No. (inc. area code)			
519 323 1151		210	

Well Location

Address of Well Location (Street Number/Name)	Township	Lot	Concession
263530 Southgate Twp Rd 26			
County/District/Municipality	City/Town/Village	Province	Postal Code
	Durham	Ontario	N0B6Z1L0
UTM Coordinates Zone	Eastings	Northings	
NAD 83	17	5210526	418187589
Municipal Plan and Sublot Number		Other	

Overburden and Bedrock Materials/Abandonment Sealing Record (see instructions on the back of this form)

General Colour	Most Common Material	Other Materials	General Description	Depth (m/ft)
Black	Topsoil			From To
Brown	Sand	Gravel		0 0.3
				0.3 6.1

Annular Space			Results of Well Yield Testing		
Depth Set at (m/ft)	Type of Sealant Used (Material and Type)	Volume Placed (m <sup>3</sup> /ft <sup>3</sup> )	After test of well yield, water was:	Draw Down	Recovery
0 2.7	3/8 hd plug	2.1	<input type="checkbox"/> Clear and sand free	Time (min)	Water Level (m/ft)
2.7 6.1	#2 sand	2.1	<input type="checkbox"/> Other, specify	Time (min)	Water Level (m/ft)
			If pumping discontinued, give reason:	Static Level	
				1	1
			Pump intake set at (m/ft)	2	2
			Pumping rate (l/min / GPM)	3	3
			Duration of pumping	4	4
			hrs + min	5	5
			Final water level end of pumping (m/ft)	10	10
			If flowing give rate (l/min / GPM)	15	15
				20	20
			Recommended pump depth (m/ft)	25	25
			Recommended pump rate (l/min / GPM)	30	30
			Well production (l/min / GPM)	40	40
			Disinfected?	50	50
			<input type="checkbox"/> Yes <input type="checkbox"/> No	60	60

Method of Construction		Well Use	
<input type="checkbox"/> Cable Tool	<input type="checkbox"/> Diamond	<input type="checkbox"/> Public	<input type="checkbox"/> Commercial
<input type="checkbox"/> Rotary (Conventional)	<input type="checkbox"/> Jetting	<input type="checkbox"/> Domestic	<input type="checkbox"/> Municipal
<input type="checkbox"/> Rotary (Reverse)	<input checked="" type="checkbox"/> Driving	<input type="checkbox"/> Livestock	<input type="checkbox"/> Test Hole
<input type="checkbox"/> Boring	<input type="checkbox"/> Digging	<input type="checkbox"/> Irrigation	<input type="checkbox"/> Cooling & Air Conditioning
<input type="checkbox"/> Air percussion		<input type="checkbox"/> Industrial	
<input type="checkbox"/> Other, specify		<input type="checkbox"/> Other, specify	

Construction Record - Casing			Status of Well	
Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/ft)	
3.8	Plastic		0 3.1	

Construction Record - Screen			
Outside Diameter (cm/in)	Material (Plastic, Galvanized, Steel)	Slot No.	Depth (m/ft)
4	Plastic	10	3.1 6.1

Water Details		Hole Diameter	
Water found at Depth (m/ft)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify	Depth (m/ft)	Diameter (cm/in)
		0 6.1	10
Water found at Depth (m/ft)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify		
Water found at Depth (m/ft)	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Gas <input type="checkbox"/> Other, specify		

Well Contractor and Well Technician Information			
Business Name of Well Contractor		Well Contractor's Licence No.	
CHT Drilling Inc		713 16 16	
Business Address (Street Number/Name)		Municipality	
1011 Industrial Cres		Waterloo	
Province	Postal Code	Business E-mail Address	
ON	N0B6Z1L0	cmhinc@xkx	
Bus. Telephone No. (inc. area code)		Name of Well Technician (Last Name, First Name)	
519 699 5555		Block, Chns	
Well Technician's Licence No.		Signature of Technician and/or Contractor	
3171111		20180808	

Map of Well Location		
Please provide a map below following instructions on the back.		
SEE MAP FOR #2		

Comments:	Well owner's information package delivered	Date Package Delivered	Ministry Use Only
	<input type="checkbox"/> Yes <input type="checkbox"/> No	20180808	Audit No. 2273198
		Date Work Completed	Received
		20180808	

Measurements recorded in: ☒ Metric ☐ Imperial

A249100

### Well Owner's Information

First Name	Last Name / Organization	E-mail Address			<input type="checkbox"/> Well Constructed by Well Owner
	H. Bye Construction Ltd.				
Mailing Address (Street Number/Name)	Municipality	Province	Postal Code	Telephone No. (inc. area code)	
395 Church St N	Mount Forest	ON	N0G2Y4P5	519 323 1526	

## Well Location

Address of Well Location (Street Number/Name) 263530 Sawtooth twp Rd 26		Township	Lot	Concession	
County/District/Municipality		City/Town/Village Durham	Province Ontario		Postal Code N0G1V0
UTM Coordinates Zone Easting Northing		Municipal Plan and Sublot Number		Other	
NAD 83 17T 52Q 81A 48876125					

**Overburden and Bedrock Materials/Abandonment Sealing Record** (see instructions on the back of this form)

[illegible]

Annular Space			Results of Well Yield Testing					
Depth Set at (mft) From To		Type of Sealant Used (Material and Type)	Volume Placed (m <sup>3</sup> /ft <sup>3</sup> )	After test of well yield, water was: <input type="checkbox"/> Clear and sand free <input type="checkbox"/> Other, specify _____	Draw Down		Recovery	
					Time (min)	Water Level (mft)	Time (min)	Water Level (mft)
0	15	3/8 hole plug	21	If pumping discontinued, give reason:	Static Level			
15	64	#2 sand			1		1	
				Pump intake set at (mft)	2		2	

Method of Construction		Well Use		
<input type="checkbox"/> Cable Tool	<input type="checkbox"/> Diamond	<input type="checkbox"/> Public	<input type="checkbox"/> Commercial	<input type="checkbox"/> Not used
<input type="checkbox"/> Rotary (Conventional)	<input type="checkbox"/> Jetting	<input type="checkbox"/> Domestic	<input type="checkbox"/> Municipal	<input type="checkbox"/> Dewatering
<input type="checkbox"/> Rotary (Reverse)	<input checked="" type="checkbox"/> Driving	<input type="checkbox"/> Livestock	<input type="checkbox"/> Test Hole	<input checked="" type="checkbox"/> Monitoring
<input type="checkbox"/> Boring	<input type="checkbox"/> Digging	<input type="checkbox"/> Irrigation	<input type="checkbox"/> Cooling & Air Conditioning	
<input type="checkbox"/> Air percussion		<input type="checkbox"/> Industrial		
<input type="checkbox"/> Other, specify _____		<input type="checkbox"/> Other, specify _____		

Construction Record - Casing				Status of Well	
Inside Diameter (cm/in)	Open Hole OR Material (Galvanized, Fibreglass, Concrete, Plastic, Steel)	Wall Thickness (cm/in)	Depth (m/in)		<input type="checkbox"/> Water Supply <input type="checkbox"/> Replacement Well <input type="checkbox"/> Test Hole <input type="checkbox"/> Recharge Well <input type="checkbox"/> Dewatering Well <input checked="" type="checkbox"/> Observation and/or Monitoring Hole <input type="checkbox"/> Alteration (Construction) <input type="checkbox"/> Abandoned,
			From	To	
3.8	Plastic		0	3.3	

Construction Record - Screen				
Outside Diameter ( <i>mm</i> )	Material (Plastic, Galvanized, Steel)	Slot No.	Depth ( <i>mm</i> )	
			From	To
4	Plastic	10	3.3	6.4

☐ Insufficient Supply  
☐ Abandoned, Poor Water Quality  
☐ Abandoned, other, specify \_\_\_\_\_  
☐ Other, specify \_\_\_\_\_

Water Details		Hole Diameter	
Water found at Depth (m/ft) <input type="checkbox"/> Gas	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Other, specify _____	Depth (m/ft) From _____ to _____	Diameter (cm/in)
Water found at Depth (m/ft) <input type="checkbox"/> Gas	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Other, specify _____	2	6.4
Water found at Depth (m/ft) <input type="checkbox"/> Gas	Kind of Water: <input type="checkbox"/> Fresh <input type="checkbox"/> Untested <input type="checkbox"/> Other, specify _____		10

## Well Contractor and Well Technician Information

Business Name of Well Contractor		Well Contractor's Licence No.
CMT Drilling Inc		7366
Business Address (Street Number/Name)		Municipality
1011 Industrial Cres		Waterloo
Province	Postal Code	Business E-mail Address
ON	N6B2M0	cmtinc.net
Bus. Telephone No. (inc. area code)		Name of Well Technician (Last Name, First Name)
5146945445		Block, Chns
Well Technician's Licence No.	Signature of Technician and/or Contractor	Date Submitted
1371111	(Signature)	2011/08/08

### Results of Well Yield Testing

After test of well yield, water was: <input type="checkbox"/> Clear and sand free <input type="checkbox"/> Other, <i>specify</i> _____	Draw Down		Recovery	
	Time (min)	Water Level (m/ft)	Time (min)	Water Level (m/ft)
If pumping discontinued, give reason:	Static Level			
	1		1	
Pump intake set at (m/ft)	2		2	
Pumping rate (l/min / GPM)	3		3	
	4		4	
Duration of pumping _____ hrs + _____ min	5		5	
Final water level end of pumping (m/ft)	10		10	
	15		15	
If flowing give rate (l/min / GPM)	20		20	
	25		25	
Recommended pump depth (m/ft)	30		30	
	40		40	
Recommended pump rate (l/min / GPM)	50		50	
	60		60	
Well production (l/min / GPM)				
Disinfected? <input type="checkbox"/> Yes <input type="checkbox"/> No				

### Map of Well Location

Please provide a map below following instructions on the back.

SEE MAP FOR IT

Comments

Well owner's Information package delivered	Date Package Delivered	<b>Ministry Use Only</b> Audit No. <b>2273199</b>  Received
	Y Y Y Y M M D D Date Work Completed <b>20180727</b>	



263530 Southgate Township Road 26

Legend

 Monitoring Well



Google Earth

Image © 2018 DigitalGlobe

300 m